



# Town of Belleair

901 Ponce de Leon Blvd.  
Belleair, FL 33756

## Meeting Agenda Infrastructure Board

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Monday, August 31, 2020

5:00 PM

Town Hall

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Please enter the link below to join the webinar: <https://us02web.zoom.us/j/81378605535>

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Webinar ID: 813 7860 5535

Welcome. We are glad to have you join us. If you wish to speak, please wait to be recognized, then step to the podium and state your name and address. We also ask that you please turn-off all cell phones.

### ROLL CALL

### SCHEDULED PUBLIC HEARING

Persons are advised that, if they decide to appeal any decision made at this meeting/hearing, they will need a record of the proceedings, and, for such purposes, they may need to ensure that a verbatim record of the proceedings is made, which record includes the testimony and evidence upon which the appeal is to be based.

### CITIZENS COMMENTS

*(Discussion of items not on the agenda. Each speaker will be allowed 3 minutes to speak.)*

### APPROVAL OF MINUTES

[20-0048](#) Approval of May 20,2019, January 6, 2020 and February 17, 2020 Meeting Minutes

Attachments: [Infra 01-06-2020](#)

[Infra 02-17-2020](#)

[Infra 05-20-2019](#)

### GENERAL AGENDA

[20-0208](#) Election of Officers

[20-0213](#) Discussion of Draft Reverse Osmosis Preliminary Engineering Report and Water Rate Study

*Attachments:* [Town of Belleair Draft Executive Summary 8.28.2020](#)  
[Belleair RO WTP\\_draft\\_8-3-20\\_reduced](#)

[20-0214](#) Discussion of Capital Improvements Master Plan

*Attachments:* [MCKCIP](#)  
[CIP - Copy of Working Drafts](#)

**OTHER BUSINESS**

**COMMISSION ADVISOR REPORT**

**ADJOURNMENT**

ANY PERSON WITH A DISABILITY REQUIRING REASONABLE ACCOMMODATIONS IN ORDER TO PARTICIPATE IN THIS MEETING, SHOULD CALL (727) 588-3769 OR FAX A WRITTEN REQUEST TO (727) 588-3767.



Legislation Details (With Text)

<b>File #:</b>	20-0048	<b>Version:</b>	1	<b>Name:</b>	
<b>Type:</b>	Minutes	<b>Status:</b>		Minutes Approval	
<b>File created:</b>	2/13/2020	<b>In control:</b>		Infrastructure Board	
<b>On agenda:</b>	8/31/2020	<b>Final action:</b>			
<b>Title:</b>	Approval of May 20,2019, January 6, 2020 and February 17, 2020 Meeting Minutes				
<b>Sponsors:</b>					
<b>Indexes:</b>					
<b>Code sections:</b>					
<b>Attachments:</b>	<a href="#">Infra 01-06-2020</a> <a href="#">Infra 02-17-2020</a> <a href="#">Infra 05-20-2019</a>				

Date	Ver.	Action By	Action	Result
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Summary

To: Board Members  
From: Christine Nicole  
Date: 7/27/2020

**Subject:**  
Approval of meeting minutes

**Summary:**

- The following minutes are up for approval:
- May 20, 2019 - This item was on the August 5, 2020 agenda for approval, however there was no quorum at that meeting. It should have been carried forward to the January 6, 2020 meeting for approval, but was missed.
  - January 6, 2020 - This item was on the February 17, 2020 agenda for approval; the February meeting did not have a quorum for approval.
  - February 17, 2020 - Most recent meeting of the board, minutes are up for approval

**Previous Commission Action:** N/A

**Background/Problem Discussion:** N/A

**Expenditure Challenges:** N/A

**Financial Implications:** N/A

**Recommendation:** N/A

**Proposed Motion:** N/A



# Town of Belleair

901 Ponce de Leon Blvd.  
Belleair, FL 33756

## Meeting Minutes Infrastructure Board

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Monday, January 6, 2020

5:00 PM

Town Hall

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**Welcome. We are glad to have you join us. If you wish to speak, please wait to be recognized, then step to the podium and state your name and address. We also ask that you please turn-off all cell phones.**

Meeting was called to order at 5:00 PM with Chairman Pace presiding.

### ROLL CALL

**Present** 4 - Chairman Doug Pace, John Hail, James White, and Joe Oder

**Absent** 3 - Chris Foley, Ron Campbell, and Shon Flaharty

### CITIZENS COMMENTS

Jim White spoke about RPD and fire requirements regarding high rise buildings; communications survey requirements; spoke with Largo Fire Chief about identifying standards for surveyors; discussed getting with the County regarding possible towers.

Gayle Grady (Board Consultant) arrived at 5:06 PM

### APPROVAL OF MINUTES

[19-0270](#) Approval of August 5, 2019 Meeting Minutes

**Mr. Oder recommended approval. Seconded by Mr. Hail.**

**Aye:** 4 - Chairman Pace, Hail, White, and Oder

**Absent:** 3 - Foley, Campbell, and Flaharty

### GENERAL AGENDA

[19-0362](#) CIP plan update by Phil Locke

JP Murphy-Town Manager-Discussed previous actions regarding CIP development; need to prioritize projects to review plan and have formalized prior to budget time.

Discussion regarding additional project funds/emergency funds; general fund reserves; Mr. Murphy spoke about various roadway projects and funding.

Phil Locke-Engineer, McKim and Creed-Updates of plan will focus on project priorities and criteria; updates to construction costs, schedules, and informational project summaries for residents; goal to establish a framework for 0-3 year time period; reviewed suggested items; background, goals and priorities; discussed

recommendations.

Mr. Murphy tasked board to select top 3-5; board identified: training staff for pavement evaluations.

Discussion ensued regarding maintaining roads; obtaining potential cost sharing funding from other municipalities and County; Bluff; Ahlf property; waiting to get further information variables from engineers before making a final decision.

Ms. Grady provided comments on information presented; comprehensive and long term; establish costs and schedule; sharing funding; identify safety issues and ADA compliance.

Mr Murphy discussed funds set aside for facilities; capital projects.

Mr. Locke stated he will come up with about 10 items (priorities) and bring back to board. Mr. Murphy suggested identifying critical projects; develop short 1-page CIP project summary; discussed past roadway projects.

Discussion ensued regarding Harold's Lake and cooperative funding opportunities.

Commissioner Kurey provided comments on what would be helpful; priority list to be provided to the board.

The board requested to have a critical priority list, with explanations, provided to them at the next meeting.

#### [19-0360](#)

#### Approval of the RO Water Plant Preliminary Engineering Report (PER) - Phase II

Mr. Murphy discussed previous studies; water can be treated in phases however it must be engineered to do so; recommends moving forward with PER (preliminary engineering report) study which will include a rate model.

Mr. Locke provided a high level of what the PER entails; developing a phased project implementation of wells coming online with RO; every aspect will be reviewed.

Commissioner Kurey inquired about safety; Mr. Locke noted corrosion and damage to structures - employee safety concern with plant not water quality.

Mr Murphy recommends the board give recommendation to commission.

Gayle Grady commented on cost of study; feels the below grade well evaluation is a high number. Mr. Locke stated a well evaluation is necessary; sub contractor costs are the same as other cities.

Commissioner Kurey expressed his thoughts and concerns regarding price.

Discussion ensued regarding cost of well evaluation; obtaining more pricing information; moving forward.

**Mr. Oder made a motion to move forward. Seconded by Mr. White.**

**Aye:** 4 - Chairman Pace, Hail, White , and Oder

**Absent:** 3 - Foley, Campbell, and Flaharty

[19-0359](#) Potable Water System Hydraulic Modeling - PH II

Mr Murphy stated this was discussed in the previous item; will be necessary as well; seeking recommendation to move forward.

[19-0361](#) Town Street Lighting - update on Duke Energy LED upgrades

Keith Bodeker-Construction Project Supervisor-Has been in contact with Duke Energy, they're changing lighting to LED; looking to upgrade to all LED as well as install additional lights in darker areas; to begin this month; Duke is maintaining.

Mr. Murphy stated Duke has created a 5G co-located pole; will match others and has a small antenna on top; better ability for town to control carriers if offer this option.

Discussion ensued regarding street lighting and light replacement and time frames.

[19-0327](#) Discussion of Action Items for the Strategic Plan

Mr. Murphy stated there were a few items needing feedback; discussed objectives and action items needed; recommends possibly a subcommittee meeting to address the items, then bring back to board; would like to deliver to the Commission in April; all boards are reviewing assigned areas.

Mr. White suggests subcommittee meetings; work session style.

**OTHER BUSINESS**

None

**COMMISSION ADVISOR REPORT**

Commissioner Kurey stated he was happy with the strategic plan process.

**ADJOURNMENT**

Meeting adjourned in due form at 6:51 PM.

**Mr. Hail moved to adjourn. Seconded by Mr. White.**

**Aye:** 4 - Chairman Pace, Hail, White , and Oder

**Absent:** 3 - Foley, Campbell, and Flaharty

**APPROVED:**

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**CHAIRMAN**



# Town of Belleair

901 Ponce de Leon Blvd.  
Belleair, FL 33756

## Meeting Minutes Infrastructure Board

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Monday, February 17, 2020

5:00 PM

Town Hall

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### Strategic Planning Workshop I

**Welcome. We are glad to have you join us. If you wish to speak, please wait to be recognized, then step to the podium and state your name and address. We also ask that you please turn-off all cell phones.**

Meeting called to order at 5:05 PM with John Hail presiding.

#### ROLL CALL

**Present** 3 - John Hail, James White, and Joe Oder

**Absent** 4 - Chairman Doug Pace, Chris Foley, Ron Campbell, and Shon Flaharty

#### CITIZENS COMMENTS

None

#### APPROVAL OF MINUTES

[20-0048](#) Approval of January 6, 2020 Meeting Minutes

No quorum present

#### GENERAL AGENDA

[20-0038](#) Discussion of Action Items for the Strategic Plan

Mr. Murphy stated this is a work session for strategic plan purposes, there will be a second workshop next week.

Board discussions regarding underground utilities; cost; investigating potential to approach in phases; purpose of beautification vs resiliency. Consensus that seems cost prohibitive; roadways are priority.

Deputy Mayor Rettstatt spoke on Belleair Beach residents paying portions to get undergrounding in community. Board discussion regarding utility undergrounding; areas already underground; costs.

Mr. Murphy stated further investigation can be done; important to identify what is most important; possibility for grants and individual resident contributions.

Deputy Mayor Rettstatt discussed her construction and underground; clearing backyards with overhead lines to prevent power interruption.

Commissioner Kurey stated underground was a high response item from community outreach; important to have Duke Energy provide an estimate but for no cost.

Mr. Murphy spoke on prioritizing from perspective of right of way and rear yards; options; agree not wanting to spend a large amount on engineering. Objective will be obtaining the Duke energy number then have a discussion on prioritization comments made on cobra head lighting; being changed to LED currently.

Mr. Murphy discussed roads - pavement condition evaluation next; seeking any additional comments from board; Mr. White suggested tying a few goals together such as goal 1 and goal 3; evaluation necessary; infrastructure master plan will address as well; bridge evaluation.

Mr. Murphy discussed goal 3 - multi-modal transportation; spoke on prioritizing pedestrian corridors; board spoke on additional traffic when apartments on Clearwater - Largo are completed; potential for a new traffic studies and funding; bluetooth beaconing.

Mr. Murphy stated action item for 2.1 could be specifically study cut through traffic; 2.3 action item could be if becoming golf cart community will allow to reduce speed on Indian Rocks Road to 25; prioritizing corridors.

Mr. Murphy stated last item is 6, relating to stormwater and drainage; will add environment to item language; importance of developing design standards based on desired outcome; resident education; action item for 6.1 either engage staff or EOR to develop stormwater design standards; 6.2 appears to be in line, tracks closely with PCI; discussed 6.3 as relates currently to rates on utility bill, opportunity for revenue.

Mr. Murphy concluded discussions; open to any additional suggestions.

## **OTHER BUSINESS**

Mr. White commended Chief Doyle for his help with the radio survey at the RPD.

## **COMMISSION ADVISOR REPORT**

Commissioner Kurey thanked board for their help with the strategic planning; Commission voted on water study, once information is back will bring back to board; spoke on the fire in the RPD, thanked all first responders for their ability to minimize the damages.

Mr. Murphy provided further information on the response to fire; provided update on water issue; RO request is with legislature, bill may or may not make it further.

Mr Hail questioned status of recycling; Mr. Murphy addressed; costs increasing, no solid answer just yet; \$80,000 increase; within next year a decision will need to be made.



**ADJOURNMENT**

No further business. Meeting adjourned at 6:24 PM.

**APPROVED:**

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Chairman



# Town of Belleair

901 Ponce de Leon Blvd.  
Belleair, FL 33756

## Meeting Minutes Infrastructure Board

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Monday, May 20, 2019

6:00 PM

Town Hall

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**Welcome. We are glad to have you join us. If you wish to speak, please wait to be recognized, then step to the podium and state your name and address. We also ask that you please turn-off all cell phones.**

Meeting was called to order at 6:03 PM with Chairman Pace presiding.

### ROLL CALL

**Present** 4 - Chairman Doug Pace, John Hail, James White, and Joe Oder

**Absent** 3 - Chris Foley, Ron Campbell, and Shon Flaharty

Others present:  
Commissioner Kurey and Deputy Mayor Rettstatt  
Consultant Gayle Grady

### SCHEDULED PUBLIC HEARING

Persons are advised that, if they decide to appeal any decision made at this meeting/hearing, they will need a record of the proceedings, and, for such purposes, they may need to ensure that a verbatim record of the proceedings is made, which record includes the testimony and evidence upon which the appeal is to be based.

### CITIZENS COMMENTS

None

### APPROVAL OF MINUTES

[19-0017](#) Approval of January 7, 2019 and February 25, 2019 Meeting Minutes

**Mr. Oder made a motion to go through. Seconded by John Hail.**

**Aye:** 4 - Chairman Pace, Hail, White, and Oder

**Absent:** 3 - Foley, Campbell, and Flaharty

### GENERAL AGENDA

[19-0144](#) General Capital Projects Update

JP Murphy-Town Manager-Provided comments on infrastructure projects; upcoming kickoff meetings for Pinellas/Ponce and Palmetto for residents; Pinellas Park laydown area; paver expenses will not be covered; uniformity for sidewalks and ADA compliance.

Deputy Mayor Rettstatt questioned if staging area will be fenced; Keith Bodeker construction project Manager stated areas will be fenced.

Mr. Murphy continued with discussion on Palmetto South; same contractor; Palmetto Road North - awaiting Governor to sign for funding.

Mr. Hail inquired about the funding going to 3 possibly 4 projects for the same contractor K&R; Mr. Murphy stated that yes, both the Pinellas/Ponce and Palmetto contractor would be K&R and had gone out to bid; staff will do some sidewalk work and drainage on Bellforest.

Discussion ensued regarding piping and drainage along Waterfall Park.

Mr. Murphy continued projects commenting on Bluff project EOR; Tackett Park still under construction, grand opening on June 13th; streetlights continue to have conduit and wiring replaced throughout town; staff looking into battery back up for monitoring lighting; 5 year CIP included in packet; need to schedule a joint Finance Board & Infrastructure Board meeting to discuss the funding and financing of future projects.

Mr. Oder spoke with RPD regarding underground utilities; cost of entire town would be prohibitive; Mr. Murphy was in agreement.

Board discussions ensued regarding need for feasibility study on cost of undergroundling; installing dark conduit during roadway projects; need to schedule joint meeting with finance board.

Mr. Murphy stated awaiting appraisal of Ahlf property; revenue sources on current 5 year plan; summarized projects on plan; need to reprioritize projects; reviewed the 5-year term for projects.

Commission Kurey provided comments regarding increase in costs; potential for borrowing with today's low rates; creative funding sources; need to get the two boards together; get lead engineer on board.

Discussion ensued on capacity to do more projects; bequests; projection on ad valorem tax base increases; JMC properties and benefits; ability to pay debt vs cost of waiting; having consultant providing scenarios at the joint meeting; maintenance of roadways.

It was the consensus of the board to schedule joint meeting with the Finance board.

#### [19-0145](#)

#### Discussion of Capital Projects Communication Guide

Mr. Murphy requested that board members review the guide as submitted on their own; would be happy to answer any questions from the board.

No questions or objections from the board.

[19-0146](#)

Consideration of Mckim & Creed as lead Engineer of Record

Mr. Murphy spoke regarding the need to have a lead EOR; need to know options; staff reviewed EORs on file, conducted interviews and would like to recommend to the Commission, McKim & Creed as primary EOR.

Gayle Grady-Board Consultant- in support of recommendation.

Comments and discussion by the board regarding budget and watching costs with professional engineers.

**OTHER BUSINESS**

None to be heard

**COMMISSION ADVISOR REPORT**

Commissioner Kurey stated the Belleair Country Club would like to purchase or lease a parcel of land at corner of Bayview; upcoming resident meetings to be held for citizen input; strategic plan progressing; asked if members would continue to serve on the board; thanked the members for their service and agreeing to continue to serve.

**ADJOURNMENT**

Meeting adjourned in due form at 7:11 PM.

**Mr. Oder moved to adjourn. Seconded by Mr. Hail.**

**Aye:** 4 - Chairman Pace, Hail, White , and Oder

**Absent:** 3 - Foley, Campbell, and Flaharty

**APPROVED:**

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**Chairman**



## Legislation Details (With Text)

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**File #:** 20-0208      **Version:** 1      **Name:**  
**Type:** Action Item      **Status:** General Agenda  
**File created:** 8/26/2020      **In control:** Infrastructure Board  
**On agenda:** 8/31/2020      **Final action:**  
**Title:** Election of Officers  
**Sponsors:**  
**Indexes:**  
**Code sections:**  
**Attachments:**

Date	Ver.	Action By	Action	Result
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Legislation Details (With Text)

**File #:** 20-0213      **Version:** 1      **Name:**

**Type:** Discussion Items      **Status:** Agenda Ready

**File created:** 8/28/2020      **In control:** Infrastructure Board

**On agenda:** 8/31/2020      **Final action:**

**Title:** Discussion of Draft Reverse Osmosis Preliminary Engineering Report and Water Rate Study

**Sponsors:** JP Murphy

**Indexes:**

**Code sections:**

**Attachments:** [Town of Belleair Draft Executive Summary 8.28.2020](#)  
[Belleair RO WTP draft 8-3-20 reduced](#)

Date	Ver.	Action By	Action	Result
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**Summary**

To: Infrastructure Board  
 From: JP Murphy, Town Manager  
 Date: 8/28/2020

**Subject:**  
 Discussion of Draft Reverse Osmosis Preliminary Engineering Report and Water Rate Study

**Summary:**  
 If the Town wishes to continue with potable water production, a new Reverse Osmosis (RO) treatment plant is recommended to reduce chloride and TDS concentrations and to address ongoing operational, maintenance and safety concerns at the water treatment plant (WTP). Two reports are provided as attachments; each with executive summaries of their own, but should be considered together for purposes of discussion of future direction of the system.

Besides developing preliminary engineering requirements for the proposed RO WTP systems, the PER was developed to provide the Town with capital and operations & maintenance (O&M) costs for the proposed WTP. Also, a potential phased implementation plan was developed that focuses on using the existing WTP infrastructure to the extent possible to reduce initial capital costs. In addition, the phased approach implements new treatment processes and modifications, only as needed, to address chloride and TDS levels projected to increase over time

In conjunction, with the proposed costs of conversion to RO a water rate study and rate model were conducted to forecast require rate increases and provide benchmark data for analysis. This report summarizes the development of identified rate adjustments for water service that are considered necessary, along with other appropriate sources of funds, to meet the projected revenue requirements. Additionally, rate analysis is included to identify rates for residents as retail customers of Pinellas County.

TOWN OF  
**Belleair**

**Water Rate Study**

**Draft Executive Summary / August 28, 2020**

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August 28, 2020

Honorable Mayor and Members  
of the Town Commission  
Town of Belleair  
901 Ponce de Leon Boulevard  
Belleair, FL 33756

**Subject: Water Rate Study**

Ladies and Gentlemen:

Raftelis Financial Consultants, Inc. ("Raftelis") has completed our review of the sufficiency of the water user rates (the "Study") for the Town of Belleair, Florida (the "Town"), and has summarized the results in this report for your consideration. This report summarizes the development of identified rate adjustments for water service that are considered necessary, along with other appropriate sources of funds, to meet the projected revenue requirements (expenditure and funding needs for the utility enterprise and referred to as the "revenue requirements") for the accounting period beginning October 1 and ending September 30 (the "Fiscal Year") for Fiscal Years 2020 through 2025 (the "Forecast Period"), the initial rate evaluation period and for a long-term planning period ending Fiscal Year 2035 (the "Study Period").

The most important objective of the Study was to develop proposed utility rates that fully recover the projected water utility revenue requirements in order to maintain sound financial operations and finance the anticipated capital needs of the water system (the "System"). The Town is in the process of evaluating its long-term water supply options, which includes the construction of a new reverse osmosis ("RO") treatment plant. A primary goal of the study is to assure that the System has sufficient funds to not only fully fund the cost of providing service but also to repay the additional debt that will be issued associated with the financing of this project. Other goals and objectives considered in the Study include the following:

- The proposed rates should promote and maintain an acceptable financial position consistent with performance criteria used by credit rating agencies and the utility industry to minimize financial risk. This guideline entails the following in support of promoting a sustainable rate plan:
  - Compliance with the rate covenants in the proposed loan agreements with the Florida Department of Environmental Protection (i.e., State Revolving Fund loan program).
  - Maintenance of adequate operating and capital reserves.
  - Maintenance of ongoing capital reinvestment margins to balance equity and debt financing of capital improvements.
- The proposed rates should be based on fully recovering the identified revenue requirements of the System (i.e., full cost recovery principle).

- To the extent practical, any rate adjustments should be phased in to limit customer "rate shock" (large rate adjustments due to recapturing the effects of inflation and significant capital investment impacting the cost of providing service).
- The proposed rates should be consistent with historical rate structures as appropriate.
- The proposed rates, to the extent practical, should be comparable or competitive with those of neighboring utility systems.

Additionally, the Town is evaluating the option of decommissioning the existing water treatment plant and becoming a retail customer of Pinellas County. A preliminary evaluation of the overall estimated rate impacts of becoming Pinellas County retail water customers were analyzed and compared to the RO treatment plant option.

Following this letter, we have provided an executive summary that provides an overview of the Study results and outlines our recommendations and conclusions. The remainder of the report provides additional details regarding the rate and financial analysis conducted on behalf of the Town.

We appreciate the opportunity to be of service to the Town and would like to thank the Town staff for their valuable assistance and cooperation over the course of this Study.

We appreciate the opportunity to be of service to the Town.

Respectfully submitted,

**Raftelis Financial Consultants, Inc.**

**Robert J. Ori**

*Executive Vice President*

**Nick T. Smith, CGFM**

*Consultant*

RJO/nts

Attachments

## List of Tables

<b>Table ES-1</b>	<b>Summary of Existing and Identified Water Rates</b>
<b>Table ES-2</b>	<b>Utility System Financial Overview</b>
<b>Table ES-3</b>	<b>Comparison of Monthly Charges for Residential Water Service to Pinellas County</b>
<b>Table 1</b>	<b>Projected System Net Revenue Requirements</b>
<b>Table 2</b>	<b>Projected Operating Expenses</b>
<b>Table 3</b>	<b>Escalation Factors</b>
<b>Table 4</b>	<b>Estimated Multi-Year Capital Improvement Program and Funding Sources</b>
<b>Table 5</b>	<b>Projected Cash Balances by Fund and Interesting Earnings</b>
<b>Table 6</b>	<b>Comparison of Typical Monthly Residential Bills for Water Service</b>
<b>Table 7</b>	<b>Projected Debt Service Coverage</b>

# Executive Summary and Recommendations

## Executive Summary

The Town of Belleair's (the "Town") water utility system (the "System") provides an essential service on a continuous basis to over 1,500 water customers. Wastewater service is provided by Pinellas County (the "County"), and therefore is not discussed or evaluated in this report. The Town's utility operates in a highly regulated environment. Regulatory agencies for the utility include the Florida Department of Environmental Protection ("FDEP") and the Southwest Florida Water Management District ("SWFWMD") and permits issued through these regulatory agencies require satisfactory operating performance.

The Town's System is established as a utility enterprise fund. As such, the System should have revenues equal to the costs of the services provided, and the Town should attempt to establish rates that are always sufficient to cover the cost of operating, maintaining, repairing, and financing the System (referred to as the "revenue requirements"). According to the Governmental Accounting Standards Board:

Enterprise Funds should be used to account for operations that are financed and operated in a manner similar to private business enterprises – where the intent of the governing body is that costs of providing services to the general public on a continuing basis should be financed or recovered primarily through user charges.

General accounting policies and prudent utility management recommend that the System have revenues (financial resources) at least equal to the costs of providing services by the System.

To help ensure that water rates on a prospective basis will be sufficient to recover the cost of operating, maintaining, and repairing, the System and meeting anticipated rate covenants associated with the financing of the System capital improvements, the Town retained Raftelis Financial Consultants, Inc. ("Raftelis") to perform a review of the sufficiency of the water user rates to meet such requirements (the "Study"). The current Study includes a multi-year projection of the System revenue requirements (i.e., the projected expenditures and funding needs of the System) and the determination of the ability of the rate revenues to fund such needs (referred to as the "revenue sufficiency evaluation"). Specifically, Raftelis was tasked to: i) provide a projection of the revenue requirements for the System for the accounting period beginning October 1 and ending September 30 (the "Fiscal Year") for Fiscal Years 2020 through 2025 (the "Forecast Period"), the initial rate evaluation period, and for a long-term planning period ending Fiscal Year 2035 (the "Study Period"); and ii) provide a projection of future annual rate adjustments considered necessary to fund the projected revenue requirements through the end of the Forecast Period and estimated for the Study Period for the Town's consideration.

Based on the assumptions and analyses reflected in this report, which should be read in its entirety, it is projected that the current water rates of the System will not be sufficient to meet projected revenue requirements identified for the Study Period. Based on our studies and evaluations, additional rate adjustments are projected to be required to fully fund the identified revenue requirements of the System.

The primary reasons for the identified rate adjustments include:

1. Providing sufficient revenues to fund the capital improvement program ("CIP") of the System. As identified by the Town's consulting engineer in *the Reverse Osmosis WTP Preliminary Engineering Report ("PER")*, there exists a need to replace the existing water treatment facilities. The proposed reverse

osmosis ("RO") water treatment plant is estimated to require funding of approximately \$11.8 million if the entire project is completed at once. However, a phased approach to the capital plan was developed by the Town's consulting engineers and which results in an estimated capital funding need of approximately \$16.1 million (engineering estimates adjusted for inflation). The phased approach has been recognized in the Study to allow rates to be adjusted over time and to avoid rate shock to the extent possible. The remainder of the CIP consists of allowances for other departmental capital such as meters and other water distribution improvements. The projected funding of the capital program during the Study Period is summarized as follows:

<b>Projected Funding Sources for Multi-Year CIP [*]</b>		
Description	Amount	Percent
<b>Identified Capital Improvements</b>	<b>\$17,623,887</b>	
<b>Assumed Funding Sources:</b>		
Rate Revenues (annual operations)	\$1,487,866	8.5%
Operating Reserves	4,024,584	22.8%
State Revolving Fund ("SRF") Loans (Three loans)	12,111,437	68.7%
<b>Total Funding Sources</b>	<b>\$17,623,887</b>	<b>100.0%</b>

[\*] Amounts derived from Table 4 at the end of this report.

2. Continuing to fund the costs of operations. Operating expenses are projected to continue to increase during the Study Period due to, among other things, the following:
  - a. Continued inflationary effects on the costs of electricity, chemicals, fuel, and other major expenditures for the utility as part of the normal cost of operations;
  - b. Increased labor costs, including additional personnel anticipated to be required to operate the proposed RO water treatment plant; and
  - c. Incremental increases to operating expenses, in addition to the cost of additional personnel, related to the implementation and bringing into service the capital improvements at the proposed RO treatment plant (e.g., increased electrical costs).

The Congressional Budget Office has projected the national consumer price index ("CPI") to increase by approximately 1.8% per year during the Study Period based on projections published as of July 2020.

3. The need to maintain appropriate debt service coverage ratios, adequate operating margins, and reserves to maintain the financial condition of the System reduce the overall financial risk to the utility and the ability to repay the allocated debt of the System. A minimum debt service coverage requirement of 115% was recognized to maintain compliance with terms of an loan agreement to be entered into associated with the financing of the RO water treatment plant through low-interest loans secured with the State Revolving Fund (SRF) loan program as administered by the FDEP. Additionally, in the development of the revenue requirements, certain financial targets or benchmarks were recognized which promotes the long-term sustainability of rates. Raftelis has recognized minimum cash reserve balances of \$450,000 within the water fund based on Town policy and a target cash or reserve balances equal to 120 days of rate revenue for working capital (operating reserves)

For the Town's water system, this Study recognizes the following annual rate revenue adjustments:

<b>Summary of Recognized Annual Rate Revenue Adjustments [1]</b>		
<u>Fiscal Year</u>	<u>Water Revenue Adjustments</u>	<u>Effective Monthly Increase for Typical Residential Customer [2]</u>
For the Forecast Period		
2021	0.00%	\$0.00
2022	6.75%	4.04
2023	6.75%	4.32
2024	6.75%	4.60
2025	6.75%	4.95
For the Remainder of the Study Period		
2026	6.75%	\$5.24
2027	6.75%	5.60
2028	6.75%	5.96
2029	4.00%	3.76
2030	4.00%	3.95
2031	4.00%	4.11
2032	4.00%	4.24
2033	4.00%	4.40
2034	4.00%	4.60
2035	4.00%	4.80

[1] The presentation of the revenue sufficiency analysis and results in the report assume that the rate adjustments shown above become effective on October 1 (beginning) of each Fiscal Year.

[2] Typical monthly residential bill assumed to require a billed water flow of 10,000 gallons.

We recommend that the Town perform annual rate reviews to ensure that the user rates are still sufficient given possible changes in economic conditions, customer usage trends, regulatory requirements, etc.

The Town's existing water rates are competitive with those of neighboring Florida utilities. A comparison of bills under the Town's existing water system rates with those of surveyed neighboring utilities is summarized as follows:

<b>Residential Water Service – Monthly Billed Flow (Gallons) [1]</b>							
<u>Description</u>	<u>0</u>	<u>2,000</u>	<u>5,000</u>	<u>10,000 [4]</u>	<u>15,000</u>	<u>20,000</u>	<u>50,000</u>
Town of Belleair: [2]							
Existing FY 2020	\$12.99	\$16.89	\$27.31	\$59.91	\$92.51	\$125.11	\$353.21
Utility Survey: [2]							
Survey Average	\$14.51	\$20.89	\$30.98	\$73.43	\$118.49	\$167.86	\$542.68
Minimum	6.80	16.29	25.11	58.10	83.75	109.40	391.55
Maximum	23.16	31.14	39.66	98.16	174.88	266.93	1,064.63

[1] The detailed survey can be found in Table 12 at the end of this report.

[2] Based on a survey of 11 neighboring utilities as discussed in this report and reflect rates that are currently in effect as of July 2020; with the majority of the utilities providing rates that have a base and volumetric or flow charge). The rates reflected in the development of the utility survey have not been adjusted for any possible or approved rate adjustments anticipated for the Fiscal Year 2021 or beyond; several utilities included in the survey are anticipating an increase in rates for 2021 through the application of a price index or as identified through a rate study process.

[3] 10,000 gallons represents the average monthly usage level for the typical residential customer.

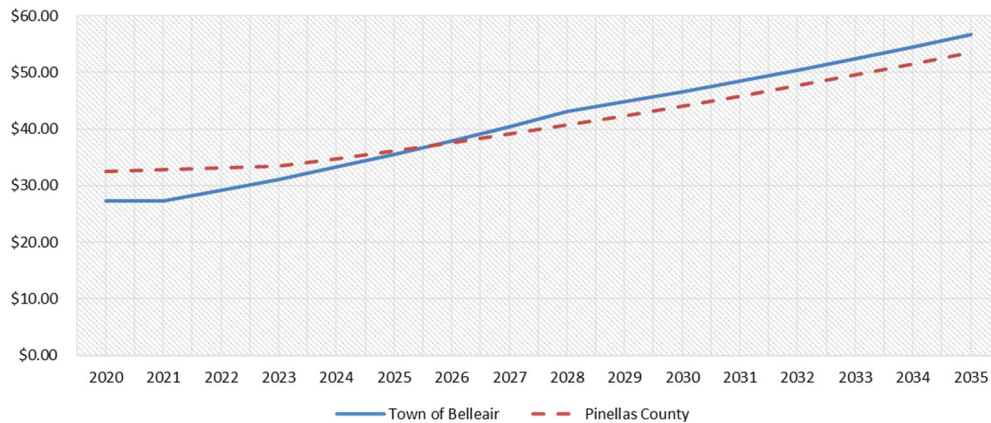
Table ES-2 at the end of this executive summary provides a graphical overview of the projected System financial results assuming the acceptance and implementation of the proposed rate adjustments by the Town (the "management dashboard"). This table indicates the following for all projected Fiscal Years, assuming the identified user rate adjustments for all Fiscal Years are implemented:

1. Maintenance of adequate debt service coverage.
2. Maintenance of adequate operating reserves (liquidity).
3. Maintenance of adequate net revenue margins necessary for debt repayment and capital reinvestment.

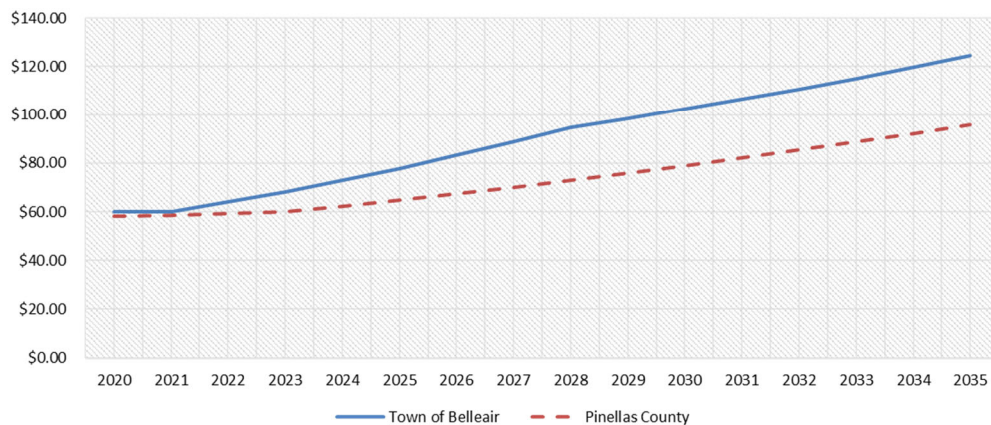
Based on the assumptions relied upon in the preparation of the financial forecast and assuming the implementation of the identified revenue adjustments and the projections as contained herein, the utility should remain in a positive financial position throughout the Study Period.

As previously mentioned, the Town is also evaluating the option of receiving retail water service from Pinellas County. Under this option the Town would no longer provide water service and existing Town water customers would become retail customers of and be charged the County's rates for service. Below are comparisons of the Town's rates, based on the identified rate adjustments, to the County's estimated rates. The comparisons represent residential water bills at 5,000 and 10,000 gallons of usage.

**Residential Bill Comparison at 5,000 Gallons**



**Residential Bill Comparison at 10,000 Gallons**



As can be seen on the charts above and based on our preliminary analysis, the Town's monthly water rates are estimated to produce bills that may be higher than the bills assumed to be incurred through the application of the County's water rates.

## Conclusions and Recommendations

Based on our studies, assumptions, considerations, and analyses as summarized herein, we are of the opinion that:

1. The Town's revenues under existing rates are not anticipated to recover the projected System revenue requirements for the Forecast Period ending Fiscal Year 2025 and for the Study Period ending 2035.
2. The Town should consider implementing the water rate adjustments identified for the Forecast Period which are anticipated to meet the projected revenue requirements of the System. The revenue requirements are based on the Town's estimated cash expenditure and funding needs and reflect the cost of operations, the financing of capital improvements, the payment of debt service on the Town's existing and anticipated utility indebtedness, and the maintenance of adequate operating reserves for the System.
3. It is projected that the Town's rates may need to be increased beyond the Forecast Period to fully fund the construction of the new RO water treatment plant based on the estimates as contained in the PER.
4. The implementation of the identified rates as reflected in the Study are projected to be adequate to meet the rate covenant requirements (e.g., debt service coverage) of the Town's outstanding and anticipated utility indebtedness.
5. The Town should perform annual rate reviews to ensure that the user rates are still sufficient given possible changes in economic conditions, the capital plan, customer usage trends, regulatory requirements, etc.



# **TABLES**

**Table ES-1**  
**Town of Belleair, Florida**  
**Water Rate Study**

**Summary of Existing and Identified Water Rates**

Line No.	Description	Existing 2020	Projected Fiscal Year Ending September 30,														
			2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035
1	Identified Rate Adjustments		0.00%	6.75%	6.75%	6.75%	6.75%	6.75%	6.75%	6.75%	6.75%	4.00%	4.00%	4.00%	4.00%	4.00%	4.00%
<b>Water System</b>																	
<u>Base Charges</u>																	
2	Residential	\$12.99	\$12.99	\$13.87	\$14.81	\$15.81	\$16.88	\$18.02	\$19.24	\$20.54	\$21.36	\$22.21	\$23.10	\$24.02	\$24.98	\$25.98	\$27.02
3	Commercial	25.98	25.98	27.73	29.60	31.60	33.73	36.01	38.44	41.03	42.67	44.38	46.16	48.01	49.93	51.93	54.01
<u>Usage Charges (per 1,000 Gallons)</u>																	
<u>Residential</u>																	
4	Tier 1 (1,000 - 4,000)	\$1.95	\$1.95	\$2.08	\$2.22	\$2.37	\$2.53	\$2.70	\$2.88	\$3.07	\$3.19	\$3.32	\$3.45	\$3.59	\$3.73	\$3.88	\$4.04
5	Tier 2 (4,001 - 25,000)	6.52	6.52	6.96	7.43	7.93	8.47	9.04	9.65	10.30	10.71	11.14	11.59	12.05	12.53	13.03	13.55
6	Tier 3 (Above 25,000)	7.82	7.82	8.35	8.91	9.51	10.15	10.84	11.57	12.35	12.84	13.35	13.88	14.44	15.02	15.62	16.24
7	Multi-Meter (per additional meter)	3.48	3.48	3.71	3.96	4.23	4.52	4.83	5.16	5.51	5.73	5.96	6.20	6.45	6.71	6.98	7.26
<u>Commercial</u>																	
8	Tier 1 (1,000 - 25,000)	\$6.52	\$6.52	\$6.96	\$7.43	\$7.93	\$8.47	\$9.04	\$9.65	\$10.30	\$10.71	\$11.14	\$11.59	\$12.05	\$12.53	\$13.03	\$13.55
9	Tier 2 (Above 25,000)	7.82	7.82	8.35	8.91	9.51	10.15	10.84	11.57	12.35	12.84	13.35	13.88	14.44	15.02	15.62	16.24
10	Multi-Meter (per additional meter)	3.48	3.48	3.71	3.96	4.23	4.52	4.83	5.16	5.51	5.73	5.96	6.20	6.45	6.71	6.98	7.26



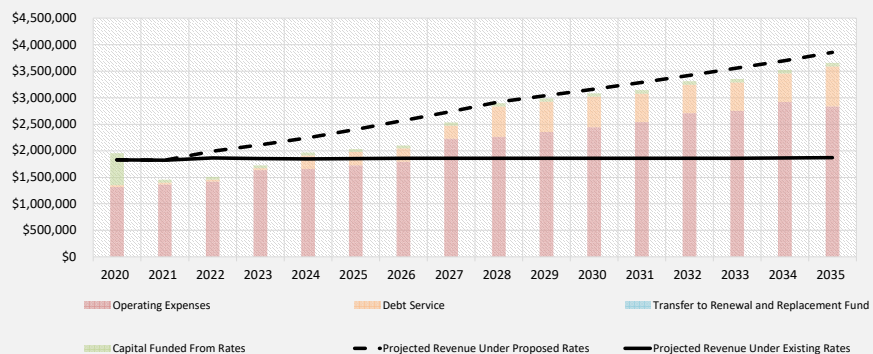
**Table ES-2**  
**Town of Belleair, Florida**  
**Water Rate Study**



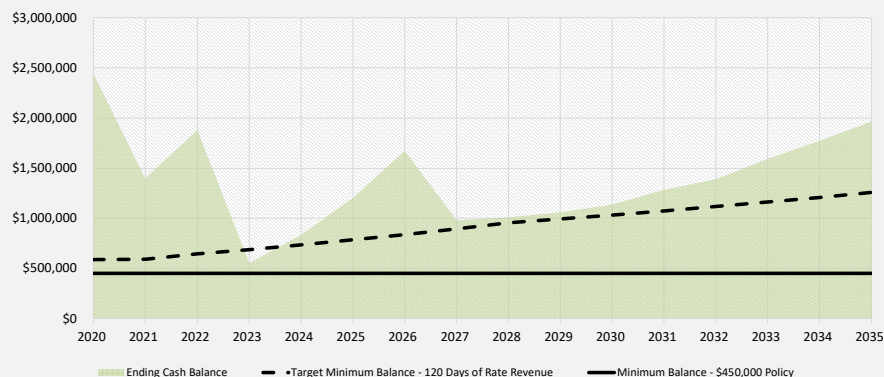
**Utility System Financial Overview**

	Fiscal Year Ending September 30,															
	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035
<b>CURRENT YEAR REVENUE ADJUSTMENTS</b>																
Water System Rate Revenue Adjustments	0.00%	0.00%	6.75%	6.75%	6.75%	6.75%	6.75%	6.75%	6.75%	4.00%	4.00%	4.00%	4.00%	4.00%	4.00%	4.00%
Effective Months	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12
<b>Residential Rates</b>																
Base Charge	\$12.99	\$12.99	\$13.87	\$14.81	\$15.81	\$16.88	\$18.02	\$19.24	\$20.54	\$21.36	\$22.21	\$23.10	\$24.02	\$24.98	\$25.98	\$24.98
Tier 1 Usage Charge	\$1.95	\$1.95	\$2.08	\$2.22	\$2.37	\$2.53	\$2.70	\$2.88	\$3.07	\$3.19	\$3.32	\$3.45	\$3.59	\$3.73	\$3.88	\$3.73
Tier 2 Usage Charge	\$6.52	\$6.52	\$6.96	\$7.43	\$7.93	\$8.47	\$9.04	\$9.65	\$10.30	\$10.71	\$11.14	\$11.59	\$12.05	\$12.53	\$13.03	\$12.53
Tier 3 Usage Charge	\$7.82	\$7.82	\$8.35	\$8.91	\$9.51	\$10.15	\$10.84	\$11.57	\$12.35	\$12.84	\$13.35	\$13.88	\$14.44	\$15.02	\$15.62	\$15.02
Average Residential Bill (5,000 Gallons)	\$27.31	\$27.31	\$29.15	\$31.12	\$33.22	\$35.47	\$37.86	\$40.41	\$43.12	\$44.83	\$46.63	\$48.49	\$50.43	\$52.43	\$54.53	\$52.43
<b>Unrestricted Cash Position - End of Year -Working Capital</b>	\$2,441,457	\$1,398,581	\$1,881,941	\$553,165	\$835,456	\$1,206,067	\$1,674,648	\$982,673	\$1,009,126	\$1,060,724	\$1,138,559	\$1,283,510	\$1,391,547	\$1,594,371	\$1,773,478	\$1,967,601
Debt Service Coverage- Senior	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Debt Service Coverage- Junior	0%	0%	0%	0%	278%	321%	368%	244%	123%	128%	133%	139%	132%	150%	146%	135%
Debt Service Coverage- All-in	1292%	1149%	1437%	1194%	234%	270%	309%	205%	115%	119%	124%	139%	132%	150%	146%	135%

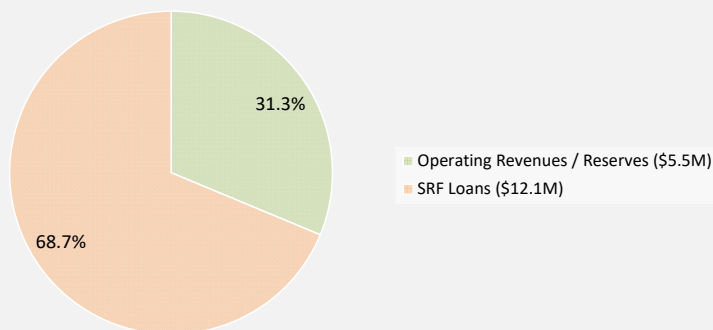
**Figure ES-1. Revenue Requirements**



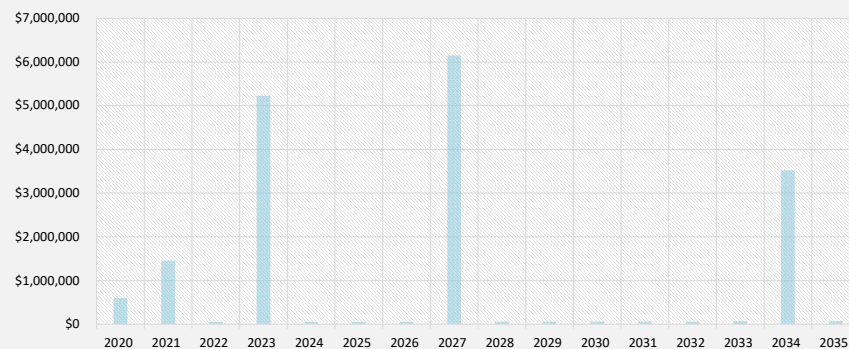
**Figure ES-2. Ending Cash Balance - Operating Fund**



**Figure ES-3. Capital Improvement Program Projected Funding Sources (Total CIP = \$17,623,887)**



**Figure ES-4. Capital Improvement Program Spending Curve**



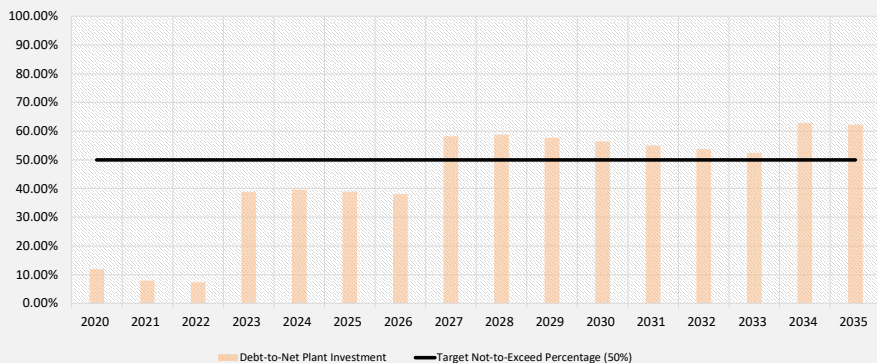


**Table ES-2**  
**Town of Belleair, Florida**  
**Water Rate Study**  
**Utility System Financial Overview**

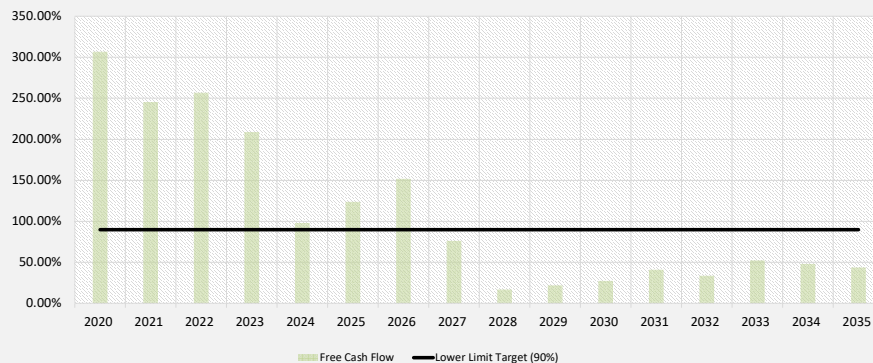


	Fiscal Year Ending September 30,																
	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035	
<b>CURRENT YEAR REVENUE ADJUSTMENTS</b>																	
Water System Rate Revenue Adjustments	0.00%	0.00%	6.75%	6.75%	6.75%	6.75%	6.75%	6.75%	6.75%	4.00%	4.00%	4.00%	4.00%	4.00%	4.00%	4.00%	
Effective Months	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	
<b>Residential Rates</b>																	
Base Charge	\$12.99	\$12.99	\$13.87	\$14.81	\$15.81	\$16.88	\$18.02	\$19.24	\$20.54	\$21.36	\$22.21	\$23.10	\$24.02	\$24.98	\$25.98	\$24.98	
Tier 1 Usage Charge	\$1.95	\$1.95	\$2.08	\$2.22	\$2.37	\$2.53	\$2.70	\$2.88	\$3.07	\$3.19	\$3.32	\$3.45	\$3.59	\$3.73	\$3.88	\$3.73	
Tier 2 Usage Charge	\$6.52	\$6.52	\$6.96	\$7.43	\$7.93	\$8.47	\$9.04	\$9.65	\$10.30	\$10.71	\$11.14	\$11.59	\$12.05	\$12.53	\$13.03	\$12.53	
Tier 3 Usage Charge	\$7.82	\$7.82	\$8.35	\$8.91	\$9.51	\$10.15	\$10.84	\$11.57	\$12.35	\$12.84	\$13.35	\$13.88	\$14.44	\$15.02	\$15.62	\$15.02	
Average Residential Bill (5,000 Gallons)	\$27.31	\$27.31	\$29.15	\$31.12	\$33.22	\$35.47	\$37.86	\$40.41	\$43.12	\$44.83	\$46.63	\$48.49	\$50.43	\$52.43	\$54.53	\$52.43	
<b>Unrestricted Cash Position - End of Year -Working Capital</b>	\$2,441,457	\$1,398,581	\$1,881,941	\$553,165	\$835,456	\$1,206,067	\$1,674,648	\$982,673	\$1,009,126	\$1,060,724	\$1,138,559	\$1,283,510	\$1,391,547	\$1,594,371	\$1,773,478	\$1,967,601	
Debt Service Coverage- Senior	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	
Debt Service Coverage- Junior	0%	0%	0%	0%	278%	321%	368%	244%	123%	128%	133%	139%	132%	150%	146%	135%	
Debt Service Coverage- All-in	1292%	1149%	1437%	1194%	234%	270%	309%	205%	115%	119%	124%	139%	132%	150%	146%	135%	

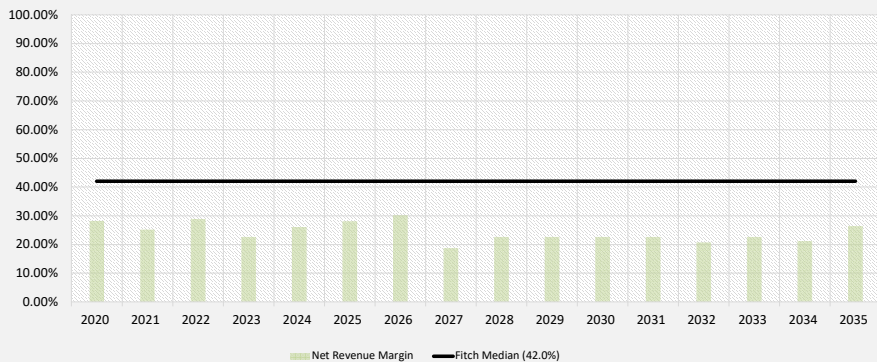
**Figure ES-5. Debt to Net Plant Investment**



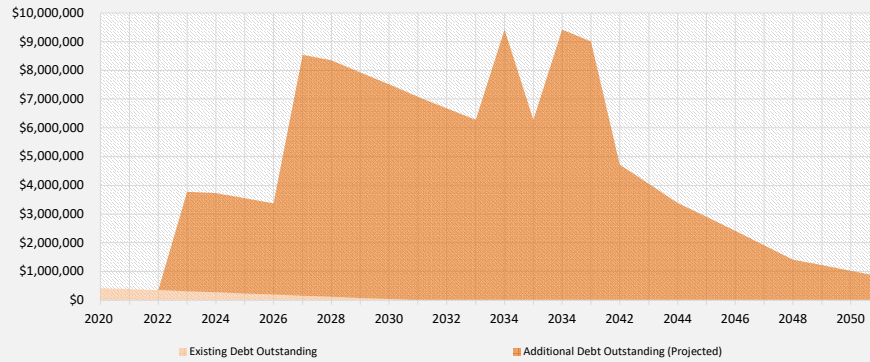
**Figure ES-6. Free Cash Flow to Depreciation**



**Figure ES-7. Net Revenue Margin**



**Figure ES-8. Total Existing and Projected Debt Outstanding - Start of Year**



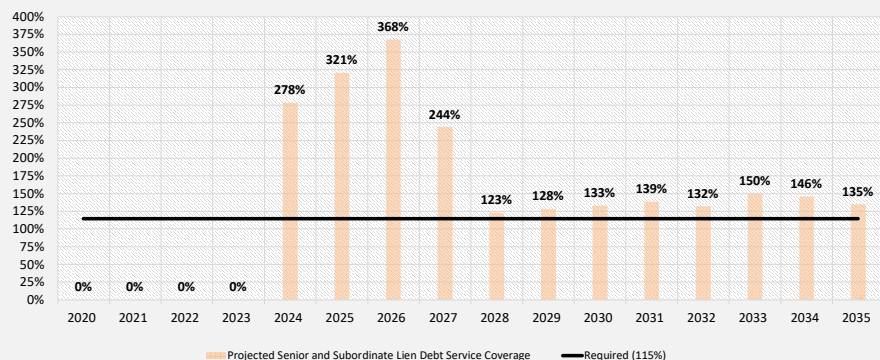


**Table ES-2**  
**Town of Belleair, Florida**  
**Water Rate Study**  
**Utility System Financial Overview**

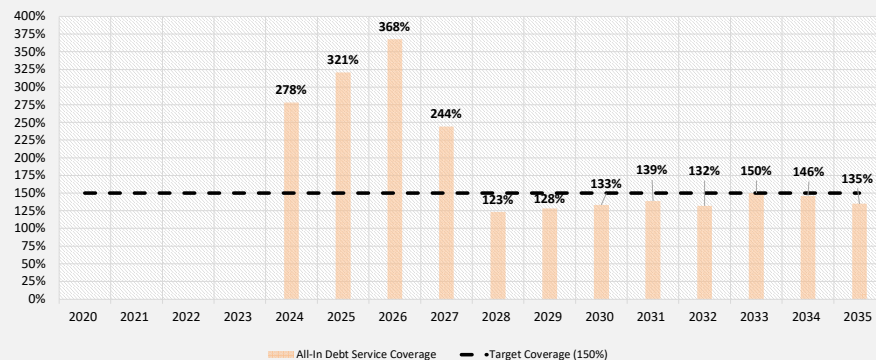


	2020	2021	2022	2023	2024	2025	2026	Fiscal Year Ending September 30,		2029	2030	2031	2032	2033	2034	2035
								2027	2028							
<b>CURRENT YEAR REVENUE ADJUSTMENTS</b>																
Water System Rate Revenue Adjustments	0.00%	0.00%	6.75%	6.75%	6.75%	6.75%	6.75%	6.75%	6.75%	4.00%	4.00%	4.00%	4.00%	4.00%	4.00%	4.00%
Effective Months	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12
<b>Residential Rates</b>																
Base Charge	\$12.99	\$12.99	\$13.87	\$14.81	\$15.81	\$16.88	\$18.02	\$19.24	\$20.54	\$21.36	\$22.21	\$23.10	\$24.02	\$24.98	\$25.98	\$24.98
Tier 1 Usage Charge	\$1.95	\$1.95	\$2.08	\$2.22	\$2.37	\$2.53	\$2.70	\$2.88	\$3.07	\$3.19	\$3.32	\$3.45	\$3.59	\$3.73	\$3.88	\$3.73
Tier 2 Usage Charge	\$6.52	\$6.52	\$6.96	\$7.43	\$7.93	\$8.47	\$9.04	\$9.65	\$10.30	\$10.71	\$11.14	\$11.59	\$12.05	\$12.53	\$13.03	\$12.53
Tier 3 Usage Charge	\$7.82	\$7.82	\$8.35	\$8.91	\$9.51	\$10.15	\$10.84	\$11.57	\$12.35	\$12.84	\$13.35	\$13.88	\$14.44	\$15.02	\$15.62	\$15.02
Average Residential Bill (5,000 Gallons)	\$27.31	\$27.31	\$29.15	\$31.12	\$33.22	\$35.47	\$37.86	\$40.41	\$43.12	\$44.83	\$46.63	\$48.49	\$50.43	\$52.43	\$54.53	\$52.43
<b>Unrestricted Cash Position - End of Year -Working Capital</b>	\$2,441,457	\$1,398,581	\$1,881,941	\$553,165	\$835,456	\$1,206,067	\$1,674,648	\$982,673	\$1,009,126	\$1,060,724	\$1,138,559	\$1,283,510	\$1,391,547	\$1,594,371	\$1,773,478	\$1,967,601
<b>Debt Service Coverage- Senior</b>	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
<b>Debt Service Coverage- Junior</b>	0%	0%	0%	0%	278%	321%	368%	244%	123%	128%	133%	139%	132%	150%	146%	135%
<b>Debt Service Coverage- All-in</b>	1292%	1149%	1437%	1194%	234%	270%	309%	205%	115%	119%	124%	139%	132%	150%	146%	135%

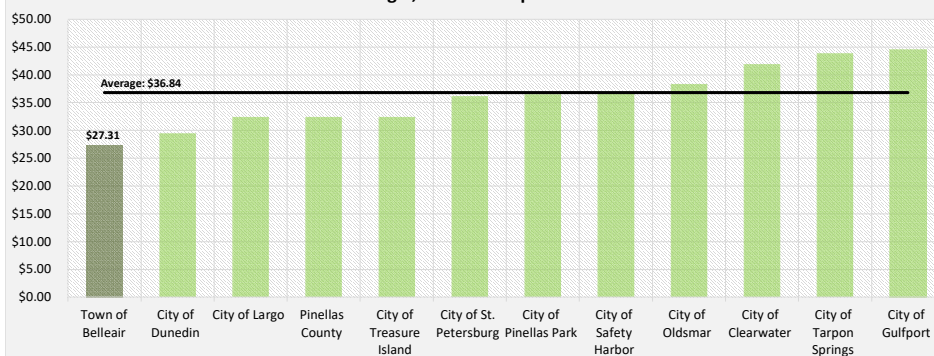
**Figure ES-9. Projected Subordinate Lien Debt Service Coverage**



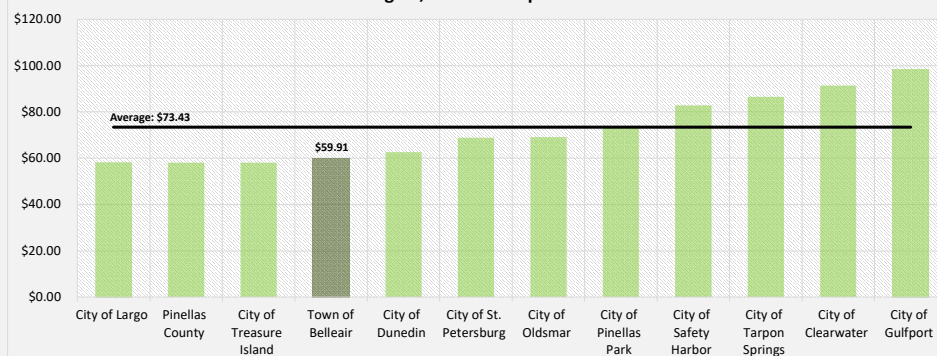
**Figure ES-10. Projected All-In Debt Service Coverage**



**Figure ES-11. Comparison of Monthly Charges for Residential Water Service for Customers Using 5,000 Gallons per Month**



**Figure ES-12. Comparison of Monthly Charges for Residential Water Service for Customers Using 10,000 Gallons per Month**



**Table ES-3**

**Town of Belleair, Florida  
Water Rate Study**

**Comparison of Typical Monthly Residential Bills for Water Service to Pinellas County**

		Residential Service for a 5/8" or 3/4" Meter										
Line No.	Description	0 Gallons	2,000 Gallons	4,000 Gallons	5,000 Gallons	7,000 Gallons	10,000 Gallons	15,000 Gallons	20,000 Gallons	50,000 Gallons	75,000 Gallons	100,000 Gallons
<b>Town of Belleair</b>												
1	2020	\$12.99	\$16.89	\$20.79	\$27.31	\$40.35	\$59.91	\$92.51	\$125.11	\$353.21	\$548.71	\$744.21
2	2021	12.99	16.89	20.79	27.31	40.35	59.91	92.51	125.11	353.21	548.71	744.21
3	2022	13.87	18.03	22.19	29.15	43.07	63.95	98.75	133.55	377.10	585.85	794.60
4	2023	14.81	19.25	23.69	31.12	45.98	68.27	105.42	142.57	402.47	625.22	847.97
5	2024	15.81	20.55	25.29	33.22	49.08	72.87	112.52	152.17	429.57	667.32	905.07
6	2025	16.88	21.94	27.00	35.47	52.41	77.82	120.17	162.52	458.62	712.37	966.12
7	2026	18.02	23.42	28.82	37.86	55.94	83.06	128.26	173.46	489.66	760.66	1,031.66
8	2027	19.24	25.00	30.76	40.41	59.71	88.66	136.91	185.16	522.66	811.91	1,101.16
9	2028	20.54	26.68	32.82	43.12	63.72	94.62	146.12	197.62	557.87	866.62	1,175.37
10	2029	21.36	27.74	34.12	44.83	66.25	98.38	151.93	205.48	580.03	901.03	1,222.03
11	2030	22.21	28.85	35.49	46.63	68.91	102.33	158.03	213.73	603.18	936.93	1,270.68
12	2031	23.10	30.00	36.90	48.49	71.67	106.44	164.39	222.34	627.29	974.29	1,321.29
13	2032	24.02	31.20	38.38	50.43	74.53	110.68	170.93	231.18	652.43	1,013.43	1,374.43
14	2033	24.98	32.44	39.90	52.43	77.49	115.08	177.73	240.38	678.53	1,054.03	1,429.53
15	2034	25.98	33.74	41.50	54.53	80.59	119.68	184.83	249.98	705.63	1,096.13	1,486.63
16	2035	27.02	35.10	43.18	56.73	83.83	124.48	192.23	259.98	733.73	1,139.73	1,545.73
<b>Pinellas County</b>												
17	2020	6.80	17.06	27.32	32.45	42.71	58.10	83.75	109.40	263.30	391.55	519.80
18	2021	6.87	17.23	27.59	32.77	43.13	58.67	84.57	110.47	265.87	395.37	524.87
19	2022	6.94	17.40	27.86	33.09	43.55	59.24	85.39	111.54	268.44	399.19	529.94
20	2023	7.01	17.57	28.13	33.41	43.97	59.81	86.21	112.61	271.01	403.01	535.01
21	2024	7.29	18.27	29.25	34.74	45.72	62.19	89.64	117.09	281.79	419.04	556.29
22	2025	7.58	19.00	30.42	36.13	47.55	64.68	93.23	121.78	293.08	435.83	578.58
23	2026	7.88	19.76	31.64	37.58	49.46	67.28	96.98	126.68	304.88	453.38	601.88
24	2027	8.20	20.56	32.92	39.10	51.46	70.00	100.90	131.80	317.20	471.70	626.20
25	2028	8.53	21.39	34.25	40.68	53.54	72.83	104.98	137.13	330.03	490.78	651.53
26	2029	8.87	22.25	35.63	42.32	55.70	75.77	109.22	142.67	343.37	510.62	677.87
27	2030	9.22	23.14	37.06	44.02	57.94	78.82	113.62	148.42	357.22	531.22	705.22
28	2031	9.59	24.07	38.55	45.79	60.27	81.99	118.19	154.39	371.59	552.59	733.59
29	2032	9.97	25.03	40.09	47.62	62.68	85.27	122.92	160.57	386.47	574.72	762.97
30	2033	10.37	26.03	41.69	49.52	65.18	88.67	127.82	166.97	401.87	597.62	793.37
31	2034	10.78	27.06	43.34	51.48	67.76	92.18	132.88	173.58	417.78	621.28	824.78
32	2035	11.21	28.15	45.09	53.56	70.50	95.91	138.26	180.61	434.71	646.46	858.21

**Table 1**  
**Town of Belleair, Florida**  
**Water Rate Study**

**Projected System Net Revenue Requirements**

Line No.	Description	Fiscal Year Ending September 30,															
		2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035
Operating Expenses:																	
1	Total Operating Expenses	\$1,314,948	\$1,363,157	\$1,416,031	\$1,634,513	\$1,661,760	\$1,726,868	\$1,794,731	\$2,226,833	\$2,261,453	\$2,351,595	\$2,445,649	\$2,543,763	\$2,711,890	\$2,753,101	\$2,919,360	\$2,836,110
Other Revenue Requirements:																	
Debt Service																	
SRF Loans																	
2	2023 SRF Loan Issue	\$0	\$0	\$0	\$0	\$210,779	\$210,779	\$210,779	\$210,779	\$210,779	\$210,779	\$210,779	\$210,779	\$210,779	\$210,779	\$210,779	\$210,779
3	2027 SRF Loan Issue	0	0	0	0	0	0	0	0	326,186	326,186	326,186	326,186	326,186	326,186	326,186	326,186
4	2034 SRF Loan Issue	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	216,887
Subordinate Lien																	
5	Interfund Loan - General Fund	40,000	40,000	40,000	40,000	40,000	40,000	40,000	40,000	40,000	40,000	40,000	0	0	0	0	0
6	Total Debt Service	\$40,000	\$40,000	\$40,000	\$40,000	\$250,779	\$250,779	\$250,779	\$250,779	\$576,966	\$576,966	\$576,966	\$536,966	\$536,966	\$536,966	\$536,966	\$753,853
Other Miscellaneous Revenue Requirements:																	
7	Departmental Capital Funded from Rate Revenues	\$603,066	\$50,450	\$51,300	\$52,400	\$53,550	\$54,800	\$56,050	\$57,350	\$58,650	\$60,000	\$61,400	\$62,800	\$64,250	\$65,750	\$67,250	\$68,800
8	Total Other Miscellaneous Revenue Requirements	\$603,066	\$50,450	\$51,300	\$52,400	\$53,550	\$54,800	\$56,050	\$57,350	\$58,650	\$60,000	\$61,400	\$62,800	\$64,250	\$65,750	\$67,250	\$68,800
9	Gross Revenue Requirements	\$1,958,014	\$1,453,607	\$1,507,331	\$1,726,913	\$1,966,089	\$2,032,447	\$2,101,561	\$2,534,962	\$2,897,069	\$2,988,561	\$3,084,015	\$3,143,529	\$3,313,106	\$3,355,816	\$3,523,576	\$3,658,763
Less Other Income and Funds from Other Sources:																	
10	Unrestricted Interest Earnings	\$32,200	\$21,800	\$17,600	\$12,900	\$7,700	\$11,200	\$15,900	\$16,100	\$14,100	\$14,400	\$15,100	\$15,900	\$17,700	\$19,100	\$21,600	\$24,600
11	Other Revenues	6,600	2,511	7,694	1,000	1,000	1,000	2,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000
12	Total Other Income	\$38,800	\$24,311	\$25,294	\$13,900	\$8,700	\$12,200	\$17,900	\$17,100	\$15,100	\$15,400	\$16,100	\$16,900	\$18,700	\$20,100	\$22,600	\$25,600
13	Total Net Revenue Requirements	\$1,919,214	\$1,429,296	\$1,482,036	\$1,713,013	\$1,957,389	\$2,020,247	\$2,083,661	\$2,517,862	\$2,881,969	\$2,973,161	\$3,067,915	\$3,126,629	\$3,294,406	\$3,335,716	\$3,500,976	\$3,633,163
14	Revenue Under Existing Rates	\$1,793,006	\$1,798,550	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121
15	Prior Year Rate Adjustments	0	0	0	124,276	256,940	398,559	549,737	711,120	883,397	1,067,302	1,183,639	1,304,629	1,430,459	1,561,322	1,697,420	1,838,961
16	Total Applicable Rate Revenue	\$1,793,006	\$1,798,550	\$1,841,121	\$1,965,397	\$2,098,061	\$2,239,680	\$2,390,858	\$2,552,241	\$2,724,518	\$2,908,423	\$3,024,760	\$3,145,750	\$3,271,580	\$3,402,443	\$3,538,541	\$3,680,082
17	Rate Adjustments	0.00%	0.00%	6.75%	6.75%	6.75%	6.75%	6.75%	6.75%	6.75%	4.00%	4.00%	4.00%	4.00%	4.00%	4.00%	4.00%
18	Effective Months	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12
19	Percent of Current Year Effective	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
20	Revenue From Current Rate Adjustment	\$0	\$0	\$124,276	\$132,664	\$141,619	\$151,178	\$161,383	\$172,276	\$183,905	\$116,337	\$120,990	\$125,830	\$130,863	\$136,098	\$141,542	\$147,203
21	Total Rate Revenue	\$1,793,006	\$1,798,550	\$1,965,397	\$2,098,061	\$2,239,680	\$2,390,858	\$2,552,241	\$2,724,518	\$2,908,423	\$3,024,760	\$3,145,750	\$3,271,580	\$3,402,443	\$3,538,541	\$3,680,082	\$3,827,286
Revenue Surplus/(Deficiency) Under Proposed Rates																	
22	Amount	(\$126,208)	\$369,254	\$483,360	\$385,048	\$282,291	\$370,611	\$468,581	\$206,655	\$26,453	\$51,598	\$77,835	\$144,951	\$108,037	\$202,824	\$179,107	\$194,123
23	% Rate Increase	(7.04%)	20.53%	24.59%	18.35%	12.60%	15.50%	18.36%	7.59%	0.91%	1.71%	2.47%	4.43%	3.18%	5.73%	4.87%	5.07%







**Table 2**  
**Town of Belleair, Florida**  
**Water Rate Study**  
**Projected Operating Expenses**

Line No.	Code	Description	Adjusted 2020	Escalation Reference	Fiscal Year Ending September 30,														
					2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035
121	54315	Energy-Street Light	0	Electric	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
122	54400	Equip. Rental	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
123	54614	Drainage	57,000	Inflation	57,513	58,491	59,719	61,033	62,437	63,873	65,342	66,845	68,382	69,955	71,564	73,210	74,894	76,616	74,894
124	54620	Maintenance - Vehicle	1,000	Repair	1,030	1,061	1,093	1,126	1,159	1,194	1,230	1,267	1,305	1,344	1,384	1,426	1,469	1,513	1,469
125	54630	Maintenance - Building	0	Repair	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
126	54670	Maintenance - Equipment	0	Repair	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
127	54900	Ordinance Codes	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
128	55100	Office Supplies	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
129	55210	Operating Supplies	2,000	Inflation	2,018	2,052	2,095	2,142	2,191	2,241	2,293	2,345	2,399	2,455	2,511	2,569	2,628	2,688	2,628
130	55213	Meter Replacement	0	Repair	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
131	55214	Meter Replacement	0	Repair	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
132	55220	Gasoline & Oil	0	Gas	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
133	55221	Tools	550	Inflation	555	564	576	589	602	616	630	645	660	675	691	706	723	739	723
134	55230	Chemicals	0	Chemicals	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
135	55240	Uniforms	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
136	55260	Protective Clothing	700	Inflation	706	718	733	750	767	784	802	821	840	859	879	899	920	941	920
137	55410	Memberships	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
138	55420	Training, Aids	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
139	56405	Computer System	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
140	57301	Miscellaneous	8,000	Inflation	8,072	8,209	8,382	8,566	8,763	8,965	9,171	9,382	9,597	9,818	10,044	10,275	10,511	10,753	10,511
141	59200	Repay Loan to General Fund	0	Eliminate	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
142		Subtotal	\$108,750		\$109,750	\$111,629	\$113,983	\$116,499	\$119,186	\$121,936	\$124,749	\$127,626	\$130,571	\$133,583	\$136,665	\$139,818	\$143,044	\$146,344	\$143,044
143	58101	Capital Purchase	\$0	Eliminate	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
144	59900	Depreciation	0	Eliminate	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
145		Subtotal	\$0		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
146	58001	Transfer of Reserves	\$0	Eliminate	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
147	59904	Support Service Fees	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
148	59906	Admin Fees	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
149		Subtotal	\$0		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
150		Total Meter Management	\$331,000		\$342,144	\$354,660	\$368,171	\$382,391	\$397,358	\$412,993	\$429,330	\$446,405	\$464,255	\$482,920	\$502,442	\$522,866	\$544,238	\$566,608	\$544,238
<b>Distribution - 640</b>																			
151	51200	Personnel Salaries	\$99,750	Labor	\$103,740	\$107,890	\$112,205	\$116,693	\$121,361	\$126,216	\$131,264	\$136,515	\$141,975	\$147,654	\$153,561	\$159,703	\$166,091	\$172,735	\$166,091
152	51201	Part-time Salaries	4,500	Labor	4,680	4,867	5,062	5,264	5,475	5,694	5,922	6,159	6,405	6,661	6,928	7,205	7,493	7,793	7,493
153	51210	Unused Medical	0	Labor	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
154	51400	Overtime	0	Labor	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
155	51500	Sick Leave	1,150	Labor	1,196	1,244	1,294	1,345	1,399	1,455	1,513	1,574	1,637	1,702	1,770	1,841	1,915	1,991	1,915
156	52100	FICA	7,800	Labor	8,112	8,436	8,774	9,125	9,490	9,869	10,264	10,675	11,102	11,546	12,008	12,488	12,988	13,507	12,988
157	52200	Retirement - 401K General Pension	8,950	Labor	9,308	9,680	10,068	10,470	10,889	11,325	11,778	12,249	12,739	13,248	13,778	14,329	14,902	15,499	14,902
158	52300	Life/Hosp. Insurance	22,500	MedIns	24,075	25,760	27,563	29,493	31,557	33,766	36,130	38,659	41,365	44,261	47,359	50,674	54,222	58,017	54,222
159	52301	Medical Benefit	2,650	MedIns	2,836	3,034	3,246	3,474	3,717	3,977	4,255	4,553	4,872	5,213	5,578	5,968	6,386	6,833	6,386
160	53100	Physical Exams	0	Labor	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
161	AddPer	Additional Personnel	0	Calculated	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
162		Subtotal	\$147,300		\$153,947	\$160,912	\$168,212	\$175,865	\$183,888	\$192,302	\$201,126	\$210,383	\$220,095	\$230,286	\$240,981	\$252,209	\$263,996	\$276,374	\$263,996
163	53151	Operating Professional Services	\$0	Inflation	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
164	54000	Travel & Per Diem	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
165	54100	Telephone	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
166	54200	Postage	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
167	54300	Electricity	0	Electric	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
168	54301	Water	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
169	54302	Sanitation	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
170	54303	Sewer	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
171	54315	Energy-Street Light	18,000	WatProdInfl	18,204	18,703	19,096	19,516	19,965	20,424	20,894	21,374	21,866	22,369	22,883	23,410	23,948	24,499	23,948
172	54400	Equip. Rental	2,750	Inflation	2,775	2,822	2,881	2,945	3,012	3,082	3,152	3,225	3,299	3,375	3,453	3,532	3,613	3,696	3,613
173	54614	Drainage	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
174	54620	Maintenance - Vehicle	1,000	Repair	1,030	1,061	1,093	1,126	1,159	1,194	1,230	1,267	1,305	1,344	1,384	1,426	1,469	1,513	1,469
175	54630	Maintenance - Building	0	Repair	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
176	54670	Maintenance - Equipment	10,000	Repair	10,300	10,609	10,927	11,255	11,593	11,941	12,299	12,668	13,048	13,439	13,842	14,258	14,685	15,126	14,685
177	54900	Ordinance Codes	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
178	55100	Office Supplies	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
179	55210	Operating Supplies	0	Inflation	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
180	55213	Meter Replacement	0	Repair	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
181	55214	Meter Replacement	0	Repair	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
182	55220	Gasoline & Oil	0	Gas	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
183	55221	Tools	3,000	Inflation	3,027	3,078	3,143	3,212	3,286	3,362	3,439	3,518	3,599	3,682	3,767	3,853	3,942	4,032	3,942



**Table 2**  
**Town of Belleair, Florida**  
**Water Rate Study**  
Projected Operating Expenses

Line No.	Code	Description	Adjusted 2020	Escalation Reference	Fiscal Year Ending September 30,														
					2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035
243	58101	Capital	\$0	Eliminate	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
244	59900	Capital Purchase Depreciation	0	Eliminate	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
245		Subtotal	\$0		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
246	58001	Fees	\$0	Eliminate	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
247	59904	Transfer of Reserves	275,300	Labor	286,312	297,764	309,675	322,062	334,945	348,342	362,276	376,767	391,838	407,511	423,812	440,764	458,395	476,731	458,395
248	59906	Support Service Fees	88,750	Labor	92,300	95,992	99,832	103,825	107,978	112,297	116,789	121,461	126,319	131,372	136,627	142,092	147,775	153,686	147,775
249		Admin Fees																	
249		Subtotal	\$364,050		\$378,612	\$393,756	\$409,507	\$425,887	\$442,922	\$460,639	\$479,065	\$498,228	\$518,157	\$538,883	\$560,438	\$582,856	\$606,170	\$630,417	\$606,170
250		Total Employee Administration	\$581,861		\$603,371	\$626,564	\$650,182	\$674,774	\$700,382	\$727,005	\$754,684	\$783,463	\$813,388	\$844,506	\$876,869	\$910,526	\$945,533	\$981,947	\$945,533
251		<b>Other Adjustments</b>																	
251		Contingency	\$12,975	Calculated	\$13,452	\$13,971	\$15,785	\$16,398	\$17,039	\$17,706	\$21,475	\$22,328	\$23,218	\$24,146	\$25,115	\$26,776	\$27,181	\$28,478	\$27,996
252		Bad Debt	4,483	Calculated	4,496	4,913	5,245	5,599	5,977	6,381	6,811	7,271	7,562	7,864	8,179	8,506	8,846	9,200	9,568
253		Incremental Operating Expenses	0	Calculated	0	0	127,276	132,006	136,939	142,051	276,888	288,026	299,640	311,789	324,465	402,803	351,651	385,647	414,782
254		Other Debt Issuance Expenses	0	Calculated	0	0	35,000	0	0	0	52,000	0	0	0	0	0	0	35,000	0
255		Subtotal	\$17,457		\$17,948	\$18,885	\$183,306	\$154,003	\$159,954	\$166,138	\$357,174	\$317,625	\$330,419	\$343,799	\$357,758	\$438,085	\$387,679	\$458,325	\$452,346
256		<b>TOTAL UTILITY SYSTEM</b>	\$1,314,948		\$1,363,157	\$1,416,031	\$1,634,513	\$1,661,760	\$1,726,868	\$1,794,731	\$2,227,739	\$2,262,382	\$2,352,547	\$2,446,625	\$2,544,763	\$2,712,915	\$2,754,151	\$2,920,437	\$2,837,160

**Table 3**  
**Town of Belleair, Florida**  
**Water Rate Study**  
Escalation Factors

Line No.	Description	Reference	Fiscal Year Ending September 30,														
			2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035
<b>Operating Escalation Factors</b>																	
1	Constant Factor	Constant	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
2	Inflation (CPI Price Index) [*]	Inflation	1.0090	1.0170	1.0210	1.0220	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230
3	Labor	Labor	1.0400	1.0400	1.0400	1.0400	1.0400	1.0400	1.0400	1.0400	1.0400	1.0400	1.0400	1.0400	1.0400	1.0400	1.0400
4	Repair and Maintenance	Repair	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300
5	Insurance - Medical	MedIns	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700	1.0700
6	Insurance - General	GenIns	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500
7	Electricity Commodity	Electric	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250
8	Electricity - Water Treatment	ElecTreatW	1.0274	1.0355	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250	1.0250
9	Gas and Fuel	Gas	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500
10	Chemicals Commodity	Chemicals	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500
11	Chemicals - Water Treatment	ChemTreatW	1.0524	1.0608	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500	1.0500
12	Elimination Factor	Eliminate	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
13	Customer Growth	WatCust	1.0052	1.0232	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
14	Change in Rate of Growth	WatUnit	0.2698	4.4313	0.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
15	Customer Growth + Inflation CPI	WatCustInfl	1.0143	1.0406	1.0210	1.0220	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230
16	Production Growth + Inflation CPI	WatProdInfl	1.0113	1.0274	1.0210	1.0220	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230	1.0230
17	Sales Revenues	WatRev	1.0031	1.0237	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
18	Engineering News Record Index	ENR	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300	1.0300

**Footnotes:**

[\*] Estimates based on projections contained in "The Budget and Economic Outlook: An Update" published by the Congressional Budget Office in July 2020.

**Table 4**  
**Town of Belleair, Florida**  
**Water Rate Study**  
**Estimated Multi-Year Capital Improvement Program and Funding Source**

Line No.	Description	Escalation Factor	Funding Source	Projected Fiscal Year Ending September 30,														Total		
				2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033		2034	2035
<b>CAPITAL IMPROVEMENT PROGRAM</b>																				
<b>System Improvements</b>																				
<b>Phase 1 - Clearwell Roof Rehab and Additional Well</b>																				
1	Mobilization/Demobilization	ENR	Operating	\$0	\$20,600	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$20,600	
2	Instrumentation	ENR	Operating	0	128,750	0	0	0	0	0	0	0	0	0	0	0	0	0	128,750	
3	Upper Floridian Well	ENR	Operating	0	257,500	0	0	0	0	0	0	0	0	0	0	0	0	0	257,500	
4	6" Raw Water Main (200 ft)	ENR	Operating	0	20,600	0	0	0	0	0	0	0	0	0	0	0	0	0	20,600	
5	Phase 1 Contingency	ENR	Operating	0	128,750	0	0	0	0	0	0	0	0	0	0	0	0	0	128,750	
6	Phase 1 Engineering	ENR	Operating	0	83,430	0	0	0	0	0	0	0	0	0	0	0	0	0	83,430	
<b>Phase 2 - Multimedia Pressure Filters</b>																				
7	Mobilization/Demobilization	ENR	SD1	\$0	\$0	\$0	\$10,930	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$10,930	
8	Sitework	ENR	SD1	0	0	0	81,975	0	0	0	0	0	0	0	0	0	0	0	81,975	
9	Booster Pump Station	ENR	SD1	0	0	0	155,206	0	0	0	0	0	0	0	0	0	0	0	155,206	
10	Pressurized Filters and Backwash Holding Tank	ENR	SD1	0	0	0	1,059,117	0	0	0	0	0	0	0	0	0	0	0	1,059,117	
11	Chemical Building	ENR	SD1	0	0	0	91,812	0	0	0	0	0	0	0	0	0	0	0	91,812	
12	Chemical Systems	ENR	SD1	0	0	0	219,693	0	0	0	0	0	0	0	0	0	0	0	219,693	
13	High Service Pump Station Rehab	ENR	SD1	0	0	0	184,717	0	0	0	0	0	0	0	0	0	0	0	184,717	
14	Yard Piping	ENR	SD1	0	0	0	292,924	0	0	0	0	0	0	0	0	0	0	0	292,924	
15	Electrical	ENR	SD1	0	0	0	342,109	0	0	0	0	0	0	0	0	0	0	0	342,109	
16	Instrumentation	ENR	SD1	0	0	0	256,855	0	0	0	0	0	0	0	0	0	0	0	256,855	
17	Upper Floridian Well (includes transmission line)	ENR	SD1	0	0	0	273,250	0	0	0	0	0	0	0	0	0	0	0	273,250	
18	Well Rehabilitation	ENR	SD1	0	0	0	491,850	0	0	0	0	0	0	0	0	0	0	0	491,850	
19	Phase 2 Contingency	ENR	Operating	0	0	0	1,038,350	0	0	0	0	0	0	0	0	0	0	0	1,038,350	
20	Phase 2 Engineering	ENR	Operating	0	0	0	675,474	0	0	0	0	0	0	0	0	0	0	0	675,474	
<b>Phase 3 - Reverse Osmosis (without new HSPS)</b>																				
21	Mobilization/Demobilization	ENR	Operating	\$0	\$0	\$0	\$0	\$0	\$0	\$147,720	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$147,720	
22	Sitework	ENR	Operating	0	0	0	0	0	0	307,750	0	0	0	0	0	0	0	0	307,750	
23	RO System (2 skids)	ENR	SD2	0	0	0	0	0	0	812,460	0	0	0	0	0	0	0	0	812,460	
24	RO Building	ENR	SD2	0	0	0	0	0	0	400,075	0	0	0	0	0	0	0	0	400,075	
25	Chemical Systems	ENR	SD2	0	0	0	0	0	0	198,191	0	0	0	0	0	0	0	0	198,191	
26	Deep Injection Well	ENR	Operating	0	772,500	0	0	0	0	0	0	0	0	0	0	0	0	0	772,500	
27	Yard Piping	ENR	SD2	0	0	0	0	0	0	254,817	0	0	0	0	0	0	0	0	254,817	
28	Electrical	ENR	SD2	0	0	0	0	0	0	590,880	0	0	0	0	0	0	0	0	590,880	
29	Instrumentation	ENR	Operating	0	0	0	0	0	0	443,160	0	0	0	0	0	0	0	0	443,160	
30	Upper Floridian Wells	ENR	SD2	0	0	0	0	0	0	615,500	0	0	0	0	0	0	0	0	615,500	
31	Phase 3 Contingency	ENR	SD2	0	0	0	0	0	0	1,408,264	0	0	0	0	0	0	0	0	1,408,264	
32	Phase 3 Engineering	ENR	SD2	0	0	0	0	0	0	915,864	0	0	0	0	0	0	0	0	915,864	
<b>Phase 4 - New HSPS</b>																				
33	Mobilization/Demobilization	ENR	SD3	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$113,550	\$0	\$113,550	
34	Sitework	ENR	SD3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	302,800	302,800	
35	RO System (1 skid)	ENR	SD3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	581,376	581,376	
36	RO Building	ENR	SD3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	165,026	165,026	
37	High Service Pump Station Rehab	ENR	SD3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	511,732	511,732	
38	Yard Piping	ENR	SD3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	193,792	193,792	
39	Electrical	ENR	SD3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	252,838	252,838	
40	Instrumentation	ENR	SD3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	189,250	189,250	
41	Phase 4 Contingency	ENR	SD3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	693,412	693,412	
42	Phase 4 Engineering	ENR	SD3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	451,172	451,172	
43	Capital Purchases / Capital Outlay	Inflation	Rates	603,066	50,450	51,300	52,400	53,550	54,800	56,050	57,350	58,650	60,000	61,400	62,800	64,250	65,750	67,250	68,800	\$1,487,866
44	<b>TOTAL WATER CAPITAL IMPROVEMENT PROGRAM</b>			<b>\$603,066</b>	<b>\$1,462,580</b>	<b>\$51,300</b>	<b>\$5,226,662</b>	<b>\$53,550</b>	<b>\$54,800</b>	<b>\$56,050</b>	<b>\$6,152,031</b>	<b>\$58,650</b>	<b>\$60,000</b>	<b>\$61,400</b>	<b>\$62,800</b>	<b>\$64,250</b>	<b>\$65,750</b>	<b>\$3,522,198</b>	<b>\$68,800</b>	<b>\$17,623,887</b>

**Table 4**  
**Town of Belleair, Florida**  
**Water Rate Study**  
**Estimated Multi-Year Capital Improvement Program and Funding Source**

Line No.	Description	Escalation Factor	Funding Source	Projected Fiscal Year Ending September 30,													Total			
				2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032		2033	2034	2035
<b>FUNDING SOURCES:</b>																				
<b>Water System Funding Sources</b>																				
45	Operating Revenue		Rates	\$ 603,066	\$ 50,450	\$ 51,300	\$ 52,400	\$ 53,550	\$ 54,800	\$ 56,050	\$ 57,350	\$ 58,650	\$ 60,000	\$ 61,400	\$ 62,800	\$ 64,250	\$ 65,750	\$ 67,250	\$ 68,800	\$ 1,487,866
46	Operating Fund		Operating	-	1,412,130	-	1,713,824	-	-	-	898,630	-	-	-	-	-	-	-	-	4,024,584
53	Subordinate Lien Debt 1		SD1	-	-	-	3,460,438	-	-	-	-	-	-	-	-	-	-	-	-	3,460,438
54	Subordinate Lien Debt 2		SD2	-	-	-	-	-	-	-	5,196,051	-	-	-	-	-	-	-	-	5,196,051
55	Subordinate Lien Debt 3		SD3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3,454,948	-	3,454,948
56	Subordinate Lien Debt 4		SD4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
58	<b>TOTAL WATER CAPITAL IMPROVEMENT PROGRAM</b>			<b>\$603,066</b>	<b>\$1,462,580</b>	<b>\$51,300</b>	<b>\$5,226,662</b>	<b>\$53,550</b>	<b>\$54,800</b>	<b>\$56,050</b>	<b>\$6,152,031</b>	<b>\$58,650</b>	<b>\$60,000</b>	<b>\$61,400</b>	<b>\$62,800</b>	<b>\$64,250</b>	<b>\$65,750</b>	<b>\$3,522,198</b>	<b>\$68,800</b>	<b>\$17,623,887</b>

**Table 5**  
**Town of Belleair, Florida**  
**Water Rate Study**  
**Projected Cash Balances By Fund and Interest Earnings**

Line No.	Description	Investment Reference [*]	Fiscal Year Ending September 30,															
			2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035
<b>ENDING CASH BALANCE BY FUND SUMMARY</b>																		
1	OPERATING FUND	(U)	\$2,441,457	\$1,398,581	\$1,881,941	\$553,165	\$835,456	\$1,206,067	\$1,674,648	\$982,673	\$1,009,126	\$1,060,724	\$1,138,559	\$1,283,510	\$1,391,547	\$1,594,371	\$1,773,478	\$1,967,601
2	CUSTOMER DEPOSITS	(R)	23,504	23,804	24,104	24,404	24,704	25,004	25,304	25,604	25,904	26,204	26,504	26,804	27,104	27,404	27,704	28,004
3	TOTAL PROJECTED YEAR-END BALANCE		<u>\$2,464,961</u>	<u>\$1,422,385</u>	<u>\$1,906,045</u>	<u>\$577,569</u>	<u>\$860,160</u>	<u>\$1,231,071</u>	<u>\$1,699,952</u>	<u>\$1,008,277</u>	<u>\$1,035,030</u>	<u>\$1,086,928</u>	<u>\$1,165,063</u>	<u>\$1,310,314</u>	<u>\$1,418,651</u>	<u>\$1,621,775</u>	<u>\$1,801,182</u>	<u>\$1,995,605</u>
<b>OPERATING FUND</b>																		
4	Beginning Balance		\$2,567,665	\$2,441,457	\$1,398,581	\$1,881,941	\$553,165	\$835,456	\$1,206,067	\$1,674,648	\$982,673	\$1,009,126	\$1,060,724	\$1,138,559	\$1,283,510	\$1,391,547	\$1,594,371	\$1,773,478
5	Transfers In - Operations		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
6	Transfers In - General Fund		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7	Total Funds Available		<u>\$2,567,665</u>	<u>\$2,441,457</u>	<u>\$1,398,581</u>	<u>\$1,881,941</u>	<u>\$553,165</u>	<u>\$835,456</u>	<u>\$1,206,067</u>	<u>\$1,674,648</u>	<u>\$982,673</u>	<u>\$1,009,126</u>	<u>\$1,060,724</u>	<u>\$1,138,559</u>	<u>\$1,283,510</u>	<u>\$1,391,547</u>	<u>\$1,594,371</u>	<u>\$1,773,478</u>
8	Transfers Out - CIP		\$0	\$1,412,130	\$0	\$1,713,824	\$0	\$0	\$0	\$898,630	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
9	Transfers Out - Operations		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10	Total Transfers Out of Fund		<u>\$0</u>	<u>\$1,412,130</u>	<u>\$0</u>	<u>\$1,713,824</u>	<u>\$0</u>	<u>\$0</u>	<u>\$898,630</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>
11	Transfer In / (Out) - Surplus / (Deficiency)		(\$126,208)	\$369,254	\$483,360	\$385,048	\$282,291	\$370,611	\$468,581	\$206,655	\$26,453	\$51,598	\$77,835	\$144,951	\$108,037	\$202,824	\$179,107	\$194,123
12	Interest Rate		1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%
13	Interest Income on Fund		32,100	21,700	17,500	12,800	6,900	10,400	15,100	15,300	12,300	12,600	13,300	14,200	16,000	17,400	19,900	22,200
14	Use of Interest Income to Fund Revenue Requirements	(U)	32,100	21,700	17,500	12,800	6,900	10,400	15,100	15,300	12,300	12,600	13,300	14,200	16,000	17,400	19,900	22,200
15	Ending Balance		<u>\$2,441,457</u>	<u>\$1,398,581</u>	<u>\$1,881,941</u>	<u>\$553,165</u>	<u>\$835,456</u>	<u>\$1,206,067</u>	<u>\$1,674,648</u>	<u>\$982,673</u>	<u>\$1,009,126</u>	<u>\$1,060,724</u>	<u>\$1,138,559</u>	<u>\$1,283,510</u>	<u>\$1,391,547</u>	<u>\$1,594,371</u>	<u>\$1,773,478</u>	<u>\$1,967,601</u>
16	Target - Days of Rate Revenue		120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120
17	Target Minimum Available Cash		\$589,481	\$591,304	\$646,158	\$689,773	\$736,333	\$786,036	\$839,093	\$895,732	\$956,194	\$994,441	\$1,034,219	\$1,075,588	\$1,118,611	\$1,163,356	\$1,209,890	\$1,258,286
18	Target Minimum Cash Balance Met - Yes or No		Yes	Yes	Yes	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
19	Calculated Days of Rate Revenue		497	284	350	96	136	184	239	132	127	128	132	143	149	164	176	188
20	Target - Days of Operating Expenses		120	120	120	120	120	120	120	120	120	120	120	120	120	120	120	120
21	Target Minimum Available Cash		\$432,312	\$448,161	\$465,544	\$537,374	\$546,332	\$567,737	\$590,049	\$732,109	\$743,492	\$773,127	\$804,049	\$836,306	\$891,580	\$905,129	\$959,790	\$932,420
	Target Minimum Available Cash - Budget Policy		\$450,000	\$450,000	\$450,000	\$450,000	\$450,000	\$450,000	\$450,000	\$450,000	\$450,000	\$450,000	\$450,000	\$450,000	\$450,000	\$450,000	\$450,000	\$450,000
23	Target Minimum Cash Balance Met - Yes or No		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
24	Calculated Days of Operating Expenses		678	374	485	124	184	255	341	161	163	165	170	184	187	211	222	253
<b>CUSTOMER DEPOSITS</b>																		
25	Beginning Balance		\$23,204	\$23,504	\$23,804	\$24,104	\$24,404	\$24,704	\$25,004	\$25,304	\$25,604	\$25,904	\$26,204	\$26,504	\$26,804	\$27,104	\$27,404	\$27,704
26	Transfer In - New Accounts		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
27	Total Funds Available		<u>\$23,204</u>	<u>\$23,504</u>	<u>\$23,804</u>	<u>\$24,104</u>	<u>\$24,404</u>	<u>\$24,704</u>	<u>\$25,004</u>	<u>\$25,304</u>	<u>\$25,604</u>	<u>\$25,904</u>	<u>\$26,204</u>	<u>\$26,504</u>	<u>\$26,804</u>	<u>\$27,104</u>	<u>\$27,404</u>	<u>\$27,704</u>
28	Transfer Out -Reimbursements		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
29	Total Transfers Out of Funds		<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>	<u>\$0</u>
30	Interest Rate		1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%	1.25%
31	Interest Income on Fund		300	300	300	300	300	300	300	300	300	300	300	300	300	300	300	300
32	Use of Interest Income to Fund Revenue Requirements	(R)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
33	Ending Balance		<u>\$23,504</u>	<u>\$23,804</u>	<u>\$24,104</u>	<u>\$24,404</u>	<u>\$24,704</u>	<u>\$25,004</u>	<u>\$25,304</u>	<u>\$25,604</u>	<u>\$25,904</u>	<u>\$26,204</u>	<u>\$26,504</u>	<u>\$26,804</u>	<u>\$27,104</u>	<u>\$27,404</u>	<u>\$27,704</u>	<u>\$28,004</u>
<b>INTEREST INCOME SUMMARY</b>																		
34	Unrestricted Interest Income		\$32,200	\$21,800	\$17,600	\$12,900	\$7,700	\$11,200	\$15,900	\$16,100	\$14,100	\$14,400	\$15,100	\$15,900	\$17,700	\$19,100	\$21,600	\$24,600
35	Restricted Interest Income		\$300	\$300	\$300	\$300	\$300	\$300	\$300	\$300	\$300	\$300	\$300	\$300	\$300	\$300	\$300	\$300
36	Total Interest Income		<u>\$32,500</u>	<u>\$22,100</u>	<u>\$17,900</u>	<u>\$13,200</u>	<u>\$8,000</u>	<u>\$11,500</u>	<u>\$16,200</u>	<u>\$16,400</u>	<u>\$14,400</u>	<u>\$14,700</u>	<u>\$15,400</u>	<u>\$16,200</u>	<u>\$18,000</u>	<u>\$19,400</u>	<u>\$21,900</u>	<u>\$24,900</u>

**Footnotes:**

[\*] (U) = Interest earnings unrestricted and assumed to be available to meet System expenditure requirements.  
(R) = Interest earnings restricted and assumed to not be available to meet System expenditure requirements.



Table 6

**Town of Belleair, Florida  
Water Revenue Sufficiency Study**

**Comparison of Typical Monthly Residential Bills for Water Service\*]**

Line No.	Description	Residential Service for a 5/8" or 3/4" Meter										
		0 Gallons	2,000 Gallons	4,000 Gallons	5,000 Gallons	7,000 Gallons	10,000 Gallons	15,000 Gallons	20,000 Gallons	50,000 Gallons	75,000 Gallons	100,000 Gallons
<b>Town of Belleair</b>												
1	Existing Rates - Fiscal Year 2020	\$12.99	\$16.89	\$20.79	\$27.31	\$40.35	\$59.91	\$92.51	\$125.11	\$353.21	\$548.71	\$744.21
<b><u>Surveyed Florida Utilities:</u></b>												
3	City of Clearwater	\$23.16	\$23.16	\$32.55	\$41.94	\$60.72	\$91.34	\$150.54	\$209.74	\$564.94	\$860.94	\$1,156.94
4	City of Dunedin	7.47	16.29	25.11	29.52	42.76	62.62	95.72	128.82	427.32	676.07	924.82
5	City of Gulfport	16.93	16.93	34.98	44.56	66.00	98.16	157.36	222.21	611.31	935.56	1,259.81
6	City of Largo	6.80	17.06	27.32	32.45	42.71	58.10	83.75	109.40	263.30	391.55	519.80
7	City of Oldsmar	13.79	19.94	32.24	38.39	50.69	69.14	110.39	163.89	484.89	752.39	1,019.89
8	Pinellas County	6.80	17.06	27.32	32.45	42.71	58.10	83.75	109.40	263.30	391.55	519.80
9	City of Pinellas Park	21.93	21.93	29.24	36.55	51.17	73.96	114.71	158.85	428.85	653.85	878.85
10	City of Safety Harbor	20.46	27.00	33.54	36.81	53.17	82.83	174.88	266.93	1,064.63	1,729.38	2,394.13
11	City of St. Petersburg	12.89	22.21	31.53	36.19	47.16	68.90	108.65	161.15	791.45	1,316.70	1,841.95
12	City of Tarpon Springs	22.62	31.14	39.66	43.92	60.96	86.52	139.87	206.67	806.17	1,325.67	1,845.17
13	City of Treasure Island	6.80	17.06	27.32	32.45	42.71	58.10	83.75	109.40	263.30	391.55	519.80
14	Surveyed Florida Utilities' Average	\$14.51	\$20.89	\$30.98	\$36.84	\$50.98	\$73.43	\$118.49	\$167.86	\$542.68	\$856.84	\$1,171.00
15	Minimum	6.80	16.29	25.11	29.52	42.71	58.10	83.75	109.40	263.30	391.55	519.80
16	Maximum	23.16	31.14	39.66	44.56	66.00	98.16	174.88	266.93	1,064.63	1,729.38	2,394.13

## Footnotes:

[\*] Unless otherwise noted, amounts shown reflect residential rates in effect July 2020 and are exclusive of taxes, franchise fees or water restriction surcharges, if any, and reflect rates charged for inside the city service. All rates are as reported by the respective utility. This comparison is intended to show comparable charges for similar service for comparison purposes only and is not intended to be a complete listing of all rates and charges offered by each listed utility.

Table 7

Town of Belleair, Florida  
Water Rate Study

Projected Debt Service Coverage

Line No.	Description	Fiscal Year Ending September 30,															
		2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	2031	2032	2033	2034	2035
<b>Gross Revenues:</b>																	
1	Water System Sales Revenue - Existing Rates	\$1,793,006	\$1,798,550	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	\$1,841,121	
2	Water Rate Increases	\$0	\$0	\$124,276	\$256,940	\$398,559	\$549,737	\$711,120	\$883,397	\$1,067,302	\$1,183,639	\$1,304,629	\$1,430,459	\$1,561,322	\$1,697,420	\$1,838,961	\$1,986,165
3	Total Sales Revenues	\$1,793,006	\$1,798,550	\$1,965,397	\$2,098,061	\$2,239,680	\$2,390,858	\$2,552,241	\$2,724,518	\$2,908,423	\$3,024,760	\$3,145,750	\$3,271,580	\$3,402,443	\$3,538,541	\$3,680,082	\$3,827,286
<b>Other Revenues</b>																	
4	Unrestricted Interest Earnings	\$32,200	\$21,800	\$17,600	\$12,900	\$7,700	\$11,200	\$15,900	\$16,100	\$14,100	\$14,400	\$15,100	\$15,900	\$17,700	\$19,100	\$21,600	\$24,600
5	Other Revenues	6,600	2,511	7,694	1,000	1,000	1,000	2,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	
6	Total Other Revenues	38,800	24,311	25,294	13,900	8,700	12,200	17,900	17,100	15,100	15,400	16,100	16,900	18,700	20,100	22,600	25,600
7	Total Gross Revenues	\$1,831,806	\$1,822,861	\$1,990,691	\$2,111,961	\$2,248,380	\$2,403,058	\$2,570,141	\$2,741,618	\$2,923,523	\$3,040,160	\$3,161,850	\$3,288,480	\$3,421,143	\$3,558,641	\$3,702,682	\$3,852,886
8	<b>Cost of Operation and Maintenance</b>	\$1,314,948	\$1,363,157	\$1,416,031	\$1,634,513	\$1,661,760	\$1,726,868	\$1,794,731	\$2,226,833	\$2,261,453	\$2,351,595	\$2,445,649	\$2,543,763	\$2,711,890	\$2,753,101	\$2,919,360	\$2,836,110
9	Total System Net Revenues	516,858	459,704	574,660	477,448	586,620	676,191	775,410	514,785	662,069	688,564	716,201	744,717	709,253	805,540	783,322	1,016,776
<b>COVERAGE TESTS:</b>																	
<b>Senior Debt Service:</b>																	
10	Proposed Debt 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11	Proposed Debt 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12	Proposed Debt 3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13	Proposed Debt 4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14	Total Senior Lien Debt Service	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0
15	Required Coverage	120%	120%	120%	120%	120%	120%	120%	120%	120%	120%	120%	120%	120%	120%	120%	120%
16	Calculated Coverage	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
<b>AND</b>																	
<b>Subordinated Debt Requirement</b>																	
17	Net Revenues After Payment of Senior Lien Debt Service	\$516,858	\$459,704	\$574,660	\$477,448	\$586,620	\$676,191	\$775,410	\$514,785	\$662,069	\$688,564	\$716,201	\$744,717	\$709,253	\$805,540	\$783,322	\$1,016,776
18	Subordinated Debt Requirement	0	0	0	0	210,779	210,779	210,779	210,779	536,966	536,966	536,966	536,966	536,966	536,966	536,966	753,853
19	Required Coverage	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%
20	Calculated Coverage	N/A	N/A	N/A	N/A	278%	321%	368%	244%	123%	128%	133%	139%	132%	150%	146%	135%
<b>SRF LOAN COVERAGE TESTS:</b>																	
21	Net Revenues After Payment of Senior Lien Debt Service	\$516,858	\$459,704	\$574,660	\$477,448	\$586,620	\$676,191	\$775,410	\$514,785	\$662,069	\$688,564	\$716,201	\$744,717	\$709,253	\$805,540	\$783,322	\$1,016,776
22	Less Allowance for Senior Lien Debt Service Coverage (20%)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
23	Net Revenues Available for SRF Loan Debt	516,858	459,704	574,660	477,448	586,620	676,191	775,410	514,785	662,069	688,564	716,201	744,717	709,253	805,540	783,322	1,016,776
<b>SRF Loan Debt Service:</b>																	
24	2023 SRF Loan Issue	0	0	0	0	210,779	210,779	210,779	210,779	210,779	210,779	210,779	210,779	210,779	210,779	210,779	210,779
25	2027 SRF Loan Issue	0	0	0	0	0	0	0	326,186	326,186	326,186	326,186	326,186	326,186	326,186	326,186	326,186
26	2034 SRF Loan Issue	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	216,887
27	Total SRF Loan Debt Service	\$0	\$0	\$0	\$0	\$210,779	\$210,779	\$210,779	\$210,779	\$536,966	\$536,966	\$536,966	\$536,966	\$536,966	\$536,966	\$536,966	\$753,853
28	Required Coverage	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%	115%
29	Calculated Coverage (115% Required)	N/A	N/A	N/A	N/A	278%	321%	368%	244%	123%	128%	133%	139%	132%	150%	146%	135%
<b>Other Revenue Requirements:</b>																	
<b>Subordinate Lien Debt</b>																	
30	Interfund Loan - General Fund	\$40,000	\$40,000	\$40,000	\$40,000	\$40,000	\$40,000	\$40,000	\$40,000	\$40,000	\$40,000	\$40,000	\$0	\$0	\$0	\$0	\$0
31	Total Other Revenue Requirements	40,000	40,000	40,000	40,000	40,000	40,000	40,000	40,000	40,000	40,000	40,000	0	0	0	0	0
32	Amount Available for Capital Outlay and Other Purposes	\$476,858	\$419,704	\$534,660	\$437,448	\$335,841	\$425,411	\$524,631	\$264,005	\$85,103	\$111,598	\$139,235	\$207,751	\$172,287	\$268,574	\$246,357	\$262,923



# REVERSE OSMOSIS WTP PRELIMINARY ENGINEERING REPORT



SUBMITTED TO:

**TOWN OF BELLEAIR** AUGUST 2020

SUBMITTED BY:

**MCKIM & CREED** | CLEARWATER, FL



**MCKIM & CREED**  
ENGINEERS SURVEYORS PLANNERS

## Executive Summary

The Town of Belleair (Town) operates an existing water treatment plant (WTP) that was designed to produce 2.2 million gallons per day (MGD). The WTP treats groundwater from seven (7) existing wells and produces an average flow of approximately 800,000 gallons per day (gpd) of potable water to supply the Town's utility customers. The levels of chloride and total dissolved solids (TDS) in the wells have been increasing and are projected to continue increasing in the future. Because of these conditions, the potable water produced by the plant has chloride and TDS levels that periodically approach the Florida Department of Environmental Protection (FDEP) secondary drinking water limit of 250 mg/L and 500 mg/L, respectively.

Additionally, the WTP is nearing the end of its useful life and there are items that need to be addressed as soon as possible. This Preliminary Engineering Report (PER) included evaluations of the Town's existing water supply and treatment systems and considered the following:

1. Drinking water regulations
2. Condition of WTP existing systems and equipment
3. Condition of wells; above- and below-ground
4. Current and projected potable water demands
5. Groundwater supply and quality projections
6. Ability of existing systems to treat projected water quality

If the Town wishes to continue with potable water production, a new Reverse Osmosis (RO) treatment plant is recommended to reduce chloride and TDS concentrations and to address ongoing operational, maintenance and safety concerns at the WTP.

Besides developing preliminary engineering requirements for the proposed RO WTP systems, this PER was developed to provide the Town with capital and operations & maintenance (O&M) costs for the proposed WTP. Also, a potential phased implementation plan was developed that focuses on using the existing WTP infrastructure to the extent possible to reduce initial capital costs. In addition, the phased approach implements new treatment processes and modifications, only as needed, to address chloride and TDS levels projected to increase over time.

The use of a "conventional" RO process was compared with a relatively new high-recovery RO process called "Closed-Circuit RO (CCRO)." In general, the conventional RO process is able to utilize approximately 80% of the water supplied to the process to produce drinking water; whereas the CCRO process is able utilize approximately 90 to 95-percent. The evaluation and comparison of the two (2) processes are detailed in this PER.

Total capital costs for the proposed RO plant were developed and compared with a phased approach. These costs are shown in **Tables ES-1 and ES-2**.

**Table ES-1– Proposed Facility Cost Summary**

Item	Traditional RO System	High-Recovery CCRO System
Mobilization/Demobilization	\$175,000	\$175,000
Sitework & Demolition	\$325,000	\$325,000
Booster Pump Station	\$142,000	\$142,000
Pressurized Filters and Backwash Holding Tank	\$969,000	\$969,000
Chemical Building	\$84,000	\$84,000
Chemical Systems	\$362,000	\$362,000
RO System	\$1,044,000	\$1,488,000
RO Building	\$434,000	\$434,000
High Service Pump Station	\$338,000	\$338,000
Deep Injection Well	\$750,000	-----
Yard Piping	\$500,000	\$500,000
Electrical	\$738,000	\$677,000
Instrumentation	\$554,000	\$508,000
Upper Floridian Wells (4) and Well Rehabilitation	\$1,450,000	\$1,450,000
<b>Total Construction Cost</b>	<b>\$7,865,000</b>	<b>\$7,452,000</b>
Contingency (30%)	\$2,360,000	\$2,236,000
Sub Total	\$10,225,000	\$9,688,000
Engineering - Design and Legal (15%)	\$1,534,000	\$1,454,000
<b>TOTAL PROJECT COST</b>	<b>\$11,759,000</b>	<b>\$11,142,000</b>

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**Table ES-2 – Phased Implementation Plan Summary**

<b>Phase / Description of Major Systems</b>	<b>Approximate Implementation Year</b>	<b>Project Cost</b>
<b>Phase 1</b>		
1) <b>Clearwell Roof Rehabilitation</b> 2) <b>Additional Well</b>	2021	\$621,000
<b>Phase 2</b>		
1) <b>Booster Pump Station</b> 2) <b>Filtration System</b> 3) <b>Chemical Systems</b> 4) <b>Yard Piping</b>	2023	\$4,734,000
<b>Phase 3</b>		
1) <b>RO System</b> 2) <b>RO Building</b> 3) <b>Additional Chemical Systems</b> 4) <b>Deep Injection Well</b> 5) <b>Yard Piping</b>	2027	\$5,701,000
<b>Phase 4</b>		
1) <b>RO System Addition</b> 2) <b>RO Building Addition</b> 3) <b>New High Service Pump Station</b> 4) <b>Yard Piping</b>	2034	\$2,282,000
<b>TOTAL COST WITH PHASED IMPLEMENTATION</b>		<b>\$13,338,000</b>

Based on the evaluations performed in this PER, McKim & Creed recommends that the Town either begins constructing the proposed RO treatment (either in phases or overall plant) or decommissions the existing WTP and begin utilizing potable water from Pinellas County by the end of calendar year 2021.

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## 1.0 Project Purpose

The potable water produced by the Town's Water Treatment Plant (WTP) has chloride and TDS levels that periodically approach the Florida Department of Environmental Protection (FDEP) secondary drinking water limit of 250 mg/L and 500 mg/L, respectively. The Town's existing WTP technology was not intended to and is not effective for removing chloride.

If the Town wishes to continue with potable water production, a new Reverse Osmosis (RO) treatment plant is recommended to reduce chloride and TDS concentrations and to address ongoing operational, maintenance and safety concerns at the WTP. One of the main purposes of this Preliminary Engineering Report (PER) is to develop construction and operation & maintenance (O&M) cost estimates to assist the Town with an analysis of long-term water supply options, which may include decommissioning the existing WTP and bulk purchasing potable water from Pinellas County. Also, a relatively new technology was evaluated that can reduce the amount of groundwater needed and may negate the need to construct a deep injection well (DIW).

## 2.0 Existing Supply and Treatment System

### 2.1 Overview of Existing Supply and Treatment Facilities

The Town currently meets its water demands by the operation and management of a local groundwater wellfield as source water to the existing WTP. The wellfield is permitted by the Southwest Florida Water Management District (SWFWMD) under Water Use Permit (WUP) No. 20007692.005 and currently includes eight (8) (seven (7) existing + one (1) proposed) upper Floridan aquifer production wells. The current water use permit (Permit No. 20007692.007) was issued on November 27, 2017 and will expire on November 27, 2037.

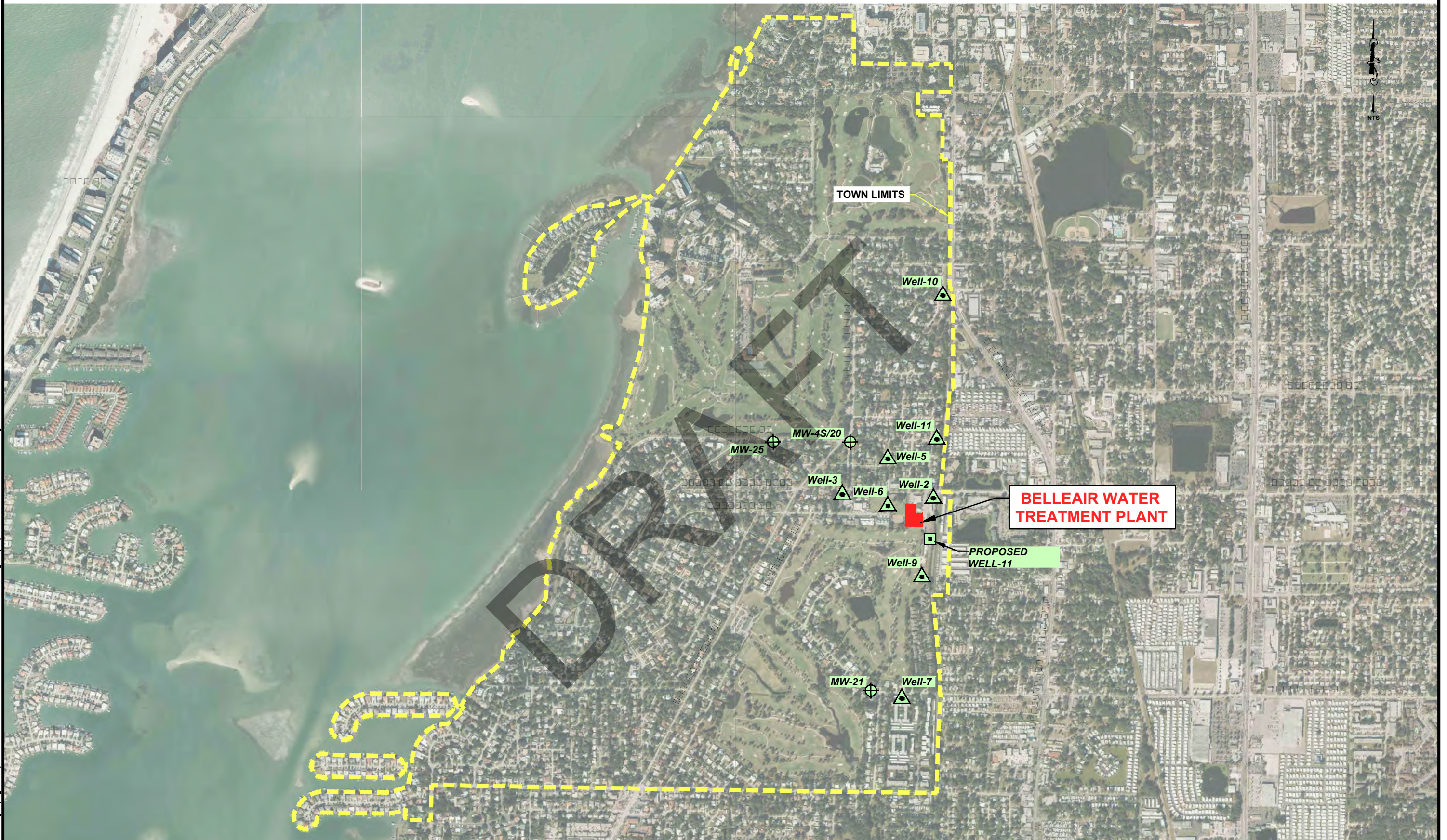
Of the seven (7) wells currently utilized, Well # 3 is used minimally due to extremely high chloride levels. The wellfield is permitted to withdraw an annual average of 885,900 gallons per day (gpd) with a peak month of 1,063,100 gpd, while the WTP is rated for 2,200,000 gpd. With the exception of periodic use of potable water from Pinellas County along with use of reclaimed water for two (2) golf courses within the Town, the WTP currently provides 100% of the Town's potable water and irrigation needs. Groundwater from the existing wells is pumped to the WTP for treatment using aeration, sedimentation, filtration, and disinfection. Current average daily production is approximately 800,000 gpd and has remained relatively consistent over the past several years.

## 2.2 Existing Wellfield and Groundwater Supply System

The Town's municipal wellfield is shown on **Figure 2-1** and is located entirely within the Town limits. Water is pumped from the wells and is transferred to the WTP via an existing raw water transmission system (Refer to **Appendix A**).

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i:\01131\0014\Water\80-Drawings\Figures\FIGURE 2-1 TOWN LIMITS\_EXIST\_WELLFIELD.dwg, 8/3/2020 1:08:55 PM, Cathy Walker



**BELLEAIR WTP IMPROVEMENTS**

**TOWN LIMITS & EXISTING  
WELLFIELD**



**TOWN OF  
BELLEAIR**  
901 PONCE DE LEON BOULEVARD  
Florida 34104  
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DWG. No.

**FIG 2-1**

Table 2-1 summarizes the permitted and current withdrawal rates for the existing wells.

**Table 2-1 – Well Characteristics Summary**

Well ID	Permitted Withdrawal Average (gpd)	Current Withdrawal Rate (gpd)
2	145,100	120,000 (125 gpm for 16 hrs)
3	145,100	105,600 (110 gpm for 16 hrs)
5	145,100	144,000 (150 gpm for 16 hrs)
6	145,100	115,200 (120 gpm for 16 hrs)
7	193,000	144,000 (150 gpm for 16 hrs)
9	193,000	134,400 (140 gpm for 16 hrs)
10	193,000	115,200 (120 gpm for 16 hrs)
11 <sup>1</sup>	145,000	-

<sup>1</sup> Permitted, but not installed

The scope of this PER included above- and below-ground well evaluations on the seven (7) existing wells to estimate recommended flows and costs to replace and/or rehabilitate the existing wells. Results from this evaluation are included in **Appendix B**.

### 2.3 Existing Water Treatment Plant

Slightly brackish raw water is pumped to the WTP from the groundwater wells. The raw water enters the treatment process through a common header that discharges to an existing tray aerator designed for hydrogen sulfide removal. Water cascades down the aeration trays into the sedimentation basin and is treated with chlorine to oxidize iron present in the groundwater wells. The oxidized iron and other suspended solids settle to the bottom of the tank and are later removed. The clarified water then flows to four (4) existing sand filters that further remove suspended solids. After filtration, the water is treated with a corrosion inhibitor, fluoride, and chlorine then flows through the clearwell where it can either be equalized with two storage tanks or pumped by four (4) high service pumps into the distribution system. The treatment process also includes ammonium hydroxide, which forms chloramines to aid with maintaining the required disinfection residual in the potable water distribution system. The two (2) ground storage tanks are located on the plant site and are rated for 0.5 million gallons (MG) and 0.3 MG, respectively. The Town also has an emergency interconnect with Pinellas County Utilities that can be used to provide potable water on a short-term basis.

## 3.0 Preliminary Engineering Considerations

### 3.1 Potable Water Demands

As mentioned in **Section 2.1**, the current average daily potable water demand is approximately 800,000 gpd. This rate has been relatively consistent over the past several years due to a

population which has remained consistent within the service area. The Town's population is not expected to have significant growth in the future considering the lack of availability of land within the service area. This information was used to confirm the proposed facility's design capacity of 1.0 MGD.

### 3.2 Water Quality Regulations and Goals

The Town's WTP must meet the requirements of the FDEP and the USEPA, which set national standards for drinking water as authorized by the Safe Drinking Water Act (SDWA). The primary rules and regulations that apply to the Town's WTP are Chapters 62-550 and 62-560 of the FAC, and the maximum contaminant levels defined by the USEPA. McKim & Creed has reviewed the Town's previous annual Water Quality Reports and found that the levels of primary drinking water contaminants have consistently been well below the regulatory limits in terms of maximum contaminant level (MCL).

The State of Florida has adopted the National Primary and Secondary Drinking Water Standards of the USEPA and has created additional rules to fulfill state requirements. There are also contaminants that are listed in one set of the standards but not in the other. Therefore, priority shall be given to the strictest standards. Water quality parameters, according to these regulations, are expected to be met, not only at the plants but also at the customers' taps.

The secondary drinking water MCL set by the FDEP for TDS and chlorides are 500 mg/L and 250 mg/L, respectively. Therefore, in an effort to comfortably and consistently stay below these levels, a goal for the maximum TDS and chloride concentrations has been set at 450mg/L and 225 mg/L, respectively.

### 3.3 Staffing Considerations

Per Florida Administrative Code 62-699.310, Water Treatment Plant Category II (Microfiltration, ultrafiltration, nanofiltration, or reverse osmosis) Class B (1 MGD to 6.5 MGD) facilities requires the following staffing:

- Staffing by Class C or higher operator: 16 hours/day for 7 days/week. The lead/chief operator must be Class B or higher.

For plants that are under an electronic surveillance system or automatic control system, staffing requirements shall be reduced as follows:

- Staffing for a Class B plant shall be reduced to no less than staffing by a Class C or higher operator 8 hours/day for 7 days/week with the 8 hours/day of staffing occurring during the 8-hour period of greatest influent flow or water production. The class of the lead/chief operator shall not be reduced.

For plants that are under an electronic control system, staffing requirements shall be reduced as follows:

- Staffing for a Class B plant shall be reduced to no less than staffing by a Class C or higher operator 4 hours/day for 5 days/week and one visit by a Class C or higher operator on each weekend day. The class of the lead/chief operator shall not be reduced.

It is recommended that the Town install an electronic control system to have the opportunity to reduce staffing requirements. However, the Town should expect to have a Class C or higher operator at the proposed facility 16 hours per day; 7 days per week for at least the first year of operations.

### 3.4 Wellfield Water Quality Projections

McKim and Creed developed trendlines using data collected and analyzed for each well in the *Hydrologic and Environmental Conditions Report Water Use Permit Special Condition 7.B*, which was prepared in March 2014 by HSW Engineering, Inc. The data was comprised of measured chloride concentrations available for Production Wells 2, 3, 5, 6, 7, 9, and 10 from inception to 2013 to create the referenced trendlines that were used to help develop projected TDS concentrations over the 20-year planning period. Also, groundwater quality data was obtained from the Town and was used to confirm and adjust the projections. The calculations for these projections are included in **Appendix C** and the projections for each well are presented in **Table 3-1**.

**Table 3-1 – TDS Projections for Production Wells**

Well #	Typical Flow <sup>1</sup> (gpm)	2020 TDS Level (mg/L)	2030 TDS Level (mg/L)	2040 TDS Level (mg/L)
2	125	233	270	305
3	110	3029	4624	6119
5	150	234	265	294
6	120	631	901	1153
7	150	361	417	470
9	140	151	156	160
10	120	158	217	272
<b>Weighted Average</b>	915	620	875	1114

<sup>1</sup> Flow rates are approximations provided by the Town

It should be noted that in agreement with the projections above, Wells 3 and 6 routinely produce water with TDS levels above the secondary drinking water MCL of 500 mg/L.

## 4.0 Proposed Well Improvements

### 4.1 Recommended Flows

McKim & Creed coordinated with our subconsultants, Applied Drilling, Inc. and WSP, to perform evaluations for the existing wells (refer to **Appendix C**) and developed recommended flows based on the evaluations as shown in **Table 4-1**.

**Table 4-1 – Recommended Flows for Existing Wells**

Well #	Typical Flow <sup>1</sup> (gpm)	Recommended Flow (gpm)
2	125	220
3	110	N/A <sup>2</sup>
5	150	150
6	120	120
7	150	150
9	140	220
10	120	120
11	N/A	130

<sup>1</sup> Flow rates are approximations provided by the Town

<sup>2</sup> It is recommended that Well 3 be taken out of service

### 4.2 Additional Supply Wells

The RO treatment process typically produces approximately 80% of the water supplied to the treatment process as potable water and the remaining water (RO concentrate) is typically disposed. Because of this, additional groundwater supplies will be needed. McKim & Creed performed calculations and estimated that up to six (6) additional wells may be needed to meet the Town's potable water demands within the 20-year planning period.

## 5.0 Proposed RO Water Treatment Plant

### 5.1 Overview of Proposed Supply and Treatment System

The proposed treatment process includes treatment of groundwater from the Town's existing and proposed supply wells. The combined flow from the wells will be routed to a booster pump station. Following the booster pump station, the water will be treated with sodium hypochlorite to oxidize iron, and a coagulant, such as ferric chloride, that will improve filtration for the three (3) proposed pressurized dual-media filters. Subsequently, the filtered water will be treated with sodium bisulfite to remove any residual free chlorine and an antiscalant will be added to reduce scaling on the RO membrane elements. The water will then be directed to, or will bypass, the proposed RO system. The flow directed to the RO processes will flow through a cartridge filtration system that will filter out particles larger than 5-microns and will be

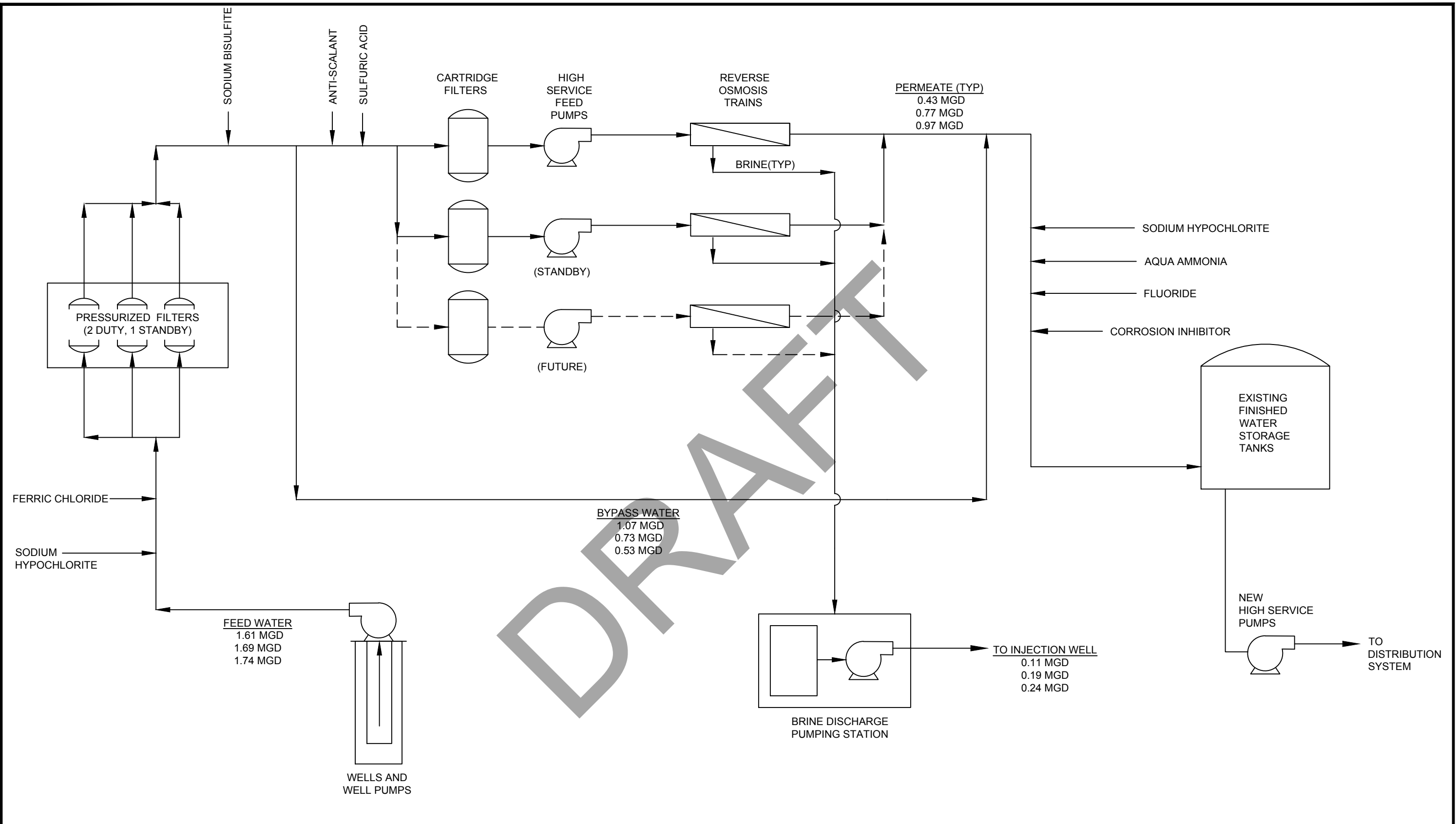


pumped into the proposed RO treatment system. The RO permeate will be corrosive in nature and will be treated with sodium hydroxide to increase pH and alkalinity. The RO treated water will then be blended with the filtered bypass flow, treated with sodium hypochlorite and fluoride and directed to the two (2) existing ground storage tanks (total of 0.8 MG of storage) after disinfection. Disinfection will be achieved using sodium hypochlorite and ammonium hydroxide to form chloramines that will provide residual disinfection in the Town's potable water distribution system. The disinfected finished water will then be pumped into the community via four (4) high service pumps.

A process flow diagram for the proposed treatment process is shown in **Figure 5-1**.

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I:\\_01131\0014\Water\80-Drawings\Figures\FIGURE 5-1 PROP TREATMENT PROCESS FLOW DIAGRAM.dwg, 7/30/2020 12:18:20 PM, Cathy Walker



**NOTE:**  
 FLOW VALUES SHOWN FROM TOP TO BOTTOM REFERENCE DESIGN  
 TDS LEVELS OF 620, 875 AND 1,114 MG/L, RESPECTIVELY.

## 5.2 Sizing of Proposed RO Treatment System

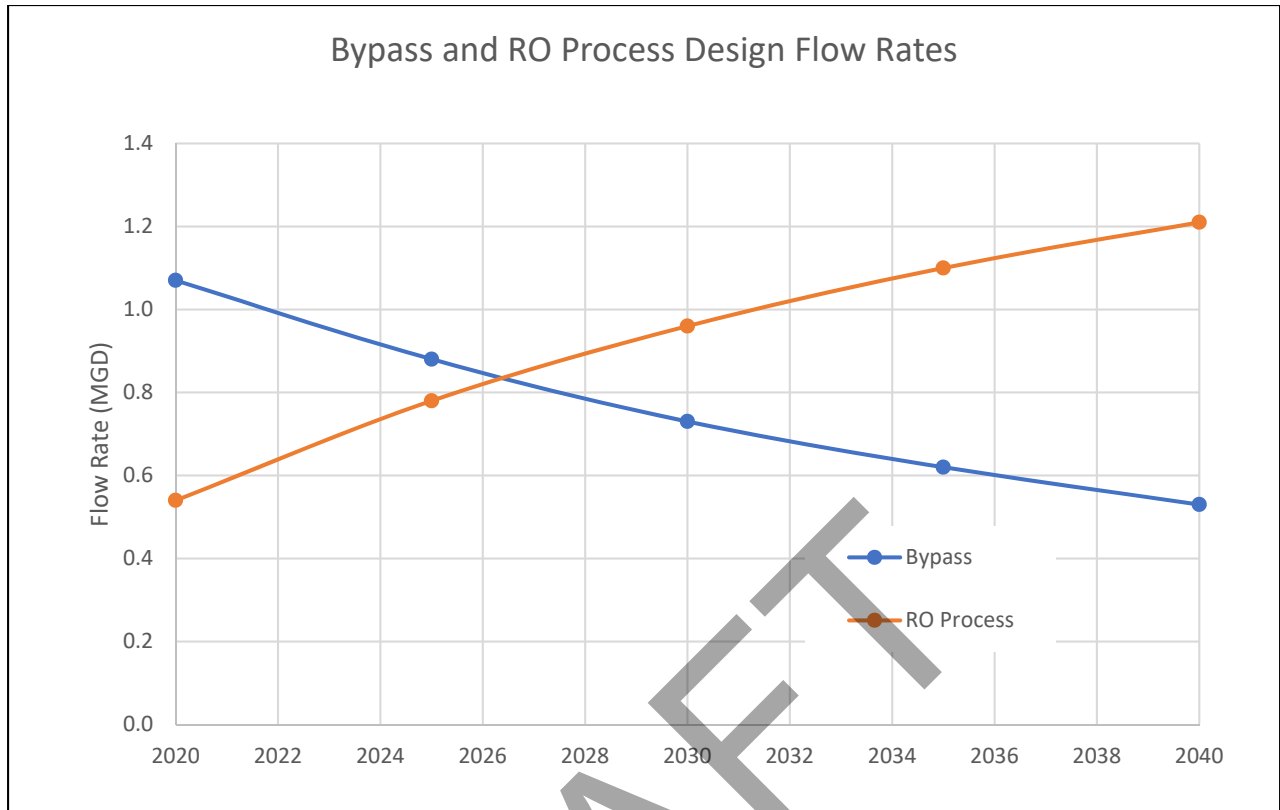
As mentioned above, the proposed RO treatment plant will be designed to produce a total average daily flow of 1.0 MGD over a 24-hour period. It should be noted that the facility would operate for only 16-hours per day and thus the design flow rate of the RO system is actually 1.5 MGD.

Based on RO membrane water quality projections and the weighted average TDS concentration projections from **Table 3-1**, calculations were conducted to estimate the percentage of raw water which would require treatment by the RO system to produce a desired quantity of finished water with a TDS concentration no higher than 450 mg/L. The membrane suppliers were consulted to estimate the TDS concentration of the RO permeate for the projected blend concentrations. **Table 5-1** presents the estimated blend scenarios that would vary as TDS levels increase in the future and **Figure 5-2** shows the bypass and RO process design flow rates over time. It is important to note that the calculation results are based on a traditional (80-percent) recovery system as a conservative approach.

**Table 5-1 – Projected TDS Average Concentrations and Design Scenarios**

Year	Projected Raw Water TDS (mg/L)	Permeate TDS Conc. (mg/L)	Flow to Bypass (MGD)	Flow to RO Process (MGD)	Design Raw Water Flow (MGD)	Design Finished Water TDS (mg/L)	Design Finished Water Flow (MGD)
2020	620	23.2	1.07	0.54	1.61	450	1.50
2025	748 <sup>1</sup>	34.0 <sup>1</sup>	0.88	0.78	1.66	450	1.50
2030	875	44.7	0.73	0.96	1.69	450	1.50
2035	995 <sup>1</sup>	65.0 <sup>1</sup>	0.62	1.10	1.72	450	1.50
2040	1114	85.2	0.53	1.21	1.74	450	1.50

<sup>1</sup> Interpolated value



**Figure 5-2 – RO and Bypass Design Flow Rates Over Time**

### 5.3 Proposed RO Water Treatment Plant Processes

The following sections describe the various processes that will treat raw water starting from the combined raw water main and culminating at the high service pump station just prior to feeding into the distribution system. Chemical treatment, yard piping, and sitework will be discussed as well. Finally, an alternative to the traditional 2-stage RO system called a closed-circuit RO (CCRO) system will be evaluated.

#### 5.3.1 Booster Pump Station

A booster pump station is required to increase the pressure from the well pumps so that the raw water can pass through the subsequent pressurized dual-media filters. The required pressure to feed the filters is approximately 30 pounds per square inch (psi) at a design flow of 1.74 MGD (1208 gpm). Based on this flow and pressure, each pump will be designed for 604 gpm @ 69.3' TDH and will include variable frequency drives (VFD) to save on energy costs and to provide operational control and flexibility. Therefore, a triplex set-up with three (3) 15-HP pumps (2 duty + 1 stand-by) is recommended for the booster pump station. **Table 5-2** summarizes key design criteria for the booster pumps.

**Table 5-2 – Booster Pump Design Criteria**

Item	Design Criteria
Quantity	3 (2 +1)
Capacity (gpm)	604 (0.87 MGD) @ 69.3 ft TDH
Power (HP)	15

### 5.3.2 Pressurized Dual-Media Filter System

Following chemical oxidation and coagulation, the groundwater will be filtered by a bank of three (3) proposed pressurized dual-media filters that will remove precipitated and coagulated iron and other particulates in the chemically treated groundwater. The treated groundwater will enter near the top of the filters; pass through a layer of anthracite and a layer of sand, and will be collected near the bottom of the filters at the filter underdrain system.

The filters will be constructed of steel in accordance with ASME code requirements and stamped with an ASME code stamp. Each tank will be equipped with the necessary flanges and connections for the main piping system and the top of the tank head will be equipped with a manhole for testing and maintenance. Each unit is to be supported on four structural legs welded to the side shell.

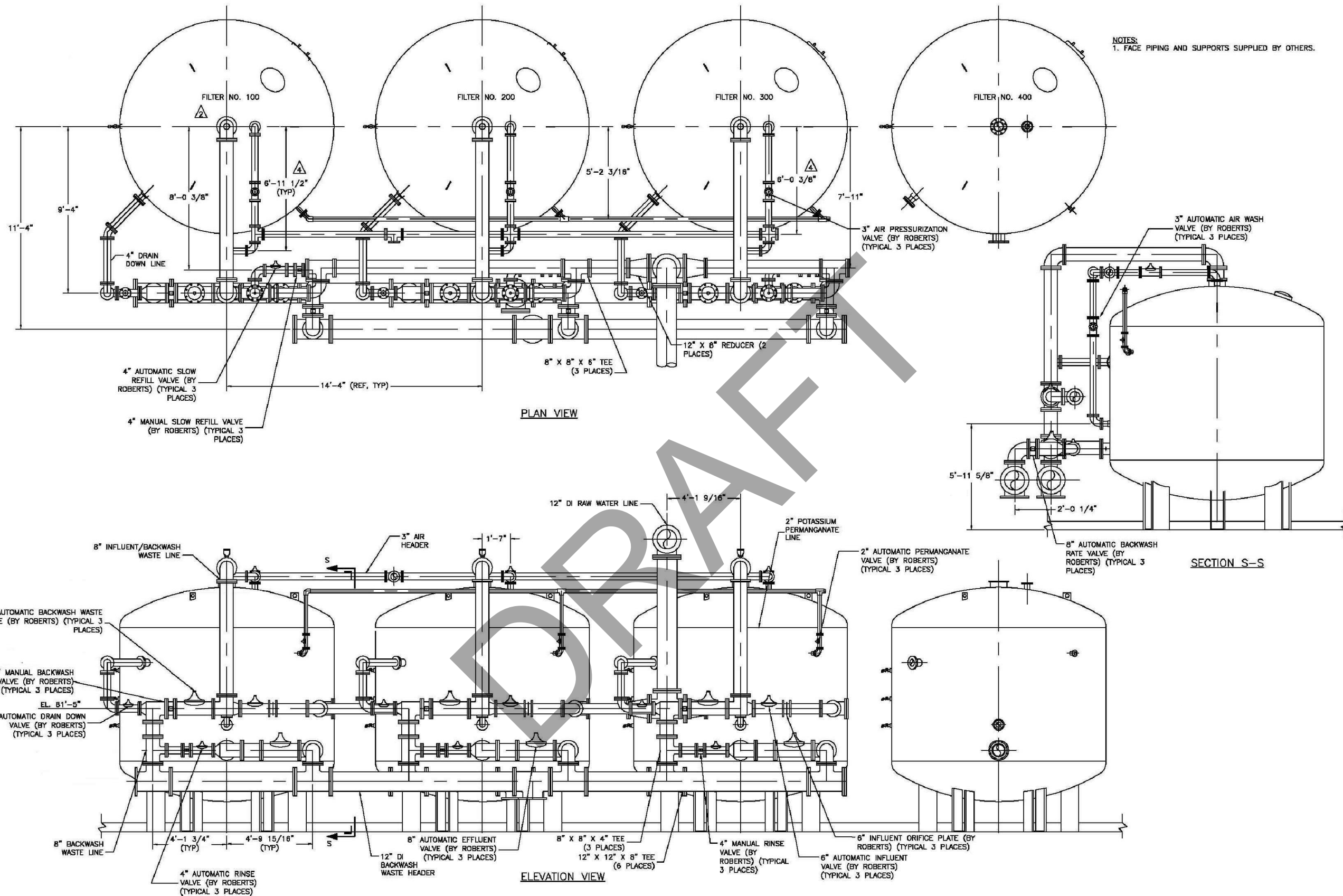
**Table 5-3** includes the design criteria for the pressurized dual-media filter system. It is recommended that final design criteria be established during pilot testing of the overall system and it is noted that the proposed filter system may utilize compressed air, rather than sodium hypochlorite, for oxidation.

**Table 5-3 – Pressurized Dual-Media Filter System Information**

Item	Design Criteria
Total Design Flow (gpm)	1208
Number of Filters Required	3
Normal Loading Rate with All Vessels Online (gpm/ft <sup>2</sup> )	4.7
Loading Rate with One Vessel Offline (gpm/ft <sup>2</sup> )	6.3
Tank Diameter (ft)	10.5
Tank Straight Side Height (ft)	11.3

See **Figure 5-3** for a conceptual drawing of the layout of the pressurized filters

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**BELLEAIR WTP IMPROVEMENTS**

**PRESSURIZED FILTERS  
CONCEPTUAL LAYOUT**



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DWG. No.

**FIG 5-3**

See **Figure 5-4** for an example of typical dual-media pressure filters.



**Figure 5-4 – Typical Dual-Media Pressurized Dual-Media Filters, City of Clearwater**

### **5.3.3 Proposed RO Treatment System**

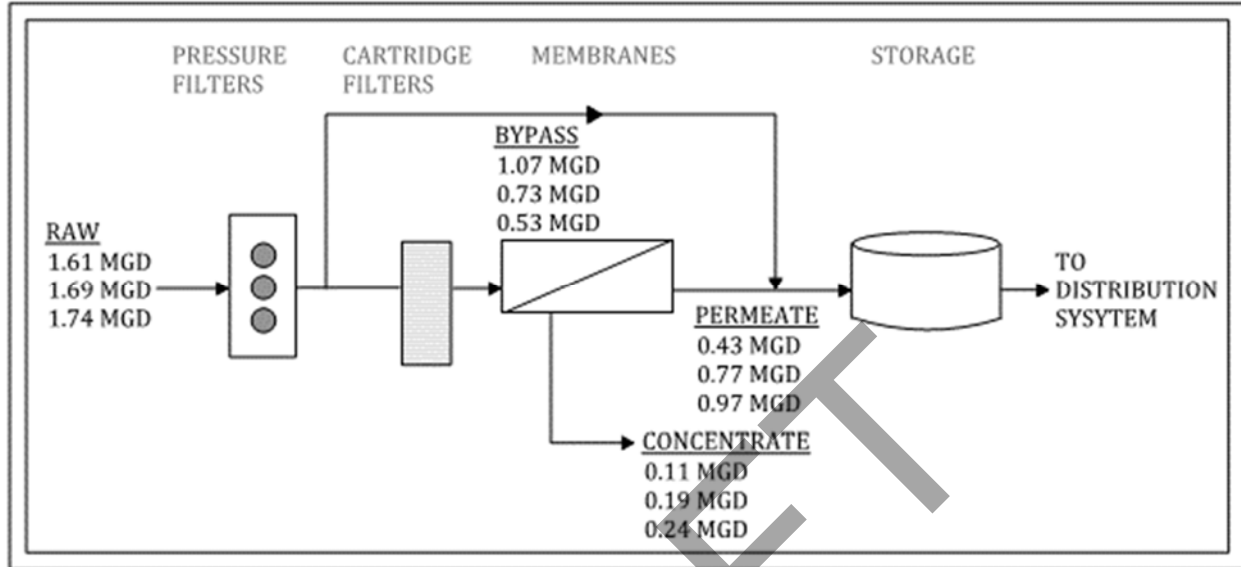
The proposed RO system described herein is based on a system design flow of 1.5 MGD that will be operated for 16-hours per day to provide 1.0 MG over a 24-hour period. The proposed RO system will include three (3) skids; 0.50 MGD each that will remove chloride and hardness causing salts, along with other contaminants. A proposed 2-stage RO system configuration allows for approximately 50% recovery in the first stage. Subsequently, the concentrate from the first stage is directed to a second stage to increase the overall recovery to approximately 80%. The proposed system employs three (3) skids and will allow the Town to perform routine maintenance, cleanings and repairs on one skid, while providing 1.0 MGD of permeate water from the other two (2) skids. High pressure feed pumps are used to transfer pre-treated feedwater through the RO membrane elements. Although a conservative approach is used to establish preliminary engineering criteria, McKim & Creed recommends that pilot testing be performed prior to detailed design to confirm RO design criteria and to optimize the RO treatment process.

#### **5.3.3.1 RO Raw Feed Water Configuration**

Plant operations will require that the effluent from the pressurized media filters be divided into two (2) streams prior to the cartridge filters: one stream will bypass the membrane system and the second stream will provide the feedwater to the cartridge filters, RO feed pumps, and membrane units. The membrane feed piping will include automated inlet valves, flow sensors, and pressure transmitters. Additionally, the process piping will include isolation valves for skid shut down for maintenance and cleaning; sample ports will be provided to monitor feedwater quality. After passing through the cartridge filters, the feedwater will be pressurized by high

pressure RO feed pumps and fed into the RO pressure vessels. A simplified operational schematic of the pretreatment and RO system is shown in **Figure 5-5**.

**Figure 5-5 - Simplified Treatment System Design Flow Diagram**



Note: Numbers from top to bottom show calculated design flows using design chloride levels of 620, 875, and 1,114 mg/L, respectively. Actual permeate and bypass flows will be based on nearest 0.5 MGD increment.

### 5.3.3.2 Cartridge Filter System

A cartridge filter system, using 5-micron cartridge filters, is proposed upstream of the RO membranes as additional protection to remove particles that might pass through the dual-media filters. This system will include one (1) cartridge filter located on each RO skid. The cartridge filter/cartridge filter vessel design criteria are provided in **Table 5-4**.

**Table 5-4 – Cartridge Filter Vessel Design Criteria**

Feature	Design Criteria
Quantity	3
Hydraulic Loading, each filter vessel (gpm)	434 (0.63 MGD)
Feedwater pH	5.9 – raw water pH
Max. Fouled Element Headloss (psi)	10
Cartridge Filter Material	Continuously Wound Polypropylene
Cartridge Vessel Material	304 SS
Cartridge Filter Vessel Pressure Rating	150 psig @ 300°F per ASME Boiler and Pressure Vessel Code, Section VIII, Division 1



### 5.3.3.3 Proposed RO Feed Pumps

One (1) RO feed pump will be provided for each RO skid (total of 3 RO feed pumps). Each RO feed pump will have a conservative design capacity of 434 gpm at a design head of 166 feet (72 psi) total dynamic head. Multistage, centrifugal, vertical turbine pumps are recommended for the RO feed pumps. The materials of construction for the wetted parts, bowls, impellers, columns, shafts, discharge heads, pump cans, of each RO feed pump will be 316 stainless steel. Each pump suction pipe will be 4-inch diameter, PVC, and Schedule 80. Each pump discharge pipe will be 4-inch diameter 304 stainless steel, Schedule 10S. Each pump will be driven by a 75-horsepower electric motor, inverter duty, premium efficiency, operating via a 460-volt, 60 hertz, 3-phase electric power source. Each vertical turbine pump will be driven by a variable frequency drive in order to adjust RO feed flow rates and pressures. **Table 5-5** summarizes key design criteria for the RO feed pumps.

**Table 5-5 – RO Feed Pump Design Criteria**

Feature	Design Criteria
Quantity	3
Capacity (gpm)	434 (0.63 MGD) @ 166 ft TDH
Power (HP)	25

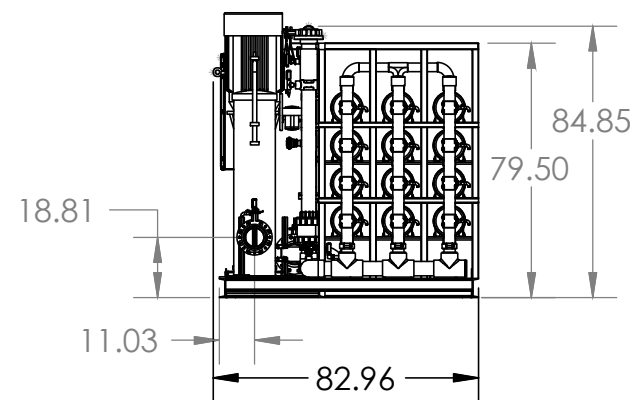
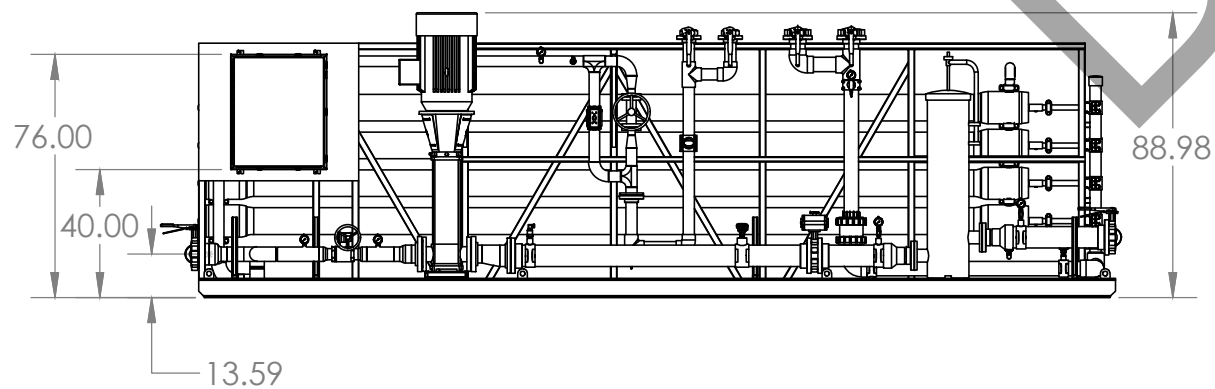
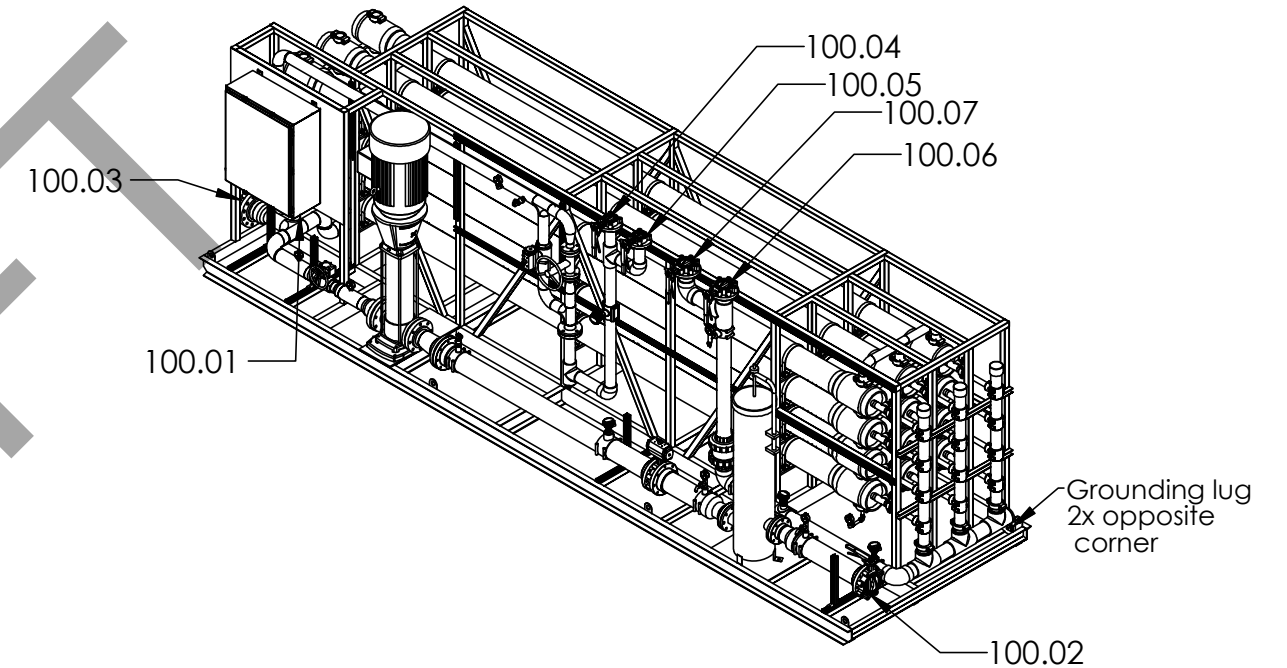
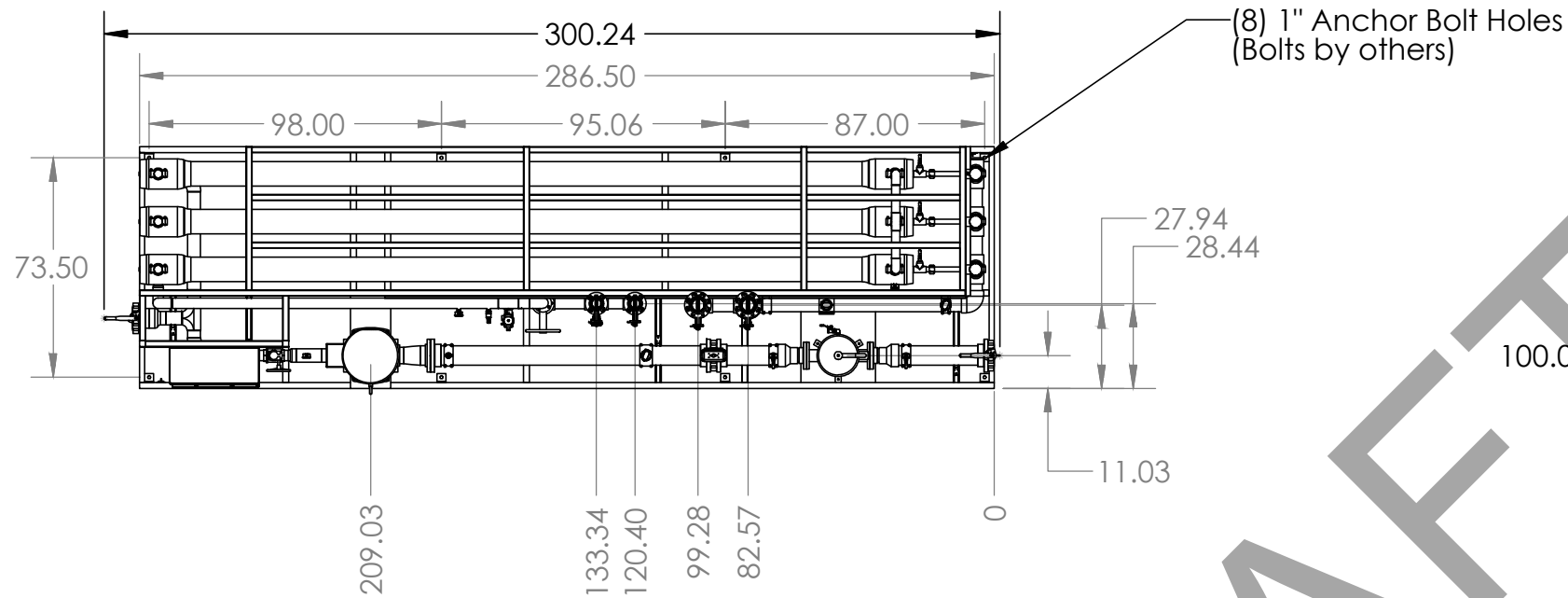
A summary of the RO Skid configuration is presented in **Table 5-6**.

**Table 5-6 – RO Skid Configuration Summary**

Feature	Design Criteria
Number of RO Treatment Skids	3
Design Feedwater Capacity of Each	0.63 MGD (434 gpm)
Design Permeate Capacity of Each	0.50 MGD (347 gpm)
Design Concentrate Flow	0.13 MGD (90 gpm)
Total Design Feedwater Flow	1.89 MGD (1312 gpm)
Total Permeate Design Flow	1.50 MGD (1042 gpm)
Total Design Concentrate Flow	0.39 MGD (271 gpm)

A conceptual membrane skid configuration and isometric view is shown in **Figure 5-6**.

Connection #	Name	Size	Description
100.01	Air Inlet	1/2"	Compression
100.02	Raw Water Inlet	6"	150# RF Flg.
100.03	CIP Inlet	6"	150# RF Flg.
100.04	Reject	3"	150# RF Flg.
100.05	Reject CIP Return 3'	150#	RF Flg.
100.06	Permeate	4"	150# RF Flg.
100.07	Permeate CIP Return	4"	150# RF Flg.



**Design Specs:**

1. Designed inlet flow:
2. Designed inlet pressure:
3. Designed product flow:

**Notes:**

1. Unloading and install by others.
2. All external loads supported by others.
3. Approx. operating weight: 13,000 lbs
4. Approx. shipping weight: 12,000 lbs
5. Electrical terminations by others.
6. Interconnecting piping by others.
7. Skid coating: Black Powder Coat
8. Skid material: Mild Steel
9. Piping material
  - 9a: Low pressure piping: Sch. 80 PVC
  - 9b: High pressure piping: 304SS
10. Allow for 48" of clearance on both ends of RO for membrane removal.
11. Allow for 48" of clearance above filter housing for filter removal.
12. Electrical conduit material to be:
13. Air line material to be: LLDPE

NOT TO SCALE

I:\01131\0014\Water\80-Drawings\Figures\FIGURE 5-6 RO CONFIG AND ISO VIEW.dwg, 7/28/2020 1:48:13 PM, Cathy Walker



**BELLEAIR WTP IMPROVEMENTS**  
**REVERSE OSMOSIS CONFIGURATION AND**  
**ISOMETRIC VIEW**



**TOWN OF**  
**BELLEAIR**  
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 Florida 34104  
 (727) 588-3769

**DWG. No.**  
**FIG 5-6**

Each proposed RO skid will have approximate dimensions of 24 feet in length, 7 feet in width, and 9 feet in height. Each skid will include twelve (12) fiberglass reinforced plastic (FRP) pressure vessels, 8-inches in diameter, that will be arrayed on a FRP structural frame (skid). The feedwater and concentrate piping on the skid will be fabricated from 304 stainless steel. The permeate piping on the skid will be fabricated from Schedule 80 PVC pipe. Each permeate connection from each pressure vessel will be equipped with a sampling connection that will be piped to a sampling panel.

The proposed pressure vessel array is based on a 2-stage system having seven (7) membrane elements in each pressure vessel. The array will include eight (8) pressure vessels in the first stage and four (4) in the second stage and will include a total of 108 RO membrane elements. The RO concentrate from the first stage will be directed to the second stage membrane elements to increase the overall recovery to approximately 80-percent.

#### **5.3.3.4 RO Membrane Elements**

The proposed low-pressure RO membranes for the skids are fabricated from polyamide, spiral wound, with a fiberglass outer wrap. Each membrane has a surface area of 440 square feet. Typical operating pressures for the membranes range from 75-175 psi, although they can withstand a maximum liquid pressure of 300 psi. Exposure of the membranes to free chlorine can damage them. The membranes are resistant to liquids with pH values between 4 and 11 during normal operation, and 2.5 to 11 during short-term operations such as cleaning. The maximum pressure differential allowed per membrane element is 15 psi and the maximum pressure in the pressure vessels is 300 psi. The turbidity of the raw water feed should be less than 1 Nephelometric Turbidity Unit (NTU), and the silt density index (SDI) should be less than 3.

#### **5.3.3.5 Clean-in-Place (CIP) System**

The most common problem with RO membranes is fouling caused by scale buildup, biological growth, or deposition of colloidal material. Fouling leads to an increased resistance to flow through the membranes, which increases differential pressure and decreases the performance of the system. Scaling is caused by a concentration of salts in excess of their saturation point, which precipitates salt deposits on the membrane surface. Fouling reduces permeate flow, increases pressure losses across the membranes, and affects the permeate quality. When the RO membrane system becomes fouled, the membranes must be cleaned to accomplish some or all of the following:

1. Dissolve and remove inorganic scales
2. Flush out particulate material
3. Breakdown bacterial slimes
4. Eliminate bacteria and other microorganisms

A proposed CIP cleaning system will generally include the following items:

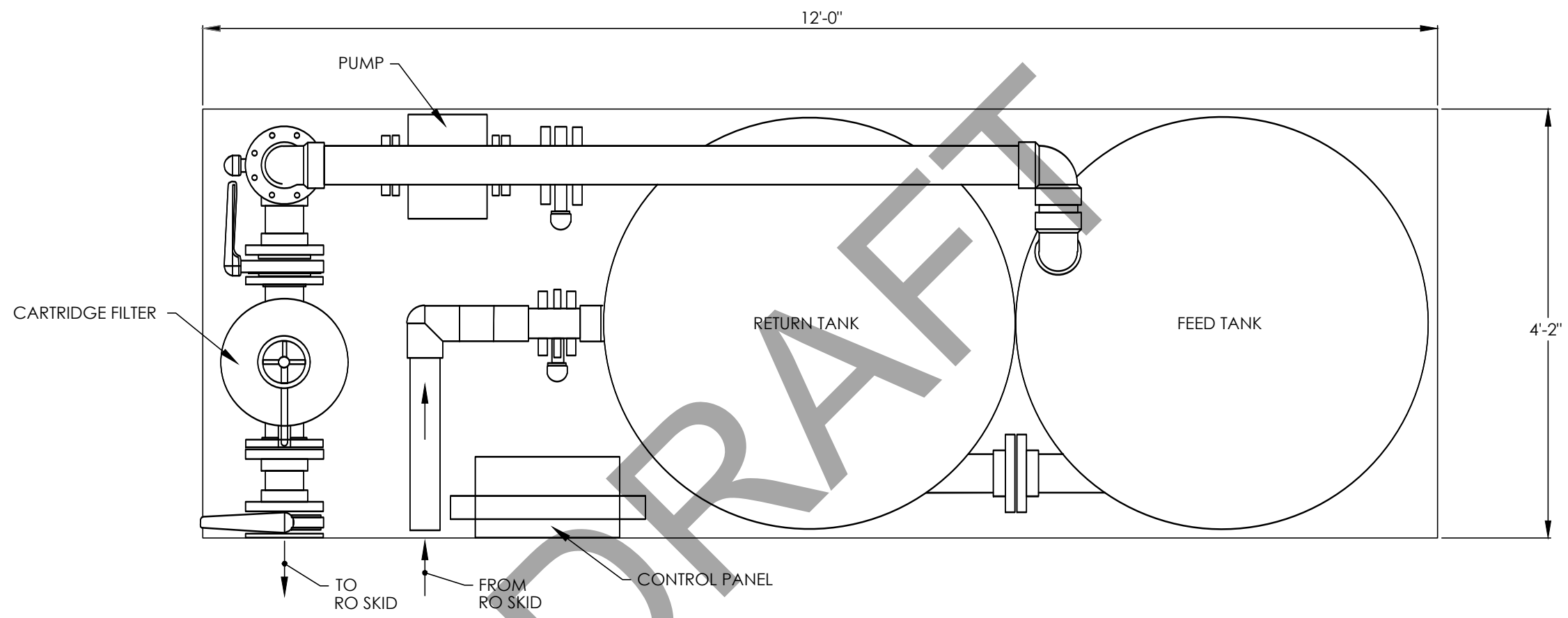
1. Two (2) 500-gallon high density polyethylene (HDPE) cone bottom tanks with carbon steel frame
2. Pump and motor
3. Plumbing and valving to send solution to drain, RO or back to the tank
4. Pressure gauge in pump discharge plumbing
5. Flow meter in recirculation plumbing
6. Cartridge Filter
7. PVC plumbing and connectors
8. Tubing and connectors to connect to RO system

The cleaning fluid is heated to a specific temperature in the mixing tank before being pumped through the membranes, one skid at a time, in the normal direction of flow. Piping connections will allow the cleaning fluid to return to the mixing tanks and re-circulate through the membranes as needed. The CIP system includes a cartridge filter system that removes solid matter from the cleaning fluid.

Concentrated cleaning fluids are mixed with water in the mixing tank to obtain the correct dilution ratio. Cleaning fluids used on the membranes generally include a low pH fluid (such as citric acid) to remove mineral scaling and a high pH fluid (such as caustic) to remove biological foulants from the membranes. Periodically, the membranes may require additional cleaning fluids to restore functionality and recommendations for these cleanings are generally provided by membrane suppliers and/or specially focused chemical suppliers. Many of the cleaning fluids are proprietary and membrane manufacturers typically provide recommended cleaning fluid lists for specific membranes. Membrane cleaning frequencies vary widely, and this is mainly due to contaminants and contaminant concentrations in the RO feedwater. Since there is no available pilot testing information yet, there is no way to accurately predict how often the Town would need to clean the membranes.

However, based on similar systems and water quality, the Town could anticipate a 2-4-months cleaning interval. A conceptual layout of the proposed CIP equipment is included as **Figure 5-7**.

I:\01131\0014\Water\80-Drawings\Figures\FIGURE 5-7 CIP SYSTEM.dwg, 7/28/2020 1:50:35 PM, Cathy Walker



NOT TO SCALE



**BELLEAIR WTP IMPROVEMENTS**

**CONCEPTUAL CIP SYSTEM**



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**FIG 5-7**

#### 5.3.4 Storage/Disinfection

The current treatment process achieves the required disinfection contact time via the existing clearwell. It is important to note that the roof of the clearwell is in very poor condition and presents a safety concern if not repaired. The proposed RO plant configuration eliminates the clearwell and uses the existing ground storage tanks to provide necessary disinfection contact time. Calculations were performed and indicate the smaller ground storage tank can achieve 4-log virus inactivation. It can be inferred that if the smaller tank can provide the necessary disinfection contact time, the larger tank will be adequate for this purpose.

#### 5.3.5 Chemical Injection, Storage and Feed System

Immediately following the booster pump station, proposed chemical injection systems will introduce sodium hypochlorite and ferric chloride to the raw water stream. Water quality data and information from the Town indicate that precipitation of iron and sulfur constituents are the main source of particulates in the source water. These precipitates are either present in the groundwater or form during transfer of the raw water through the existing piping. Sodium hypochlorite is used to convert the compounds to their insoluble forms via chemical oxidation so that they can then be removed with the pressurized media pressure filters. Ferric chloride, or some other type of coagulant, may be added upstream of the filter units to increase particle size and aid with precipitate removal.

Following filtration and before the cartridge filters of the RO process, sodium bisulfite is added for de-chlorination and an anti-scalant is added to reduce the accumulation of precipitants on the RO membranes. Permeate water leaving the RO process is treated with sodium hydroxide to increase pH and alkalinity and is then blended with the RO bypass stream. The blended water is then treated with sodium hypochlorite for disinfection and then with fluoride prior to entering the ground storage tanks. After the disinfected water leaves the ground storage tanks, it is treated with aqua ammonia to form chloramines and to help maintain the required level of disinfectant in the distribution system. Finally, a mixture of poly/ortho phosphates is added prior to entering the distribution system. This chemical prevents corrosion in the piping of the distribution system and in the consumer's plumbing pipe and fixtures.

In general, a 30-day supply of all chemicals is desired. **Table 5-7** summarizes the approximate chemical dosages required along with storage volumes.

**Table 5-7 Chemical Dosages and 30-day Storage Requirements**

Chemical	Dosage (mg/L)	14-day Storage Volume (gal)	Storage Tank Size (gal)
<b>Sodium Hypochlorite (12.5%)</b>	6 pre + 6 post	1333	1500
<b>Ferric Chloride (38%)</b>	2	70	Tote or similar
<b>Sodium Bisulfite (40%)</b>	15	474	Tote or similar
<b>Sulfuric Acid (93%)</b>	20	198	250
<b>Anti-scalant</b>	4	56	Tote or similar
<b>Aqua Ammonia<sup>1</sup></b>	-	89	Tote or similar
<b>Caustic</b>	46	600	650
<b>Polyorthophosphate<sup>1</sup></b>	-	30	Tote or similar
<b>Fluoride<sup>1</sup></b>	-	14	Tote or similar

<sup>1</sup> Based on usage data provided by the Town

### 5.3.6 High Service Pump Station

The existing high service pump station is comprised of four (4) pumps, two (2) of which are 100 HP each and two (2) smaller (jockey) pumps are 40-HP each. This pump station is nearing the end of its useful life and either needs to be rehabilitated or replaced. A new high service pump station is proposed that will be located southeast of the existing 0.5-MG ground storage tank (refer to **Figure 5-8**). The proposed high service pump station will feature the same configuration (two (2) smaller pumps + two (2) larger pumps) as the existing high-service pump station. **Table 5-8** summarizes key high-service pump station design criteria.

**Table 5-8 – High Service Pump Station Design Criteria**

Feature	Design Criteria
<b>Jockey Pumps</b>	2 (1+1)
<b>Main Pumps</b>	2 (1+1)
<b>Jockey Pump Horsepower</b>	40
<b>Main Pump Horsepower</b>	100
<b>Jockey Pump Flow (gpm)</b>	700
<b>Main Pump Flow (gpm)</b>	1400
<b>Pump Head (ft)</b>	162

### 5.3.7 Deep Injection Well

A deep injection well (DIW) has been proposed to dispose of the estimated peak daily RO concentrate flow of 161,000 gpd. The proposed site for the DIW is located south of the Town Public Works Building. It is noted that this PER includes an evaluation of a Closed-Circuit RO (CCRO) process that may reduce RO concentrate flow to less than 50,000 gpd. In this case, a deep injection may not be required because the Town could potentially dispose of the

concentrate via the Pinellas County sanitary sewer system. The CCRO process is evaluated in **Section 5.5**.

### 5.3.8 Sitework

The proposed Chemical Storage, Pressurized Filters, and RO Process Building are shown in **Figure 5-8**. Site improvements include additional dedicated parking and concrete sidewalk adjacent to the RO building. The proposed site plan is only conceptual in nature and its final location would be evaluated further during final design.

### 5.3.9 Yard Piping

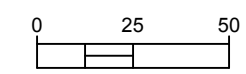
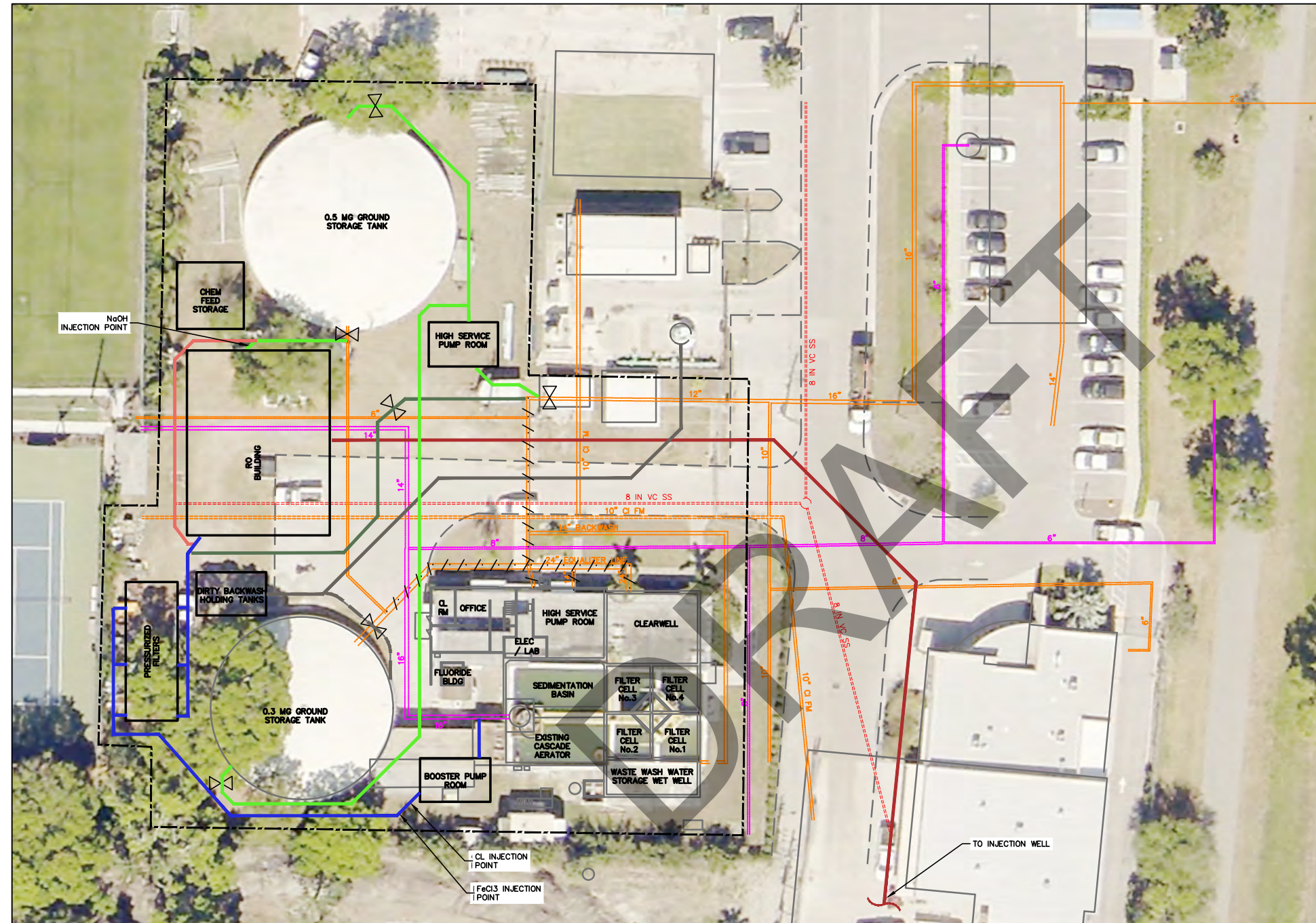
Implementation of the proposed treatment process will substantial changes to the existing yard piping configuration. Most of the existing piping from the 0.3-MG ground storage tank to the existing high service pump station and clearwell will need to be removed or abandoned along with a portion of the distribution line leaving the high service pump station. New yard piping will be required to manifold the existing raw water lines to a common header that would then feed through the new treatment process. The RO permeate will be directed to either the ground storage tanks or the existing clearwell (depending on whether a phased alternative approach is implemented). The concentrate will be directed to the deep injection well using a new pipe. The proposed yard piping configuration includes the following:

1. Existing raw water intake to pressurized media filters
2. Pressurized media filters to the RO process
3. RO bypass line
4. Blended permeate and RO bypass line to the existing line connected to the 0.5-MG ground storage tank
5. 0.5-MG ground storage tank to the new high service pump station
6. 0.3-MG ground storage tank to the new high service pump station
7. High service pump station to existing distribution line
8. Existing distribution line (or high service pump station) to pressurized media filters (for backwash)
9. Dirty backwash tank to existing Pinellas County wastewater lift station
10. RO process to DIW

The proposed yard piping layout is presented in **Figure 5-8**.



i:\01131\0014\Water\80-Drawings\Figures\FIGURES 5-8\_5-9.dwg, 8/3/2020 1:11:33 PM, Cathy Walker



- LEGEND**
- ..... ABANDONED
  - NEW CONCENTRATE
  - NEW RO BYPASS
  - NEW PERMEATE
  - NEW BACKWASH WASTE
  - NEW BACKWASH
  - NEW RAW/FILTERED
  - RW
  - EXIST. PW



**BELLEAIR WTP IMPROVEMENTS**  
**PROPOSED FACILITY SITE LAYOUT**  
**AND YARD PIPING PLAN**



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**FIG 5-8**

## 5.4 Proposed RO Process and Operations Building

The RO building will be a single-story structure, about 2,000 square feet. The rooms will include a RO process area, electrical room, control room, and unisex restroom. A proposed layout is included as **Figure 5-9**. The proposed building will be located southwest of the existing 0.5-MG ground storage tank (refer to **Figure 5-8**). The control room will contain computers and monitor screens necessary for the plant operators to monitor and operate the facility, a work station, and a viewing window out to the process equipment area. The building will be designed in accordance with the current edition of the Florida State Building Code (FBC) and all components and cladding materials will have Florida Product Approval numbers. Building design live and wind loads will be determined in accordance with the aforementioned FBC and the ASCE Standard 7-05 "Minimum Design Loads for Buildings and Other Structures."

The building will also include the following components:

- The foundation will be either a shallow or deep foundation based on Geotechnical investigations that will be performed during detailed design. It is anticipated that based upon existing adjacent structures a shallow foundation system may be utilized, consisting of reinforced concrete spread footings below columns and strip footings below exterior walls. The interior floor will be a concrete slab on grade constructed independent of the footing system.
- The building framing system will consist of structural steel columns and beams supporting the roof. In addition, the frame will provide support for the exterior walls. The roof and floor diaphragms will help provide lateral stability for the building from the wind loads transferred by the exterior walls. The roof will be standing seam metal roof panels supported by structural steel roof joists.

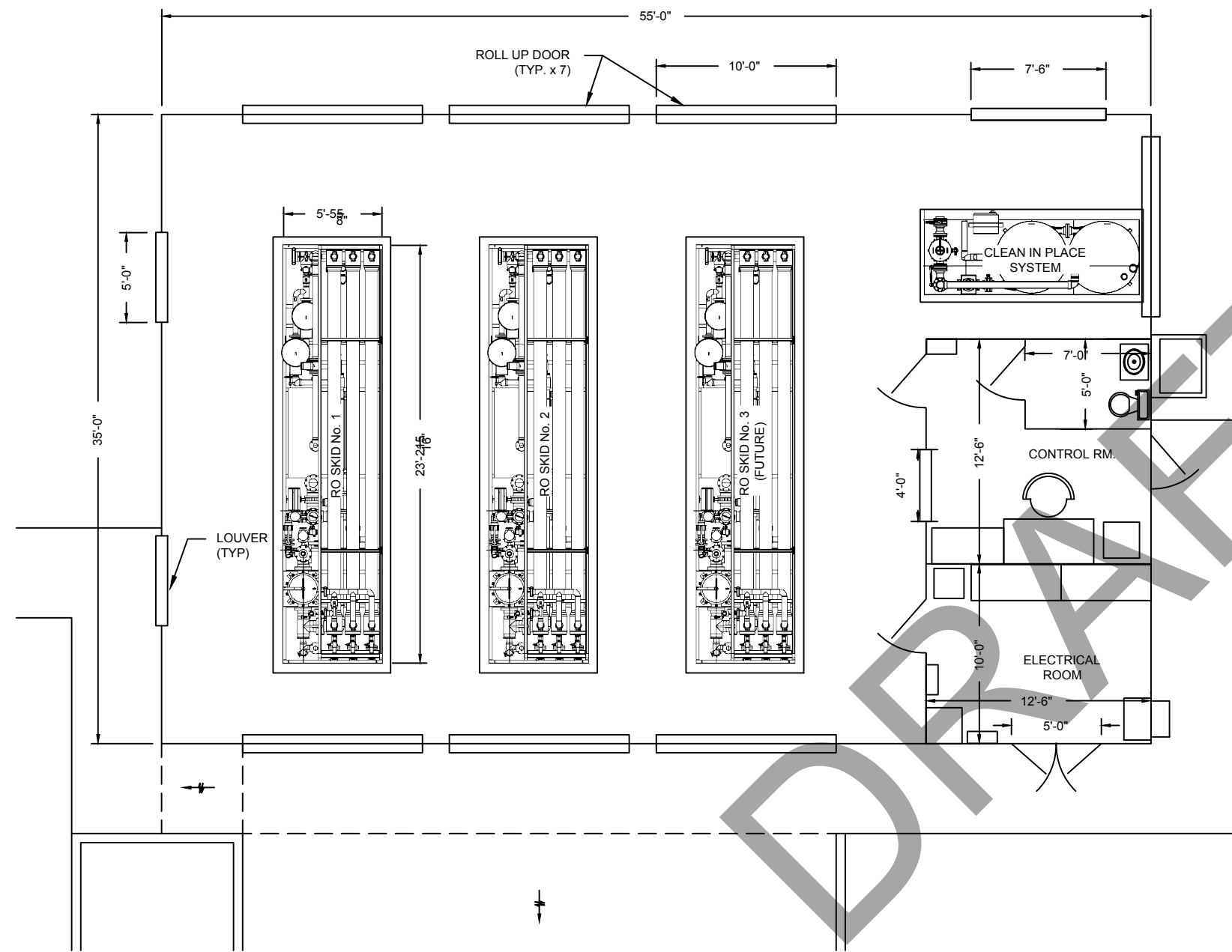


Example building construction, City of North Port  
(Belleair Facility would be one-story)

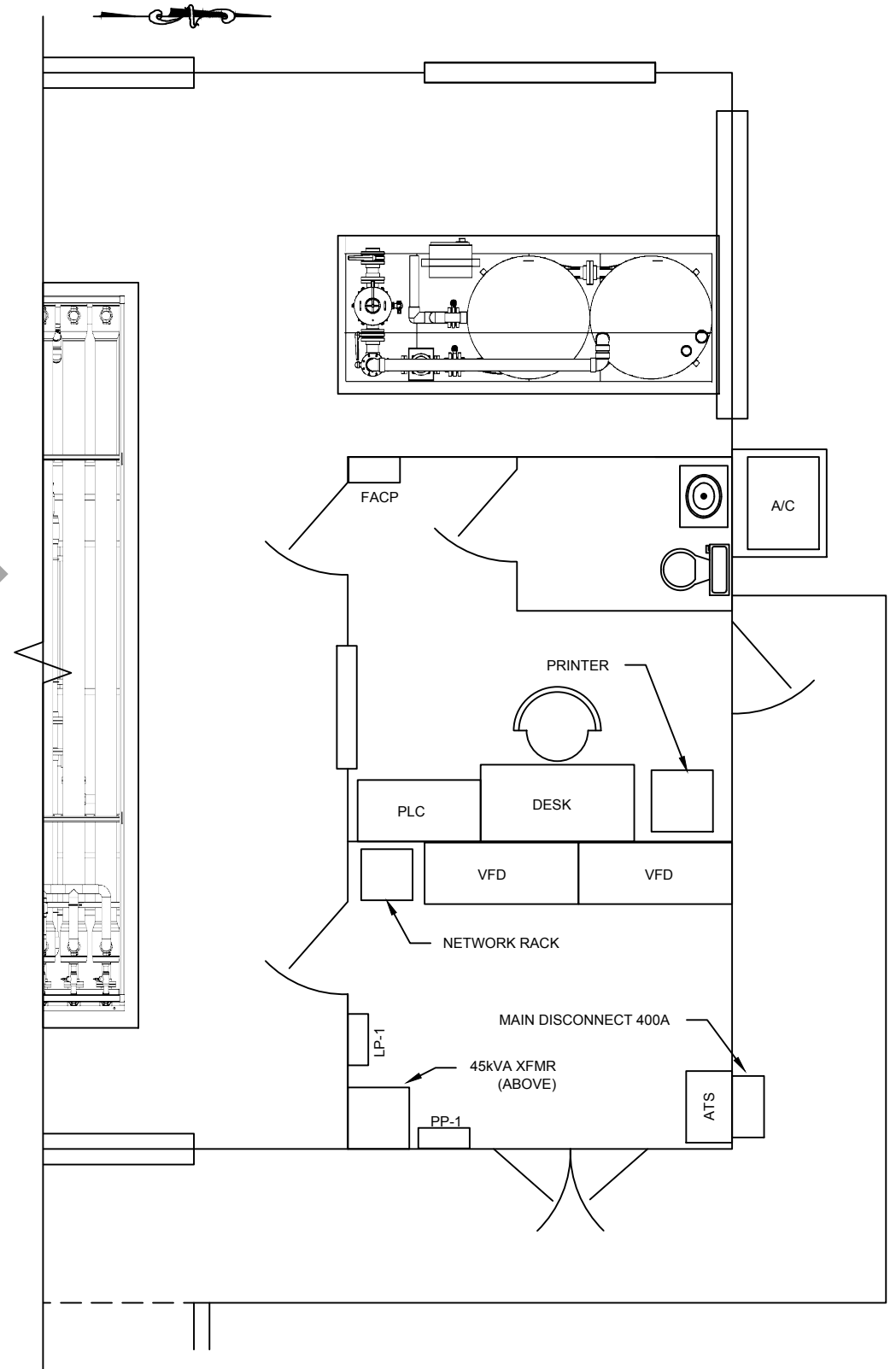
- Stucco, metal panel, and split face block exterior finish.
- FRP Exterior personnel doors with aluminum frames.
- 12' W x 12' H overhead coiling doors centered on each of the RO skids.
- Insulated, aluminum frame, laminated glass exterior windows.
- Example building construction, City of North Port (Belleair Facility would be one-story)
- Interior 8" CMU partitions with a painted finish.
- Aluminum interior doors and frames and doors.
- Suspended ceiling system in the office areas and in conditioned spaces.
- Process area ceiling will be the exposed structural frame and roof deck with a painted finish.
- HVAC will be provided by direct expansion (DX) split systems in all rooms exclusive of the process areas.
- Mechanical ventilation consisting of fixed louvers and exhaust fans will be provided in the process area.
- Hot and cold potable water in the restroom in accordance with the Florida Plumbing Code (FPC).
- Water lines will be CPVC.
- Hot water will be provided by an electric water heater.
- Cold water hose bibs will be provided on the exterior of the building.
- Plumbing fixtures in the restrooms will be vitreous china.
- Floor drains tied to the sanitary waste lines will be provided in the restrooms.

The RO section of the proposed building will contain three (3) RO skids, clean-in-place system, process piping, control panels, electric power and instrumentation conduits, and remote instrumentation units. Space will be provided in the process area to house spare parts, cartridge filters, and membrane cleaning chemicals.

i:\01131\0014\Water\80-Drawings\Figures\FIGURES 5-8 \_5-9.dwg, 8/3/2020 1:11:59 PM, Cathy Walker



**R.O. BUILDING LAYOUT**



**PARTIAL BUILDING LAYOUT**

NOT TO SCALE



**BELLEAIR WTP IMPROVEMENTS**  
**CONCEPTUAL RO BUILDING LAYOUT**



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DWG. No.  
**FIG 5-9**

## 5.5 Proposed Electrical System Modifications

### 5.5.1 Existing Conditions

The existing power service is from Duke Energy using a 300kVA transformer single point of connection that is located next to the maintenance building. The primary 12.47kV feed comes from the street, underground to the transformer. The secondary side is connected to a main disconnect that is mounted on the south wall of the filters. An Automatic Transfer Switch (ATS) next to the disconnect serves as the transfer point of power between normal utility service and generator. There is a generator disconnect mounted next to the ATS on the filter wall.

The utility building is fed from the motor control center (MCC) in the high service pump (HSP) room through a junction box also mounted on the filter wall. This allows for the building to be fed by the WTP generator and transformer.

Electricity billing information was obtained from July 2013 to Jun 2014 and has the combined plant and Utility Building average power usage around 103kW with the usage relatively flat throughout the year. The Duke Power Utility rate schedule is GSLM-2 or General Service Load Management in which the operation of the standby generator can be at Duke Power's request. Power to the facility from Duke Power will normally remain as back up for the standby generation. The customer is given 15 minutes to initiate demand reduction (i.e. turn on the generator) before Duke Power capacity is impacted. This rate schedule and requirement affords the plant a lower power cost. However, changes in the Environmental Protection Agency (EPA) air regulations are likely to have a future impact on generator operation.

Standby power is provided by a Caterpillar Model 3412 - 500kW generator located south of the filter building. This generator provides power to both the plant and Utility building across the street. Based on the billing information the generator is at 21% capacity. This is very low for this size unit, as manufacturers typically specify a generator to run at least 30% to avoid wet stacking.

The fuel supply is a belly tank located under the generator. This tank holds 2,000 gallons of usable fuel, which would give approximately 58 hours of running time at full load. The engine generator was relocated in 2008 from the Town's decommissioned/demolished wastewater treatment facility, has 1567 hours of running time, and appears to have been reasonably maintained.

The existing 800 ampere ATS enclosure (1999) has issues of corrosion and should be cleaned and painted. If not performed in the last five years, a complete cleaning and check out by the manufacturer's representative is recommended. This will require a method of bypassing the switch, but once done, will add confidence in the reliability of the system. In conjunction, it is

recommended that a load test be done on the generator, if one has not been performed within the past five years.

### 5.5.2 Proposed Electrical System Modifications

The initial stages of plant upgrades will involve the installation of a new raw-water booster pump station and associated equipment for the new high-pressure filter system. New pumps will utilize a combined starter and control panel. The combined load is estimated at 40kW and will be fed from a new 80A breaker in the existing MCC.

The proposed RO system will be housed in a separate building northwest of the existing plant. The load of the new system involves the proposed and future load which has a combined estimate of around 450kW at build out. Initially the load for the building is estimated to be approximately 125kW.

Because of the age and connectability of the existing electrical system, it is recommended that a separate utility transformer be installed with its own utility meter. This will allow for the RO building energy costs to be easily monitored and will add redundancy to the existing WTP electrical system. If warranted, a manual transfer system could be installed to link the existing WTP distribution and RO transformer together in case of failure of either.

The existing generator will be utilized to provide standby power to the RO building. A connection from the generator to the RO building will be installed around the existing 0.3 MG ground storage tank (GST). This ductbank will be composed of a concrete cap so as to allow for machinery to run on top of it. A new 800 ampere ATS will be installed to handle all of the RO building loads.

An 18' x 12' electrical room will be included in the RO building. This room will house three variable frequency drives (VFD) for the RO skids, which may be installed in phases. The RO buildings' ATS, 480V distribution panel and a 208V/120V 45kVA three phase transformer along with its power distribution panel will also be installed in this room.

Additional equipment space in the RO electrical room shall be reserved for the new high service pump station soft starters, which will be installed during the final stages of plant upgrades. No new electrical loads are associated with these pumps, as they will be an in-kind replacement of the existing.

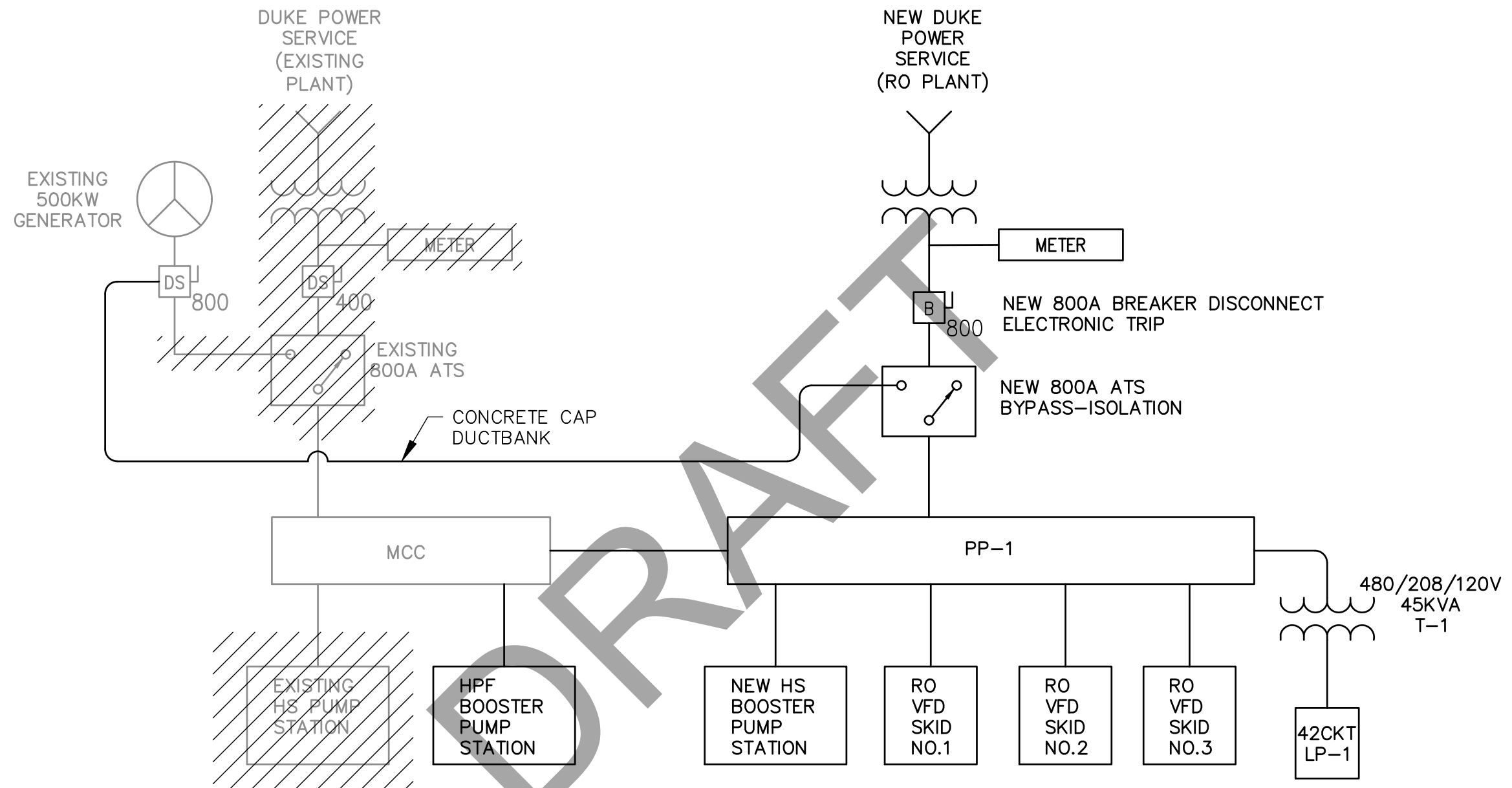
The VFDs will be designed as six-pulse modulation with tuned passive filter to mitigate harmonics. No output filters are necessary as the motors are within 100' of the drives. However, VFD rated cable will be used and the motors, along with being premium efficiency, will be inverter duty rated (Type H insulation). It is also suggested the drives be rated for 50°C ambient

in order to be fully operational if there was a failure of the air conditioning (HVAC) system. There are VFD manufacturers that can produce a 50°C rated drive without derating. We would only recommend those manufacturers; otherwise the Town would be paying for an unneeded larger drive to meet the criteria.

Ventilation and air conditioning are planned to be part of the building for the electrical and control room. Corrosion resistant condensing units will be deployed outside as a split system. Only heat will be provided for the control room.

Surge suppression will be incorporated at different points of the electrical distribution. The incoming service entrance will have surge suppression along with the ATS, power distribution panel and VFD's. In addition, a lightning redirection system is recommended on the exterior of the building and bonded to the buildings grounding grid. A low impedance grounding system is critical to direct the path of lightning to earth. This will be all tin-plated copper with grounding rods deployed to maximize the effectiveness. Once installed, the grounding system will be tested using "fall of potential" method to not only meet National Electric Code (NEC) code (<25ohms) but not to exceed <5 ohms.

Building interior and exterior lighting will be LED for maximum efficiency without a decrease in lumens. This lighting will have long bulb life to minimize maintenance. To meet the Town's needs, any exterior lighting will be "Dark Skies" compliant to reduce unnecessary light pollution. A conceptual One Line Diagram is included as **Figure 5-10**.



I:\01131\0014\Water\80-Drawings\Figures\FIGURE 5-10.dwg, 8/3/2020 3:06:14 PM, Laurel Smith



**BELLEAIR WTP IMPROVEMENTS**  
**CONCEPTUAL ONE LINE DIAGRAM**



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**FIG 5-10**



## 5.6 Proposed Instrumentation Modifications

### 5.6.1 Instrumentation Existing Conditions

The existing control system of the WTP equipment is a combination of Programmable Logic Controller (PLC) and manual relay controls, providing a limited capability of operation. The functions of the high service pumps are controlled by a Siemens S7 – 300 programmable controller. The plant at one time had the ability to monitor the high service pumping via a computer based human machine interface (HMI) software made by Wonderware. However, this functionality is no longer operational, which means even though the Siemens PLC still has automatic control of the HSP, there is no ability for remote intervention without a direct connection to the PLC via a laptop computer and the PLC programming software. The operator must switch control at the VFD to manual in order to adjust the pump speed when needed. This has not typically been an issue, as the PLCs automatic functionality of pump speed and number of pumps based on pressure has been functioning properly and is the primary control for the HSP.

The filter system control is done using manual relay timers and a Square D Sy/Max micro controller for filter backwash sequencing. The Sy/Max controller was in production until the late 1990s but is now considered beyond end of life by the manufacturer. As such, parts are no longer available through standard retail channels and only available through from third parties. The control panel appears to be in good condition with little to no corrosion and can continue to be used as long as there is no failure of the processor.

The gas chlorine system is controlled using a Depolox 3 Total Chlorine analyzer and the Superior Autovalve chlorinator controller. The controller uses a compound loop of flow and desired chlorine residual setpoint (currently 3.0 mg/l) to control the chlorine dosage. In addition, as the distribution system uses chloramines for complete disinfection, the ammonia flow pace control is performed utilizing the same output from the Autovalve controller. The ratio of ammonia to chlorine is manually adjusted via the stroke settings on the individual pumps (currently 4:1). The Depolox 3 analyzer has been discontinued by Wallace and Tiernan/Evoqua with replacement using the Depolox 400 M, however, the same measurement sensors can be used with the new unit.

The chemical addition of hydrofluosilicic acid and orthopolyphosphate are manually controlled to turn on when the well system is activated. There is no automatic adjustment for flow or setpoint. Stroke and pump speed are manually set based on the dosage requirements. As the

well flow is constant, adjustment of these chemicals is rare. However, if wells are taken off line or because of failure, manual adjustment to these settings would become necessary.

The well field pumps are controlled for speed through individual VFDs located at the well. The speed is adjusted manually when needed. Remote start/stop control for the wells are available through an Internet web-based hosted cellular telemetry system which also provides monitoring of the well pump run status. Access is provided through a computer web browser which connects to the host server through a network connection. While basic start/stop control and status monitoring is provided, the operation is limited and availability is contingent on a recurring monthly fee. As more information collection is desired for the wells and the process complexity requires a higher level of control, it will eventually be necessary to replace this capability with a more robust control system.

### 5.6.2 Proposed Control System Changes

The proposed control system for the new facilities will be designed in stages to help control the implementation costs but with the level of monitoring and control needed from a modern control system. The existing plant control system will mostly remain as is, unless budget can be allocated for it to be incorporated. The control system will be designed around a centralized point for monitoring and control of the plant. The City has utilized a no-cost 50-point license for VTScada by Trihedral as a SCADA software package for monitoring a selection of water quality parameters. The operations staff has been pleased with the performance and the recognition that the software does not require a server computer for operation. Expanding this software with a higher point count license will allow the operations to expand the monitoring and control ability of the plant. Additionally, the software can provide data collection of key plant parameters for trending and reporting, as well as other features such as local and remote alarm notification.

Communication from the SCADA software to fields control panels should be handled over Ethernet communications, with all devices connected together using a managed network switch. The control system will initially be designed for the RO skid system, starting with an open architecture (PLC) that communicates using Ethernet/IP. This main PLC will be housed in a new control room constructed as part of the RO building. The PLC will also be expandable for future hardwired inputs or Ethernet communication to other control panels. This will allow the PLC to communicate with the RO skid PLC(s) and any future PLC that is installed in the plant. The control logic for the RO skids will reside on a PLC that is part of the RO system. This PLC would monitor and control the RO system operation. Options for this control system are to have an individual PLC on each skid or one single PLC mounted in a local control panel near the skids. Having a PLC on each skid as opposed to one for both skids adds another level of redundancy however, this can be decided during the design phase.

The new pressurized filter system will require a separate set of controls with a local control panel for each vessel. Operators on the enclosure will allow local operation including backwashing of the filter. Connection of these control panels to the PLC network, either as remote input/outputs or as standalone system will allow remote operation and monitoring of the system.

The HMI will consist of an industrial computer with a touchscreen monitor installed on the main PLC enclosure and a desktop workstation in the control room. The workstation will act as the primary means of monitoring and control with the screen on the main PLC enclosure as backup. If the budget allows; additional plant systems could be added, during design, to the main PLC in the RO building. The HSP currently is controlled using a Siemens S7. Utilizing this PLC and having it connected to the RO system PLC will require a third party "gateway" along with programming changes. This could introduce issues during construction and implementation, as the gateway translates between protocols. Building a system around the S7 could also be an issue, as support for it is limited and the cost of upgrading the processor to an Ethernet based unit is substantial. The cost of this approach would be enough to warrant the installation of a new PLC and remote I/O rack in the HSP building to monitor and control the high service pumps, chemical feed systems and generator.

With the implementation of the RO process, monitoring and control of the wells will also be increased. It will be important to operate the wells at flow rates controlled to maintain the process, monitor the downstream pressure to recognize piping issues and monitor the well levels to maintain water quality conditions. This will require a greater capability level than available in the current system. While the full capability is not required until the actual RO process is placed into operation, it is important to recognize these needs and to incorporate them into the different phases. Ideally, additional instrumentation can be added during the earlier phase, including adding a transmitter to the flow meter and adding new transmitters for discharge pressure and well level. A new local control panel that also functions as the telemetry station can be added as part of the later stages, replacing the existing telemetry. With the decreasing costs associated with cellular communications, it would be recommended to utilize an imbedded input/output Ethernet based PLC processor to handle the local controls and telemetry reporting to the plant. Remote start/stop controls would remain and automatic speed control would be added to better operate the wells locally and as a total well field system.

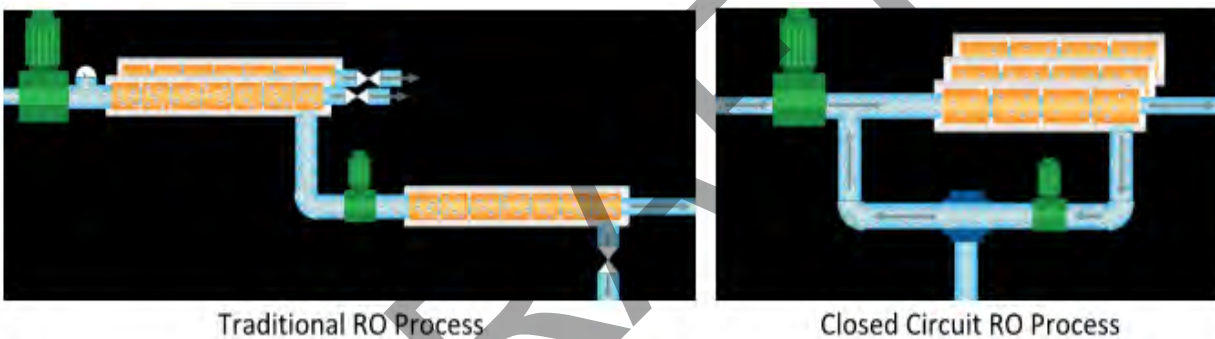
## **5.7 High-Recovery Closed Circuit RO System Alternative**

The following sections evaluate a potential CCRO system and compare it with a traditional RO system.

### 5.7.1 High-Recovery Closed Circuit RO (CCRO) Description

A CCRO system offers the ability to operate the RO process and achieve higher overall RO recovery that may approach 90 to 95-percent. CCRO systems feature equal feed and permeate flow rates during normal operation mode. At a software-based set point, the system automatically flushes out all of the concentrate, and then returns to its normal operation mode. The concentrate flush is triggered by the CCRO operating software, based on any combination of flow, concentration, pressure and additional set points. During the concentrate flush step, the system continues to be fed and generates permeate, while concentrate is pushed out of the system in one sweep. Concentrate is recirculated to the membrane feed and recovery increases with each concentration cycle. **Figure 5-11** shows a side by side comparison of a traditional 2-stage RO and a CCRO configuration. As with the 2-stage system, it is recommended that pilot testing be performed to confirm CCRO design criteria and to optimize the CCRO treatment process.

**Figure 5-11 Closed Circuit RO and 2-Stage RO Comparison**



*Images courtesy of Desalitech, Inc.*

### 5.7.2 CCRO System Benefits

The primary benefit of the CCRO system is the ability to achieve a higher recovery rate as compared to a 2-stage system. Higher recovery results in higher RO permeate production and lower RO concentrate production (potentially as low as 40,000 gpd). As a result, the Town may have an opportunity to discharge the CCRO concentrate to the Pinellas County sanitary sewer system instead of constructing and operating a DIW. In addition, the CCRO system requires less cleaning than traditional RO systems which in turn extends RO membrane life.

### 5.7.3 CCRO System Drawbacks

System challenges of the CCRO system include higher permeate TDS concentrations than those of a 2-stage RO system.. Because of this, a higher percentage of CCRO permeate would be needed while blending with CCRO bypass flow. Also, the CCRO system requires higher pressures than those for traditional RO and the characteristic translates to higher power consumption.

### 5.7.4 CCRO Evaluation Summary

As previously discussed, there are several differences between 2-stage RO and CCRO systems. **Table 5-9** summarizes the advantages and disadvantages associated with traditional and CCRO systems.

**Table 5-9 – RO Process Alternative Comparisons**

Alt	Description	Advantages	Disadvantages
1	Conventional RO System	<ul style="list-style-type: none"> <li>• Lower capital and O&amp;M costs</li> <li>• Known effectiveness (lower risk)</li> <li>• Higher quality RO permeate</li> </ul>	<ul style="list-style-type: none"> <li>• Uses more groundwater</li> <li>• Requires deep injection well</li> <li>• Requires more space</li> </ul>
2	High-Recovery (CCRO) System	<ul style="list-style-type: none"> <li>• Uses less groundwater</li> <li>• May not require deep injection well</li> <li>• Uses less space – may result in smaller RO process building design</li> </ul>	<ul style="list-style-type: none"> <li>• Higher capital and O&amp;M costs</li> <li>• Less available information for prior installations (higher risk)</li> <li>• Higher energy usage costs</li> </ul>

Based on our evaluation of a potential CCRO system, we recommend the following:

- 1) Evaluate the potential discharge (approximately 40,000 gpd) of CCRO concentrate into the County’s sanitary sewer system.
- 2) Determine capital costs and fees for the CCRO flow
- 3) Compare present value of traditional and CCRO systems
- 4) Evaluate risks and consider performing a pilot study for a CCRO system

## 6.0 Potential Use of Existing Treatment Plant Systems

With the exception of the ground storage tanks and some yard piping, the proposed facility will not reuse the existing equipment. Yard piping to be reused can be seen in **Figure 5-8**.

## 7.0 Phased Implementation Plan

Considerations for a phased design/construction process were evaluated to help the Town with budgeting and providing for capital outlays over an extended period. Of special note, the phasing is designed to minimize capital costs in the earlier phases to provide only the treatment processes needed to achieve chloride and TDS levels of 225 mg/L and 450 mg/L, respectively. The phasing plan includes recommended improvements that would be designed and constructed in four (4) phases. **Table 7-1** shows the improvements needed if the complete facility was constructed together and the improvements needed for each of the proposed phases.

**Table 7-1 – Phased Alternative Implementation**

Task/Improvement	All at Once	Phase 1	Phase 2	Phase 3	Phase 4
Mobilization/Demobilization	✓	✓	✓	✓	✓
Sitework	✓		✓	✓	
Clearwell Roof Rehabilitation		✓			
Booster Pump Station	✓		✓		
Pressurized Filters and Backwash Holding Tank	✓		✓		
Chemical Building	✓		✓		
Chemical Systems	✓		✓	✓	
RO System	✓			✓	✓
RO Building	✓			✓	
Deep Injection Well	✓ <sup>1</sup>			✓ <sup>1</sup>	
Additional Well(s)	✓	✓		✓	
Well Rehabilitation	✓		✓		
High Service Pump Station Rehabilitation		✓			
New High Service Pump Station	✓				✓
Yard Piping	✓		✓	✓	✓
Electrical	✓	✓ <sup>2</sup>	✓	✓	✓
Instrumentation	✓	✓ <sup>2</sup>	✓	✓	✓

- 1) The inclusion of the deep injection well is contingent upon implementation of traditional or closed-circuit RO system  
 2) At proposed well site(s)

### 7.1 Phase 1 – Clearwell Roof Rehabilitation and Additional Well

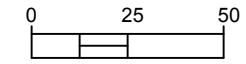
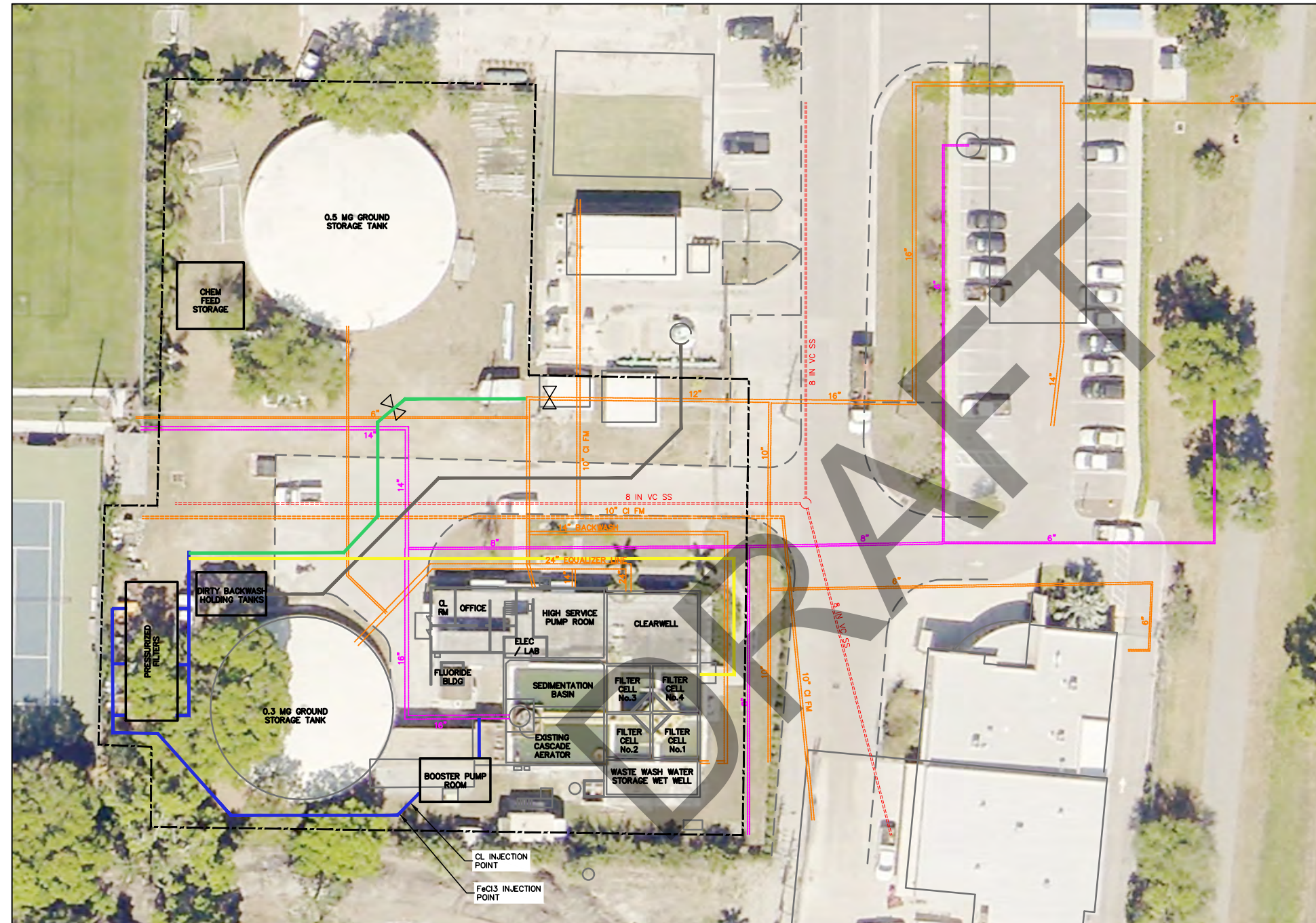
Phase 1 involves the addition of one (1) new production well (Well 11), repairs / rehabilitation at the existing wells, rehabilitation of the existing clearwell, and the existing high service pump station. Due to the rapidly deteriorating water quality from Well 3, we recommend that the Town take this well off-line. The lost flow from Well 3 will be made up from the proposed Well 11. In subsequent Phases 2 and 3, the existing clearwell would remain in operation to achieve the required contact time for disinfection. A tank inspection was performed by Liquid Engineering Corporation on December 11, 2019 and a summary report was provided. In general, the summary report indicated that the clearwell roof is in poor condition and needs to be repaired or replaced. The summary report also indicated that the existing tank walls and bottom are in fair condition would require that tank concrete and coating issues be addressed for continued utilization. A sketch of the roof repair has been included in **Appendix D**. Updates to the high service pump station would include pump rehabilitation, along with updates to exiting electrical, mechanical and instrumentation/control systems. It is noted that the existing walkway running from the sedimentation tank to the filters is in poor condition and that repairs are needed to allow for temporary operation for Phase 1.

## 7.2 Phase 2 – Pressurized Dual-Media Filters

In Phase 2, the pressurized dual-media filters would be installed along with the proposed booster pump station. All of the ground water would need to flow through the filters; therefore, the existing sedimentation basin, cascading aerator, and filtration system will be decommissioned, while use of the existing clearwell will be retained. **Figure 7-1** shows the proposed yard piping layout for this phase. In addition, a small building for chemical storage and feed systems needed for this phase would be constructed.

DRAFT

I:\01131\0014\Water\80-Drawings\Figures\FIGURES 7-1 THRU 3.dwg, 7/29/2020 10:34:54 AM, Cathy Walker



LEGEND

- NEW BACKWASH WASTE
- NEW BACKWASH
- NEW FILTERED TO CLEARWELL
- NEW RAW/FILTERED
- RW
- EXIST. PW



**BELLEAIR WTP IMPROVEMENTS**  
**PHASE 2 SITE LAYOUT AND YARD PIPING PLAN**



**TOWN OF BELLEAIR**  
 901 PONCE DE LEON BOULEVARD  
 Florida 34104  
 (727) 588-3769

DWG. No.  
**FIG 7-1**

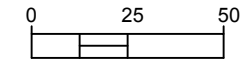
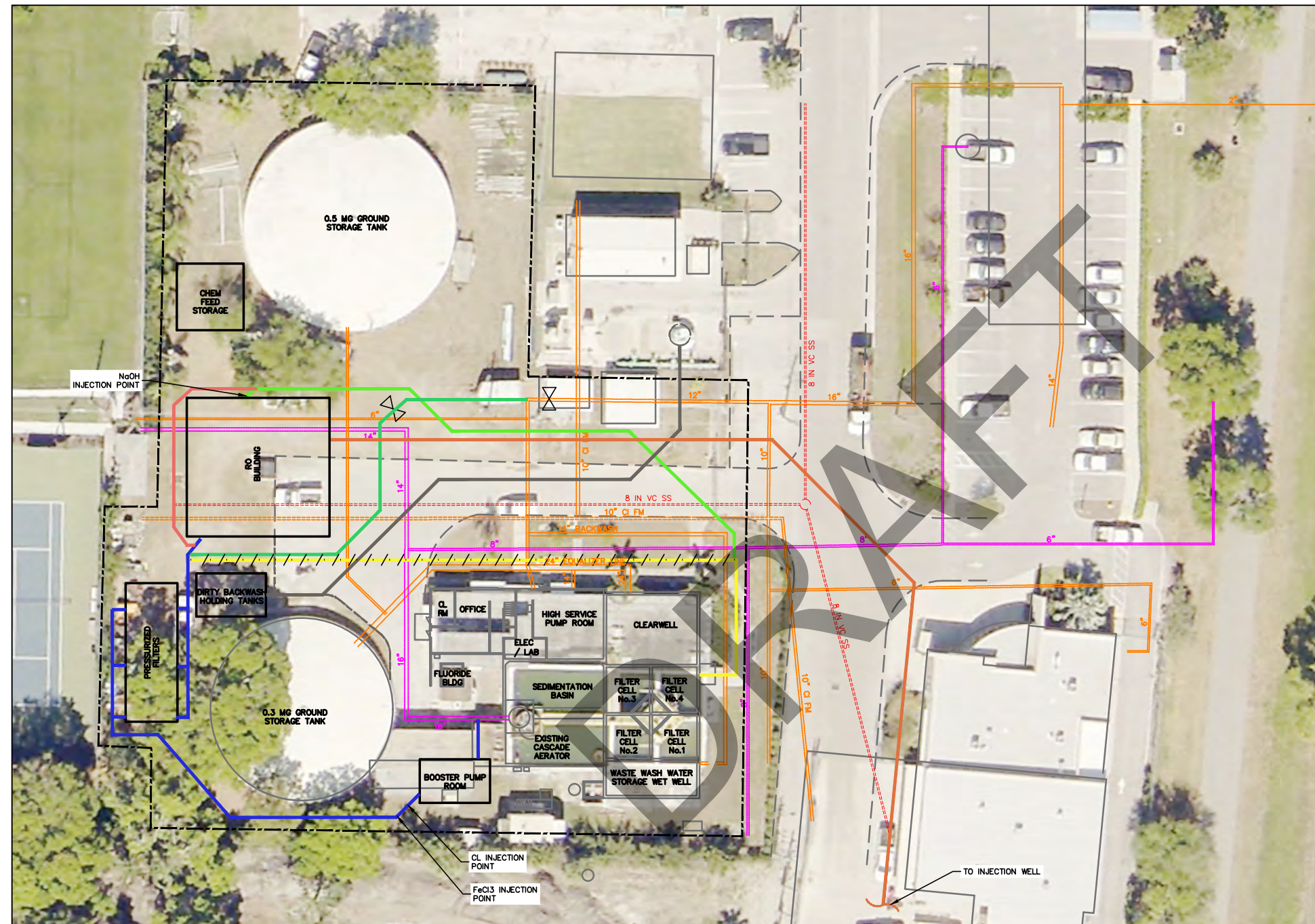


### 7.3 Phase 3 – RO System

During Phase 3, the installation of two (2) RO skids will take place. Due to current raw water quality trends, it is anticipated that approximately 50% of the water coming from the pressurized dual-media filters would be directed to the RO system when placed into operation. The modified high-service pump station and clearwell continues during this phase. **Figure 7-2** shows the proposed yard piping layout for this phase.

DRAFT

I:\01131\0014\Water\80-Drawings\Figures\FIGURES 5-8 \_7-1 THRU 3.dwg, 7/29/2020 11:37:53 AM, Cathy Walker



- LEGEND**
- ABANDONED
  - NEW CONCENTRATE
  - NEW RO BYPASS
  - NEW PERMEATE
  - NEW BACKWASH WASTE
  - NEW BACKWASH
  - NEW FILTERED TO CLEARWELL
  - NEW RAW/FILTERED
  - RW
  - EXIST. PW



**BELLEAIR WTP IMPROVEMENTS**  
**PHASE 3 SITE LAYOUT AND**  
**YARD PIPING PLAN**



**TOWN OF BELLEAIR**  
 901 PONCE DE LEON BOULEVARD  
 Florida 34104  
 (727) 588-3769

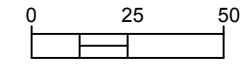
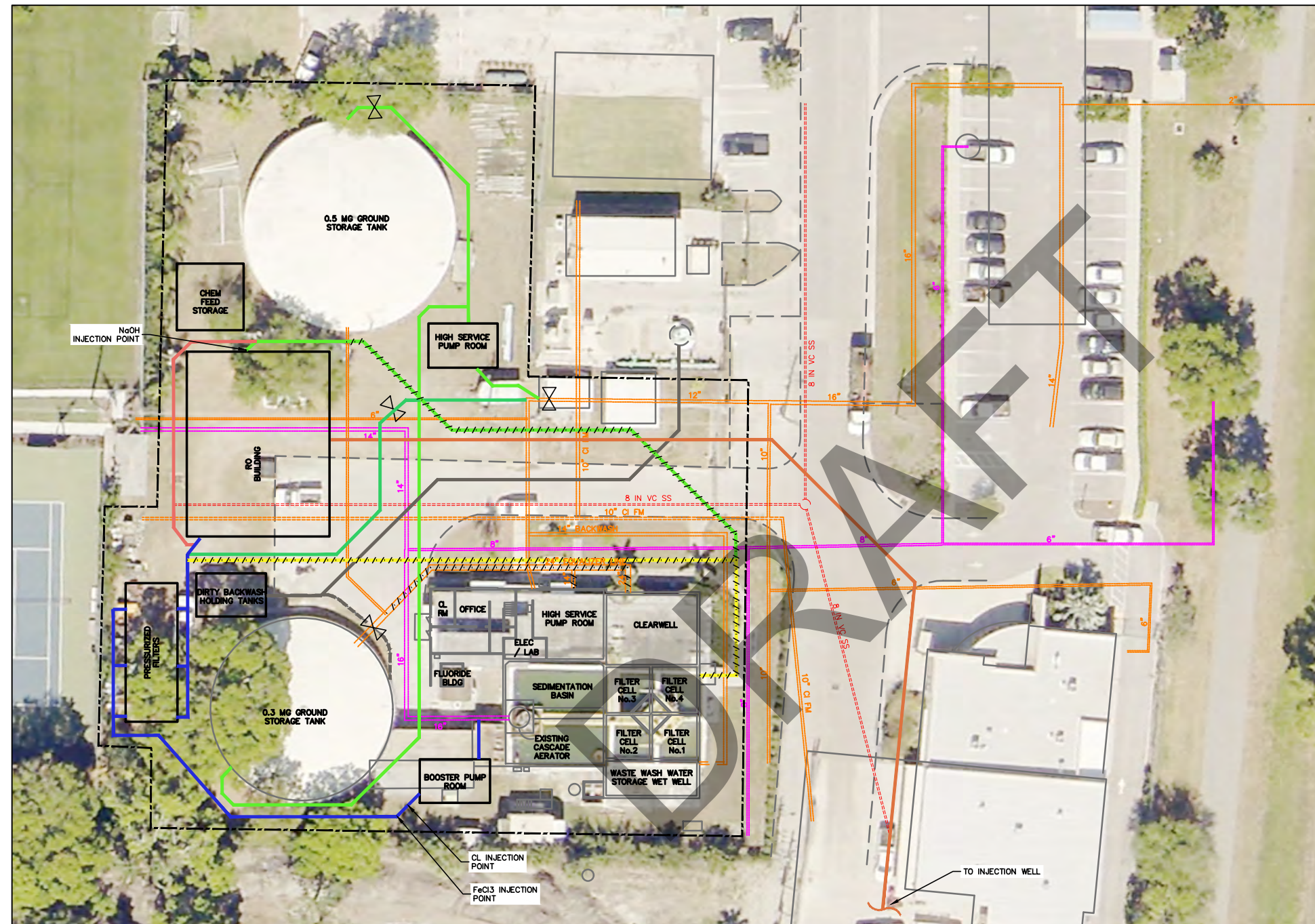
DWG. No.  
**FIG 7-2**

#### 7.4 Phase 4 – RO System Buildout and New High-Service Pump Station

In the last phase, the final build-out for the RO system is installed along with a new high service pump station. The existing high-service pump station and clearwell will be decommissioned. Required chlorine contact time will be achieved in the existing ground storage tanks. **Figure 7-3** shows the proposed yard piping layout.

DRAFT

i:\01131\0014\Water\80-Drawings\Figures\FIGURES 7-1 THRU 3.dwg, 7/29/2020 10:36:24 AM, Cathy Walker



- LEGEND**
- · · · · ABANDONED
  - NEW CONCENTRATE
  - NEW RO BYPASS
  - NEW PERMEATE
  - NEW BACKWASH WASTE
  - NEW BACKWASH
  - NEW FILTERED TO CLEARWELL
  - NEW RAW/FILTERED
  - RW
  - EXIST. PW



**BELLEAIR WTP IMPROVEMENTS**  
**PHASE 4 SITE LAYOUT AND YARD PIPING PLAN**



**TOWN OF BELLEAIR**  
 901 PONCE DE LEON BOULEVARD  
 Florida 34104  
 (727) 588-3769

DWG. No.  
**FIG 7-3**

## 8.0 Engineer’s Opinion of Probable Construction Cost

### 8.1 Construction Costs

Estimated capital costs were developed for the traditional RO and CCRO supply and treatment systems using similar project costs, bid tabulations, manufacturer/supplier provided estimates, and previous experience with similar facilities.

#### 8.1.1 Proposed Facility

**Table 8-1** summarizes the capital costs for the two (2) potential RO supply and treatment systems.

**Table 8-1 – Proposed Facility Cost Summary**

Item	Traditional RO System	High-Recovery CCRO System
<b>Mobilization/Demobilization</b>	\$175,000	\$175,000
<b>Sitework</b>	\$325,000	\$325,000
<b>Booster Pump Station</b>	\$142,000	\$142,000
<b>Pressurized Filters and Backwash Holding Tank</b>	\$969,000	\$969,000
<b>Chemical Building</b>	\$84,000	\$84,000
<b>Chemical Systems</b>	\$362,000	\$362,000
<b>RO System</b>	\$1,044,000	\$1,488,000
<b>RO Building</b>	\$434,000	\$434,000
<b>High Service Pump Station</b>	\$338,000	\$338,000
<b>Deep Injection Well</b>	\$750,000	-----
<b>Yard Piping</b>	\$500,000	\$500,000
<b>Electrical</b>	\$738,000	\$677,000
<b>Instrumentation</b>	\$554,000	\$508,000
<b>Upper Floridian Wells (6) and Well Rehabilitation</b>	\$1,450,000	\$1,450,000
<b>Total Construction Cost</b>	\$7,865,000	\$7,452,000
<b>Contingency (30%)</b>	\$2,360,000	\$2,236,000
<b>Sub Total</b>	\$10,225,000	\$9,688,000
<b>Engineering - Design and Legal (15%)</b>	\$1,534,000	\$1,454,000
<b>TOTAL PROJECT COST</b>	<b>\$11,759,000</b>	<b>\$11,142,000</b>

#### 8.1.2 Phased Implementation Plan

### 8.1.2.1 – Phase 1 – Clearwell Roof Rehabilitation and Additional Well

Table 8-2 summarizes the capital costs associated with Phase 1.

**Table 8-2 – Phase 1 Capital Cost Estimate**

Item	Cost
Mobilization/Demobilization	\$20,000
Clearwell Roof Rehab	\$125,000
Upper Floridian Well	\$250,000
6" Raw Water Transmission Line	\$20,000
Total Construction Cost	\$415,000
Contingency (30%)	\$125,000
Sub Total	\$540,000
Engineering - Design and Legal (15%)	\$81,000
<b>TOTAL PROJECT COST</b>	<b>\$621,000</b>

### 8.1.2.2 – Phase 2 – Pressurized Dual-Media Filters

Table 8-3 summarizes the capital costs associated with Phase 2.

**Table 8-3 – Phase 2 Capital Cost Estimate**

Item	Cost
Mobilization/Demobilization	\$100,000
Sitework	\$75,000
Booster Pump Station	\$142,000
Pressurized Filters and Backwash Holding Tank	\$969,000
Chemical Building	\$84,000
Chemical Systems	\$201,000
High Service Pump Station Rehabilitation	\$169,000
Yard Piping	\$268,000
Electrical	\$313,000
Instrumentation	\$235,000
Upper Floridian Well and Transmission Main	\$250,000
Well Rehabilitation	\$450,000
Total Construction Cost	\$3,166,000
Contingency (30%)	\$950,000
Sub Total	\$4,116,000
Engineering - Design and Legal (15%)	\$618,000
<b>TOTAL PROJECT COST</b>	<b>\$4,734,000</b>

### 8.1.2.3 - Phase 3 – RO System

Table 8-4 summarizes the capital costs associated with Phase 3.

**Table 8-4 – Phase 3 Capital Cost Estimate**

Item	Cost
Mobilization/Demobilization	\$120,000
Sitework	\$250,000
RO System (2 skids)	\$660,000
RO Building	\$325,000
Chemical Systems	\$161,000
Deep Injection Well	\$750,000
Yard Piping	\$207,000
Electrical	\$480,000
Instrumentation	\$360,000
Upper Floridian Wells (2)	\$500,000
<b>Total Construction Cost</b>	<b>\$4,438,000</b>
Contingency (30%)	\$1,332,000
<b>Sub Total</b>	<b>\$3,813,000</b>
Engineering - Design and Legal (15%)	\$744,000
<b>TOTAL PROJECT COST</b>	<b>\$5,701,000</b>

### 8.1.2.4 – Phase 4 – RO System Addition and New High Service Pump Station

Table 8-5 summarizes the capital costs associated with Phase 4.

**Table 8-5 – Phase 4 Capital Cost Estimate**

Item	Cost
<b>Mobilization/Demobilization</b>	<b>\$75,000</b>
<b>Sitework</b>	<b>\$200,000</b>
<b>RO System (1 skid)</b>	<b>\$384,000</b>
<b>RO Building</b>	<b>\$109,000</b>
<b>High Service Pump Station</b>	<b>\$338,000</b>
<b>Yard Piping</b>	<b>\$128,000</b>
<b>Electrical</b>	<b>\$167,000</b>
<b>Instrumentation</b>	<b>\$125,000</b>
<b>Total Construction Cost</b>	<b>\$1,526,000</b>
Contingency (30%)	\$458,000
<b>Sub Total</b>	<b>\$1,984,000</b>
Engineering - Design and Legal (15%)	\$298,000
<b>TOTAL PROJECT COST</b>	<b>\$2,282,000</b>

## 8.2 Operational and Maintenance Cost

### 8.2.1 Proposed Facility

Operational and Maintenance costs were determined from manufacturer proposals, industry standards, information from local municipalities, and information from equipment providers. **Table 8-6** summarizes the estimated annual operations and maintenance costs associated with the proposed facility.

**Table 8-6 – Annual Operations and Maintenance Cost**

Item	Traditional RO System	High-Recovery CCRO System
<b>Power<sup>1</sup></b>	\$74,000	\$98,000
Chemicals	\$146,000	\$146,000
RO System – Cartridge Filters	\$10,000	\$10,000
RO System – Membrane Replacement	\$10,000	\$8,000
Major Equipment Replacement	\$140,000	\$140,000
<b>TOTAL ANNUAL COST</b>	<b>\$380,000</b>	<b>\$402,000</b>

1. Based on power cost of \$0.1025/kW-hr per [www.electricitylocal.com/states/florida/belleair/](http://www.electricitylocal.com/states/florida/belleair/)

## 9.0 Summary and Recommendations

This PER was performed and considered the condition of the existing WTP, along with projected increases in chloride and TDS levels in the Town's groundwater. If the Town wishes to continue with potable water production, a reverse osmosis (RO) treatment plant is recommended to reduce chloride and TDS concentrations and to address ongoing operational, maintenance and safety concerns at the WTP.

Besides developing preliminary engineering requirements for the proposed RO WTP, this PER was intended to provide the Town with capital and operational / maintenance (O&M) costs for the proposed WTP. Also, a potential phased implementation plan was developed to spread capital costs over an extended period. This phased implementation plan would install new processes and modifications only as needed as chloride and TDS levels increase over time.

The use of a conventional 2-stage RO system was compared with a high-recovery CCRO system to determine if the CCRO system provides enough benefit, such as removing the requirement for a deep injection well, to offset potential risks. Based on the evaluations performed during the development of this PER, our recommendations include the following:



1. Determine the viability of disposing of RO concentrate into the Pinellas County’s sewer system
2. Identify potential locations for six (6) potential well sites and evaluate their viability
3. Compare costs of Town producing water from the proposed RO plant versus long-term bulk purchase from Pinellas County
4. Compare cost impacts for a potential phased implementation plan
5. Perform pilot study for the conventional or CCRO system

Based on the evaluations performed in this PER, McKim & Creed recommends that the Town either begin constructing the proposed RO treatment plant (all at once or in phases) or decommissions the existing WTP and begins utilizing potable water from Pinellas County by the end of calendar year 2021.

**Table 9-1** summarizes the total costs associated with the two (2) alternatives.

**Table 9-1 Cost Summary**

Approach	Total Capital Cost <sup>1</sup>
<b>Alternative 1 – All-At-Once</b>	\$11,759,000
<b>Alternative 2 – Four Separate Phases</b>	\$13,383,000

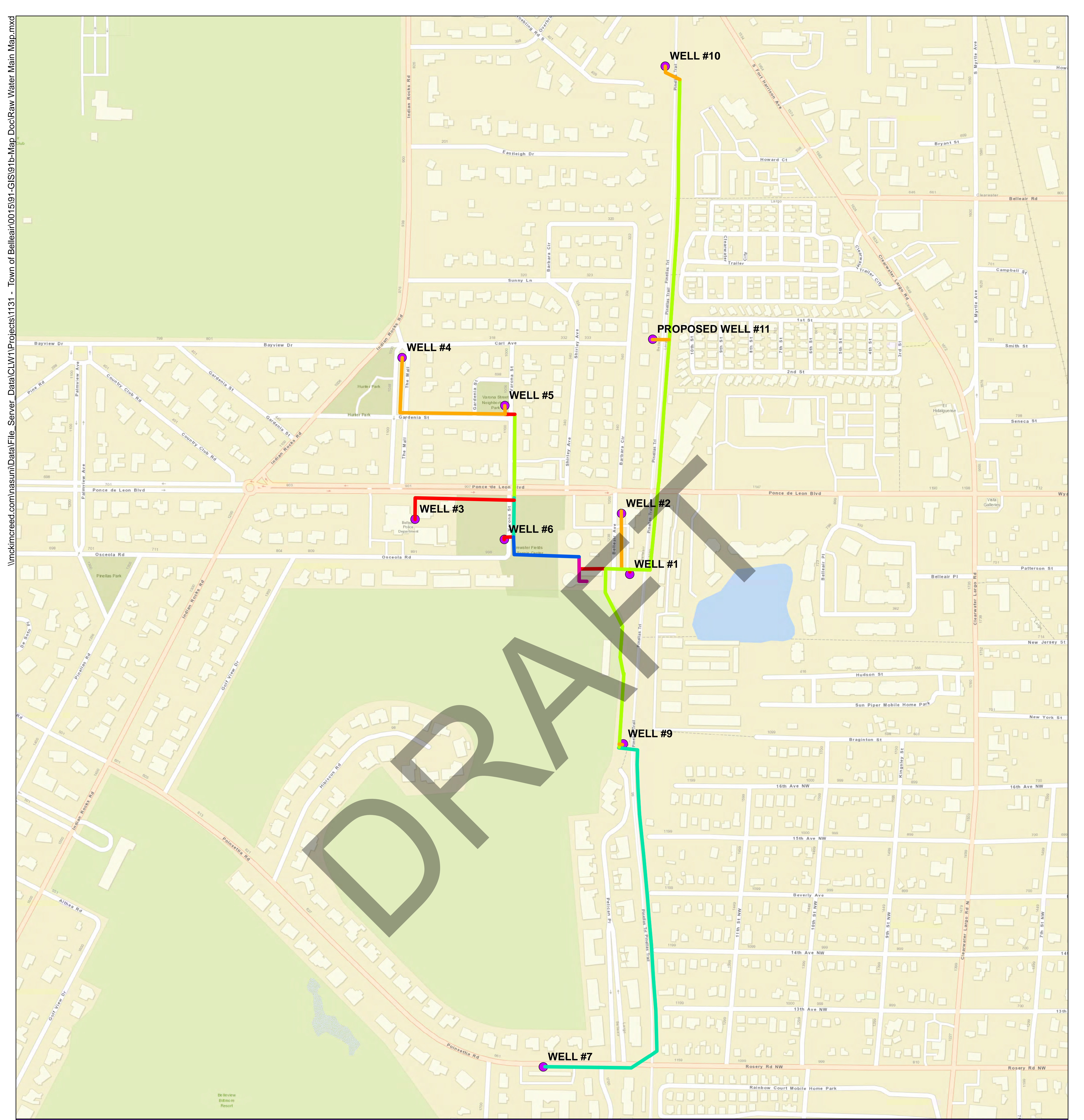
1. Based on 2020 construction costs

**References**

*Preliminary Engineering Report for the Town of Belleair Water Treatment Plant Improvements (Cardno, McKim & Creed, Arcadis, 2015)*

# **APPENDIX A**

DRAFT



# Raw Water System

Map Disclaimer: This product is for informational purposes only and is based on some unverified information provided by others. This product has not been prepared for nor is it suitable for legal, engineering, or surveying purposes. It represents only the approximate relative location of property boundaries. McKim & Creed, Inc. assumes no liability or damages due to inaccuracies, errors

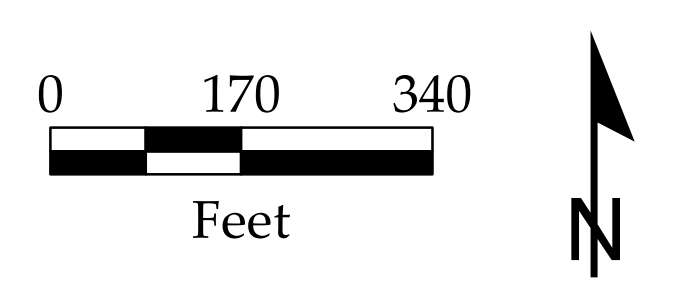
<b>Raw Water Main</b>		10	<b>Well</b>	
<b>DIAM</b>		12		
		14		
		8		
		6		
		16		



**MCKIM & CREED**  
 1365 Hamlet Avenue  
 Clearwater, FL 33756-3331  
 Tel: (727) 442-7196

**Raw Water Main Map**  
 Prepared For: TOWN OF BELLEAIR

Jul. 2020 Pg 1 of 1



# **APPENDIX B**

DRAFT



## MEMO

**TO:** Phil Locke, P.E.  
**FROM:** Jeff Trommer, P.G.  
**SUBJECT:** Town of Belleair Supply Well Evaluation  
**DATE:** July 31, 2020

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WSP reviewed and evaluated data collected by Applied Drilling Engineering, Inc. (ADE) from six Town of Belleair (Town) public supply wells. Data collected in the field by ADE, included geophysical logs, downhole video surveys, specific capacity test data, and observations of the pumping equipment. The data were reviewed to evaluate the following:

1. Physical condition of the well casings, boreholes, and pump equipment;
2. Local hydrogeologic conditions;
3. Production capability and producing zone profile, and;
4. Water-quality characteristics.

Specific capacity tests were performed by ADE using the existing production pumps. The wells were pumped for one hour at their operational pumping rates. Static water levels were measured manually with an electric water level tape for five to 10 minutes prior to the start of pumping. Water levels were measured at intervals from 2 to 13 minutes during pumping. Drawdown is calculated as pumping water level – static water level. Specific capacity is calculated as pumping rate (gpm)/drawdown (ft).

Upon completion of the specific capacity test the production pump was removed from the well, and downhole geophysical logging were performed by Advanced Borehole Services (ABS). ABS performed the following suite of geophysical logs: caliper, natural gamma, long/short normal electrical resistivity, fluid resistivity, temperature, and flow. Fluid resistivity, temperature and flow will be performed under static and pumping conditions. The wells were pumped at 100 gallons per minute (gpm) during the pumping logs. A downhole video survey was also performed to the entire depth of the well.

WSP was not on site during the well evaluation work. Data sheets, geophysical logs, and the video survey were provided to WSP by ADE. Data were evaluated as follows:

- 1) Specific capacity results were reviewed to document the productivity of the wells to assist with recommendations for operational pumping rates.
- 2) The borehole geophysical logs were reviewed and analyzed as follows:

- a. The caliper log provides an outline of the casing and borehole profile and diameter. The log is reviewed to verify diameters and depths of the casing and open borehole section of the well. The log was reviewed to identify intervals of massive non-fractured rock, which typically indicate low water productivity, versus highly-irregular-diameter borehole sections, which indicate potential fracture zones or solution cavities that are water-producing zones.
- b. The long/short normal electrical resistivity log measures changes in the electrical resistivity of the rock formation to assist in identifying changes in rock lithology between the formations comprising the aquifer. For example, changes in resistivity mark the contact between the Tampa Limestone and Suwannee Limestone. This mark is used to differentiate upper and lower Zone A. The electrical resistivity log, along with the spontaneous potential (SP) log, also provides indications of porewater salinity variations in the open interval of the well.
- c. The natural gamma log responds to changes in natural gamma activity in the formation material. This log most notably responds to the presence of confining layer clays separating the Surficial and Upper Floridan aquifers. The top of the Upper Floridan aquifer is marked by a significant decrease in gamma activity at the contact between the overlying clay confining layer and the top of limestone.
- d. The fluid resistivity, specific conductance, and temperature (water-quality logs) measure the resistivity and temperature of the water in the borehole. Under static conditions, the logs measure the variation in water resistivity and temperature with depth. Static logs that show no change with depth indicate that water across the open borehole has a relatively consistent water quality across the depth of the well. Static logs that show a decrease in fluid resistivity/increase in specific conductance with depth indicate that the water in the well increases in salinity with depth. Static logs that show an abrupt change in fluid resistivity or temperature indicate the presence of a specific flow zone with a change in water quality in the aquifer. Changes in fluid resistivity/specific conductance and temperature measured under pumping conditions indicate the presence of flow zones in a well and provide an estimate of the net fluid resistivity being produced during pumping, which can be a relative indication of TDS concentration.
- e. The static flow log is run down and up the borehole, and the differential in flow velocity between the two runs is used to identify whether there is a significant vertical flow gradient within the well. The pumping flow log is used to identify zones where flow is entering the well (producing zones).
- f. The video survey is used to observe the condition of the casing and borehole wall. The presence of fractures and solution cavities in the formation can be observed, and the presence of bacterial growth, mineral scale, and partial bore hole blockages can be identified.

The following sections provide a summary of our evaluation of the test data provided by ADE. Well evaluation data sheets provided by ADE are included in **Appendix A**. Borehole geophysical logs are provided in **Appendix B**.



## Well No. 2

A summary of the well construction information and specific capacity test results is shown in **Table 1**. The specific capacity of 922 gpm/ft is extremely high for the area, and is indicative of flow from a large fracture or solution cavity.

**Table 1 – Well No. 2 Data Summary**

Casing Diameter (inches)	12
Casing Depth (ft bls)	57
Total Well Depth (ft bls)	282
Specific Capacity (GPM/ft)	922

The borehole geophysical logs in **Appendix B** provided the following information:

- Casing diameter and depth, and total well depth shown in **Table 1** were obtained from the caliper log. The caliper log shows a fracture/cavity interval from 64 to 79 feet bls. An additional fracture/cavity is shown from 190 to 195 feet bls. The remainder of the borehole shows an irregular profile between 12 and 15 inches in diameter.
- The gamma log shows high activity peaks from approximately 15 to 55 feet bls indicative of a clayey layer overlying the Upper Floridan aquifer. Intermittent gamma activity peaks are indicated at 115, 176, 195, and 206 feet bls, which are likely due to clayey layers within and at the base of upper Zone A.
- The long-short normal resistivity log shows a consistent response from 75 to 175 feet bls indicative of the limestone comprising upper Zone A. The decrease in resistivity from 175 to 215 feet bls is due to the higher clay content in the limestone in this interval. The increase at 215 feet bls marks the top of lower Zone A. The decrease and more consistent response below 220 feet bls is indicative of the occurrence of higher TDS water in lower Zone A.
- The static water-quality logs show a shift at 67 feet bls indicative of primary flow zone from the fracture/cavity interval observed in the caliper log from 64 to 79 feet bls. The logs are then consistent to a depth of 227 feet bls, below which specific conductance increases and fluid resistivity decreases, indicating an increase in total dissolved solids (TDS) in lower Zone A.
- The static up hole and downhole flow logs suggests a slight down hole gradient below 205 feet bls and no vertical gradient above 205 feet bls.
- The pumping flow logs show that all the flow occurs from the cavity at 64 to 79 feet bls.

- The pumping water quality log also indicates that the primary producing zone occurs between 64 and 79 feet bls. The pumping flow log also shows the transition to higher TDS water below 227 feet bls, but does not indicate that the higher TDS water is flowing upward under pumping conditions.
- The video survey shows 12-inch steel casing to a depth of 57 feet bls. The static water level was at 41 feet bls. An interval of fractured rock creating a cavity was observed from 65 to 69 feet bls. The open borehole has a thin layer of apparent microbial growth that had scrape marks through it from the caliper tool arms. The borehole wall became smoother from 115 to 178 feet bls due to a change to softer, clayey limestone as indicated on the gamma log. The apparent cavity observed on the caliper log at 190 feet bls is an interval of enlarged borehole and not an actual open cavity or fracture. Visibility decreased significantly below 263 feet bls, indicating no significant flow of water into the borehole below this depth.

As noted in the well evaluation data sheet (**Appendix A**) the production pump and drop pipe had to be cut to be removed and ADE ordered a replacement pump, motor, and drop pipe. Details of the new pump are provided on the well evaluation data sheet.

According to information provided by McKim & Creed, chloride concentrations in well No. 2 showed an increasing trend in chloride concentration at a rate of 1.7 mg/L/year from 1979 through 2013. The well was pumped at 295 gallons per minute for the specific capacity test, which is significantly higher than the rate at which the well is operated (125 gpm). The specific capacity of the well is high enough to support an operational rate greater than this, however higher pumping rates could increase the rate of chloride increase in the well. Recommended pumping rates are discussed later in the Technical Memorandum. Although the well appears to have a thin layer of bacteriologic growth on the borehole, it does not appear to be adversely affecting well performance or water quality. There are no recommendations for well maintenance or rehabilitation activities for well No. 2 at this time.

## Well No. 5

A summary of the well construction information and specific capacity test results is shown in **Table 2**. The specific capacity of 11.7 gpm/ft is low for the area, and is indicative of flow from primary porosity in the limestone.

**Table 2 – Well No. 5 Data Summary**

Casing Diameter (inches)	6
Casing Depth (ft bls)	80
Total Well Depth (ft bls)	257
Specific Capacity (GPM/ft)	11.7



The borehole geophysical logs in **Appendix B** provided the following information:

- Casing diameter and depth, and total well depth shown in **Table 2** were obtained from the caliper log. The caliper log shows an irregular profile between 10 and 15 inches in diameter, with no significant fractures or cavities.
- The gamma log shows high activity peaks from approximately 18 to 80 feet bls indicative of a clayey layer overlying the Upper Floridan aquifer. The gamma activity peaks from 87 to 100 feet bls and 187 to 206 feet bls, are likely due to clayey layers within and at the base of upper Zone A, respectively.
- The long-short normal resistivity log decreases from 130 feet to 205 feet bls due to increasing clay content within and at the base of upper Zone A. The increase at 215 feet bls marks the top of lower Zone A.
- The static water-quality logs are consistent to a depth of 230 feet bls, below which specific conductance increases slightly and fluid resistivity decreases slightly, indicating the start of an increase in total dissolved solids (TDS) in lower Zone A.
- The static up hole and downhole flow logs do not indicate a consistent vertical flow gradient in the borehole.
- The pumping flow log shows that most of the flow is produced between 80 and 90 feet bls.
- The pumping water quality logs also show that most of the flow is produced between 80 and 90 feet bls.
- The video survey shows a 6-inch diameter PVC liner to a depth of 80 feet bls. The static water level was at 43 feet bls. The water in the open borehole was cloudy. A section of cable was observed from 85 to 108 feet bls. Minor bacteriological growth was noted on the borehole wall. No significant fractures or cavities were observed. Water clarity improved between 200 and 220 feet bls, suggesting that this interval of the borehole may be a minor producing zone.

As noted in the well evaluation data sheet (**Appendix A**) minor replacements were made to parts in the existing Cert-lok drop pipe. Since the specific capacity of this well is only 11.7 gpm/ft, flow to the well is via primary porosity, and there is observable bacteriological growth on the bore hole wall, the performance of this well can likely be improved by well acidizing.

## Well No. 6

A summary of the well construction information and specific capacity test results is shown in **Table 3**. The specific capacity of 37.6 gpm/ft is moderate for the area, and is indicative of flow from primary porosity and minor fractures in the limestone.

**Table 3 – Well No. 6 Data Summary**

Casing Diameter (inches)	12
Casing Depth (ft bls)	72
Total Well Depth (ft bls)	302
Specific Capacity (GPM/ft)	37.6

The borehole geophysical logs in **Appendix B** provided the following information:

- Casing diameter and depth, and total well depth shown in **Table 3** were obtained from the caliper log. The caliper log shows an irregular profile between 12 and 24 inches in diameter. Areas of enlarged borehole due to fractured or washed-out limestone are indicated from 208 to 213, 233 to 240, and 261 to 264 feet bls.
- The gamma log shows high activity peaks from approximately 16 to 56 feet bls indicative of a clayey layer overlying the Upper Floridan aquifer. The gamma activity peaks from 185 to 219 feet bls, are likely due to clayey layers in the base of upper Zone A.
- The long-short normal resistivity log shows increased resistivity from 125 to 180 feet bls, which generally indicates higher porosity limestone. The decrease in resistivity 175 to 220 feet bls is due to increasing clay content in the lower portion of upper Zone A. The increase at 220 feet bls marks the top of lower Zone A.
- The static water-quality logs are consistent to a depth of 200 feet bls, below which specific conductance increases slightly and fluid resistivity decreases slightly, indicating the start of an increase in total dissolved solids (TDS) in lower Zone A. Specific conductance increases rapidly below 235 feet bls, indicating the transition to higher TDS concentration water in lower Zone A.
- The static up hole and downhole flow logs do not indicate a consistent vertical flow gradient in the borehole.
- The pumping flow log shows the primary flow zone is from 72 to 100 feet bls.
- The pumping water quality logs also indicate that the primary producing zone occurs at approximately 100 feet bls. The pumping flow logs also show the transition to higher TDS water below 210 feet bls, but do not indicate that the higher TDS water is flowing upward under pumping conditions.
- The video survey shows 12-inch diameter steel casing to a depth of 72 feet bls. The static water level was at 41 feet bls. The water in the open borehole was cloudy. Large moldic porosity was observed from 72 to 90 feet bls. The area of enlarged borehole from 183 to 265 feet bls appears to be due to larger blocks of limestone that broke from the borehole wall during drilling, and not open fractures or cavities. No significant fractures or cavities were observed.

As noted in the well evaluation data sheet (**Appendix A**) the production pump and drop pipe had to be cut to be removed and ADE ordered a replacement pump, motor, and drop pipe. Details of the new pump are provided on the well evaluation data sheet. There are no recommendations for well maintenance or rehabilitation activities for well No. 6. Chloride data shown in a report By HSW (2014) indicate that chloride concentrations began to increase at a higher rate in 2004. Recent data suggest that this rate of increase has continued through the present time. Well No. 6 has the deepest total depth of all the wells evaluated, which could be the main cause of the higher rate of chloride concentration increase. Since most of the production from the wells appears to be from the interval between 72 and 100 feet bls, the lower portion of the well could potentially be back-plugged to reduce the connection to the deeper, higher chloride concentration water. While back-plugging could slightly reduce the specific capacity of the well, this would not significantly impact the performance of the well due to the relatively low operating pumping rate of 120 to 140 gpm. Therefore, consideration should be given to back-plugging the well to approximately 180 feet bls.

## Well No. 7

A summary of the well construction information and specific capacity test results is shown in **Table 4**. The specific capacity of 21.6 gpm/ft is low for the area, and is indicative of flow from primary porosity in the limestone.

**Table 4 – Well No. 7 Data Summary**

Casing Diameter (inches)	12
Casing Depth (ft bls)	80
Total Well Depth (ft bls)	144
Specific Capacity (GPM/ft)	21.6

The borehole geophysical logs in **Appendix B** provided the following information:

- Casing diameter and depth, and total well depth shown in **Table 4** were obtained from the caliper log. The caliper log shows an irregular profile between 12 and 13 inches in diameter. No indications of fractures or cavities were observed.
- The gamma log shows high activity peaks from approximately 20 to 78 feet bls indicative of a clayey layer overlying the Upper Floridan aquifer.
- The long-short normal resistivity log shows consistent resistivity to the total depth of the borehole, indicating consistent lithology.
- The static water-quality logs are consistent to a depth of 100 feet bls, below which specific conductance increases slightly and fluid resistivity decreases slightly, indicating a slight increase in total dissolved solids (TDS).

- The static up hole and downhole flow logs indicate a downward vertical flow gradient in the borehole.
- The pumping flow log shows the primary flow zone is from 110 to 125 feet bls.
- The pumping water quality logs also indicate that the primary producing zone occurs at approximately 110 to 125 feet bls.
- The video survey shows 12-inch diameter steel casing to a depth of 80 feet bls. The static water level was at 46 feet bls. The water in the open borehole was slightly cloudy. Moldic porosity was observed from 80 to 95 feet bls. Visibility decreased with depth below 105 feet bls. No significant fractures or cavities were observed.

As noted in the well evaluation data sheet (**Appendix A**) the production pump and drop pipe had to be cut to be removed and ADE ordered a replacement pump, motor, and drop pipe. Details of the new pump are provided on the well evaluation data sheet. There are no recommendations for well maintenance or rehabilitation activities for well No. 7.

## Well No. 9

A summary of the well construction information and specific capacity test results is shown in **Table 5**. The specific capacity of 3,300 gpm/ft is extremely high for the area, and is indicative of flow from a large cavity in the limestone.

**Table 5 – Well No. 9 Data Summary**

Casing Diameter (inches)	12
Casing Depth (ft bls)	88
Total Well Depth (ft bls)	145
Specific Capacity (GPM/ft)	3,300

The borehole geophysical logs in **Appendix B** provided the following information:

- Casing diameter and depth, and total well depth shown in **Table 5** were obtained from the caliper log. The caliper log shows an interval of apparent fractured rock from 88 to 125 feet bls.
- The gamma log shows high activity peaks from approximately 25 to 46 feet bls indicative of a clayey layer overlying the Upper Floridan aquifer.
- The long-short normal resistivity log shows low resistivity from 74 to 87 feet bls, indicative of a clayey limestone layer. Resistivity increases from 87 to 95 feet bls and then remains consistent to the bottom of the borehole, indicating an increase in porosity.

- The static water-quality logs show a shift between 85 and 91 feet bls, which is likely due to the influence of the flow zone identified from the caliper log.
- The static up hole and downhole flow logs do not indicate a consistent vertical flow gradient in the borehole.
- The pumping flow log shows the primary flow zone is from 87 to 91 feet bls.
- The pumping water quality logs also indicate that the primary producing zone occurs at approximately 87 to 91 feet bls.
- The video survey shows 12-inch diameter steel casing to a depth of 88 feet bls. The static water level was at 45 feet bls. The water in the open borehole was slightly cloudy. Extensive, large moldic porosity and solution cavities were observed from 90 to 125 feet bls. The solution cavities are the source of the extremely high productivity in this well.

As noted in the well evaluation data sheet (**Appendix A**) the production pump and drop pipe were re-installed with no changes or repairs. There are no recommendations for well maintenance or rehabilitation activities for well No. 9.

## Well No. 10

A summary of the well construction information and specific capacity test results is shown in **Table 6**. The specific capacity of 3,300 gpm/ft is extremely high for the area, and is indicative of flow from a large cavity in the limestone.

**Table 6 – Well No. 10 Data Summary**

Casing Diameter (inches)	12
Casing Depth (ft bls)	75
Total Well Depth (ft bls)	146
Specific Capacity (GPM/ft)	29.8

The borehole geophysical logs in **Appendix B** provided the following information:

- Casing diameter and depth, and total well depth shown in **Table 6** were obtained from the caliper log. The caliper log shows fractured interval from 77 to 90 feet bls.
- The gamma log shows high activity peaks from approximately 18 to 66 feet bls indicative of a clayey layer overlying the Upper Floridan aquifer. The gamma activity increase from 105 to 120 feet bls, is likely due to a clayey layer within upper Zone A.
- The long-short normal resistivity log shows a slight increase in resistivity below 120 feet bls, indicating an increase in porosity in the lower portion of the borehole.

- The static water-quality logs show a shift above 90 feet bls, which is likely due to the influence of the flow zone identified from the caliper log.
- The static up hole and downhole flow logs do not indicate a consistent vertical flow gradient in the borehole.
- The pumping flow log shows the primary flow zone is from 77 to 82 feet bls.
- The pumping water quality logs also indicate water quality is relatively consistent with depth in the borehole.
- The video survey shows 12-inch diameter steel casing to a depth of 74 feet bls. The static water level was at 27 feet bls. The water in the open borehole was slightly cloudy. Well-developed moldic porosity was observed from 85 to 95 feet bls. An apparent small cavity was observed at 86 feet bls. Abundant moldic porosity was observed in the interval from 125 feet to 144 feet bls.

As noted in the well evaluation data sheet (**Appendix A**) the production pump and drop pipe had to be cut to be removed and ADE ordered a replacement pump, motor, and drop pipe. Details of the new pump are provided on the well evaluation data sheet. There are no recommendations for well maintenance or rehabilitation activities for well No. 10.

## Recommended Pumping Rates

McKim & Creed prepared a flow and raw water quality blend analysis as part of a reverse osmosis treatment plant evaluation. The base analysis used reported current pumping rates for the seven existing wells (note: well 3 was not included in the well evaluation program due to lack of accessibility, and is not included in this report) are shown in **Table 7**. These rates have been established by the plant operators over the past several years to manage the rate of increase of chloride concentrations, and to address wells with higher iron concentrations. If the Town elects to continue with their current water supply and treatment system, the pumping rates should continue to be managed as currently done. However, if additional treatment to reduce iron and TDS concentrations is added, some wells may be able to be operated at higher rates. Our evaluation of potential pumping rates is based on specific capacity of the wells, existing chloride concentration trends from HSW (2014), and well locations. Increasing the pumping rates may increase the rate at which chloride concentrations increase. Projections of chloride increases at higher pumping rates were not included in this evaluation as the data needed to make these projections are not available. Increases in chloride concentration can be accounted for in the design of the reverse osmosis facility and have been factored into the McKim & Creed raw water quality blend analysis. The increases were based on current chloride concentration trends and well pumping rates.

Higher rates of increases could potentially exceed the design parameters for the reverse osmosis plant, increase the TDS concentration of the concentrate, which could limit the concentrate disposal options, or adversely impact other users of the Upper Floridan aquifer near the Towns wells. Therefore, if the Town were to consider increasing pumping rates to the potential rates shown in **Table 7**, chloride concentrations should be monitored closely to avoid undesirable increases.

Jeffrey M. Trommer, P.G.

Lead Hydrogeologist



**Table 7 – Well Pumping Rate Summary**

Well No.	Current Pumping Rate (gpm)	Potential Rate (gpm)	Comments
2	125	220	High specific capacity Assumes iron removal and RO treatment
5	150	150	Low specific capacity
6	120	120	Current rate of chloride concentration increase
7	150	150	Low specific capacity
9	140	220	High specific capacity Assumes RO treatment
10	120	120	Location relative to existing users



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**APPENDIX A**

**Well Evaluation Summary Reports**



## Town of Belleair

### Well Evaluations

### Well 2

March 24, 2020 and April 15, 2020

Time	Static Water Level
9:09	42.65
9:12	42.65

### Pump Test Data at 295 gpm

Time	Dyn. Water Level
9:15	42.73
9:17	42.78
9:19	42.79
9:22	42.80
9:25	42.81
9:33	42.82
9:48	42.85
9:55	42.95
10:08	42.96
10:15	42.96
10:17	42.97

The old pump had to be removed with a torch, and a replacement Pump. Motor, and Certa-lok drop pipe, was ordered.

Static and Dynamic Geophysical Logs were then conducted and a Downhole Video

The Wellhead was modified with a 4" coupling and the new Pump installed.

#### New Pump Details

Franklin 7.5 Hp 3ph 460v Motor

Grundfos 230S75-3-BB Pump End

(2) Stainless Steel Crossover Subs

50' (2x20' & 1x10') of 4" Certa-lok Drop Pipe with an extra Coupling

60' of Pump Wire with one Splice Kit

65' of 3/16 Stainless Steel Safety Cable with Clamps

Town of Belleair  
 Well Evaluations

Well 5

April 28, 2020 – April 29, 2020

Time	Static Water Level
11:02	44.77
11:09	44.75

Pump Test Data at 150 gpm

Time	Dyn. Water Level
11:15	57.60
11:20	57.58
11:25	57.65
11:30	57.62
11:45	57.64
12:00	57.64
12:15	57.61

Recovery

Pump Off	Water Level
5 minutes	44.92
8 minutes	44.64
10 minutes	44.65
12 minutes	44.64

Pulled 70' of 3" Certa-lok drop pipe from the 6" Well. Need to replace the two plastic Certa-lok cross overs subs with stainless steel ones and replace one leaking 3" Certa-lok coupling. We also need to add a stainless steel safety cable upon re-installation. Parts and materials ordered.

Static and Dynamic Geophysical Logs were then conducted and a Downhole Video

New Items Installed

- (1) 3" Certa-lok Coupling with New O-Rings
- (2) Stainless Steel Crossover Subs
- 80' of 3/16 Stainless Steel Safety Cable with Clamps

Town of Belleair  
 Well Evaluations

Well 6  
 April 15, 2020

Time	Static Water Level
10:15	44.72
10:20	44.73
10:25	44.73

Pump Test Data at 140 gpm

Time	Dyn. Water Level
10:30	48.43
10:32	48.45
10:34	48.45
10:40	48.45
10:45	48.45
10:50	48.45
10:55	48.46
11:00	48.45
11:05	48.46
11:10	48.45
11:15	48.45

Recovery

Pump Off	Water Level
2 minutes	42.20
3 minutes	43.50
5 minutes	44.70
6 minutes	44.75
7 minutes	44.68
10 minutes	44.66

The old pump had to be cut out, and a replacement Pump, Motor, and Certa-lok drop pipe, was ordered.

Static and Dynamic Geophysical Logs were then conducted and a Downhole Video

Town of Belleair  
Well Evaluations

Well 6  
April 28,2020

The Wellhead was modified with a 4" coupling and the new Pump installed.

New Pump Details

Franklin 7.5 Hp 3ph 460v Motor  
Grundfos 230S75-3-BB Pump End  
(2) Stainless Steel Crossover Subs  
70'(2x20' & 1x10') of 4" Certa-lok Drop Pipe with an extra Coupling  
70' of Pump Wire with one Splice Kit  
85' of 3/16 Stainless Steel Safety Cable with Clamps

DRAFT

Town of Belleair  
 Well Evaluations

Well 7

May 12, 2020 and May 13, 2020

Time	Static Water Level
8:47	47.14
9:10	46.99

Pump Test Data at 100 gpm

Time	Dyn. Water Level
9:15	51.60
9:17	51.61
9:19	51.61
9:23	51.66
9:26	51.66
9:31	51.61
9:39	51.61
9:48	51.61
10:05	51.63
10:15	51.61

Recovery

Pump Off	Water Level
5 minutes	44.14
10 minutes	42.64
14 minutes	46.98
15 minutes	46.94
16 minutes	46.93
18 minutes	46.92

The old pump was removed, and a replacement Pump. Motor, and Certa-lok drop pipe, was ordered.

Static and Dynamic Geophysical Logs were then conducted and a Downhole Video

## Town of Belleair

Well Evaluations

### Well 7

May 12, 2020 and May 13, 2020

The Wellhead was modified with a 4" coupling and the new Pump installed.

#### New Pump Details

Franklin 7.5 Hp 3ph 460v Motor

Grundfos 230S75-3-BB Pump End

(2) Stainless Steel Crossover Subs

70' (2x20' & 1x10') of 4" Certa-lok Drop Pipe with an extra Coupling

70' of Pump Wire with one Splice Kit

80' of 3/16 Stainless Steel Safety Cable with Clamps

DRAFT

Town of Belleair  
Well Evaluations

Well 9  
May 5, 2020

Time	Static Water Level
13:42	46.37

Pump Test Data at 100 gpm

Time	Dyn. Water Level
13:45	46.59
13:47	46.48
13:49	46.40
13:52	46.40
14:00	46.40
14:10	46.40
14:20	46.40
14:30	46.40
14:45	46.40

Recovery

Pump Off	Water Level
4 minutes	46:20
6 minutes	46:35
8 minutes	46:33
11 minutes	46:35

Pulled pump 7.5 Hp set on 3" Certa-lok to 80 feet.

Static and Dynamic Geophysical Logs were then conducted and a Downhole Video

Pump re-installed with no changes.

Town of Belleair  
 Well Evaluations

Well 10  
 June 9, 2020

Time	Static Water Level
9:00	26.41

Pump Test Data at 125 gpm

Time	Dyn. Water Level
9:14	30.76
9:16	30.75
9:17	30.74
9:18	30.72
9:19	30.71
9:20	30.71
9:30	30.70
9:40	30.70
9:50	30.65
10:05	30.61
10:15	30.61

Recovery

Pump Off	Water Level
10:16	26.85
10:17	27.04
10:18	26.87
10:19	26.71
10:20	26.71
10:22	26.51
10:24	26.51
10:26	26.49
10:27	26.43
10:31	26.41
10:33	26.41

The old pump was removed for logging.



Town of Belleair  
Well Evaluations

Well 10  
June 10,2020

Static and Dynamic Geophysical Logs were then conducted and a Downhole Video

The Wellhead was modified with a 4" coupling and the new Pump. Motor, and Certa-lok drop pipe, was installed.

New Pump Details

Franklin 7.5 Hp 3ph 460v Motor  
Grundfos 230S75-3-BB Pump End  
(2) Stainless Steel Crossover Subs  
70'(2x20' & 1x10') of 4" Certa-lok Drop Pipe with an extra Coupling  
80' of Pump Wire with one Splice Kit  
85' of 3/16 Stainless Steel Safety Cable with Clamps

DRAFT

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**APPENDIX B**  
**Geophysical Logs**



# GAMMA RAY (API)-CALIPER

## WELL 2

COMPANY	: APPLIED DRILLING ENG.	OTHER SERVICES:
WELL	: WELL 2	8044
FIELD	:	8711
COUNTY	: PINELLAS	VIDEO
STATE	: FLORIDA	

LOCATION	: BELLAIRE
SECTION	:
TOWNSHIP	:
RANGE	:
API NO.	:
UNIQUE WELL ID.	: NONE

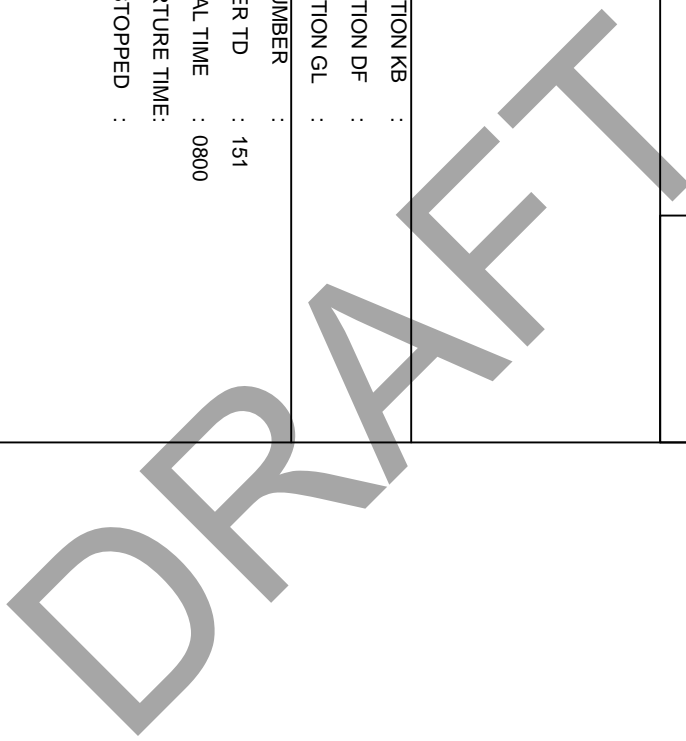
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LOG MEASURED FROM:	PAD	ELEVATION DF	:
DRL MEASURED FROM:	NA	ELEVATION GL	:

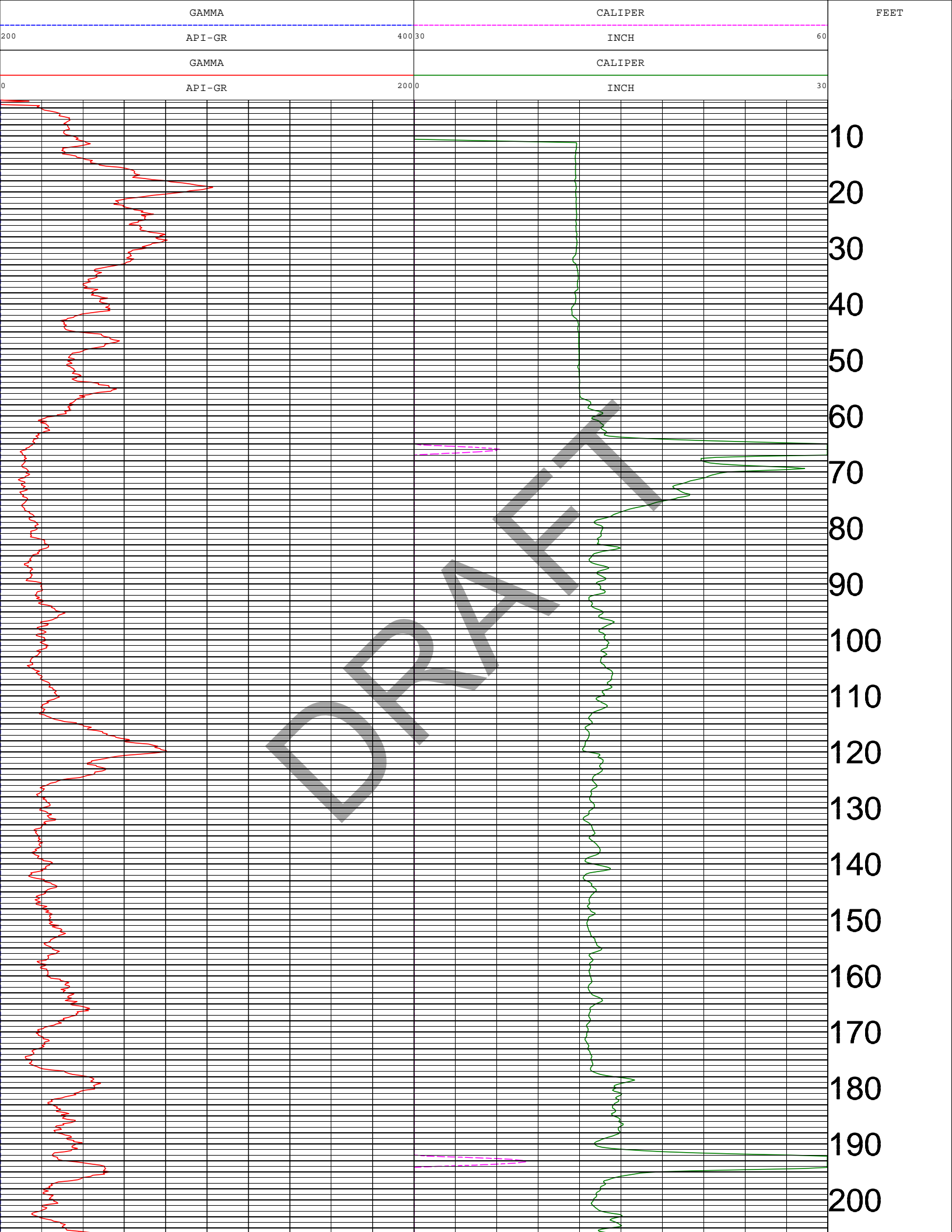
DATE	: 03/25/20	RIG NUMBER	:
DEPTH DRILLER	: NA	LOGGER TD	: 151
BIT SIZE	: 6	ARRIVAL TIME	: 0800
LOG TOP	: 3.60	DEPARTURE TIME:	
LOG BOTTOM	: 282.20	CIRC STOPPED	:

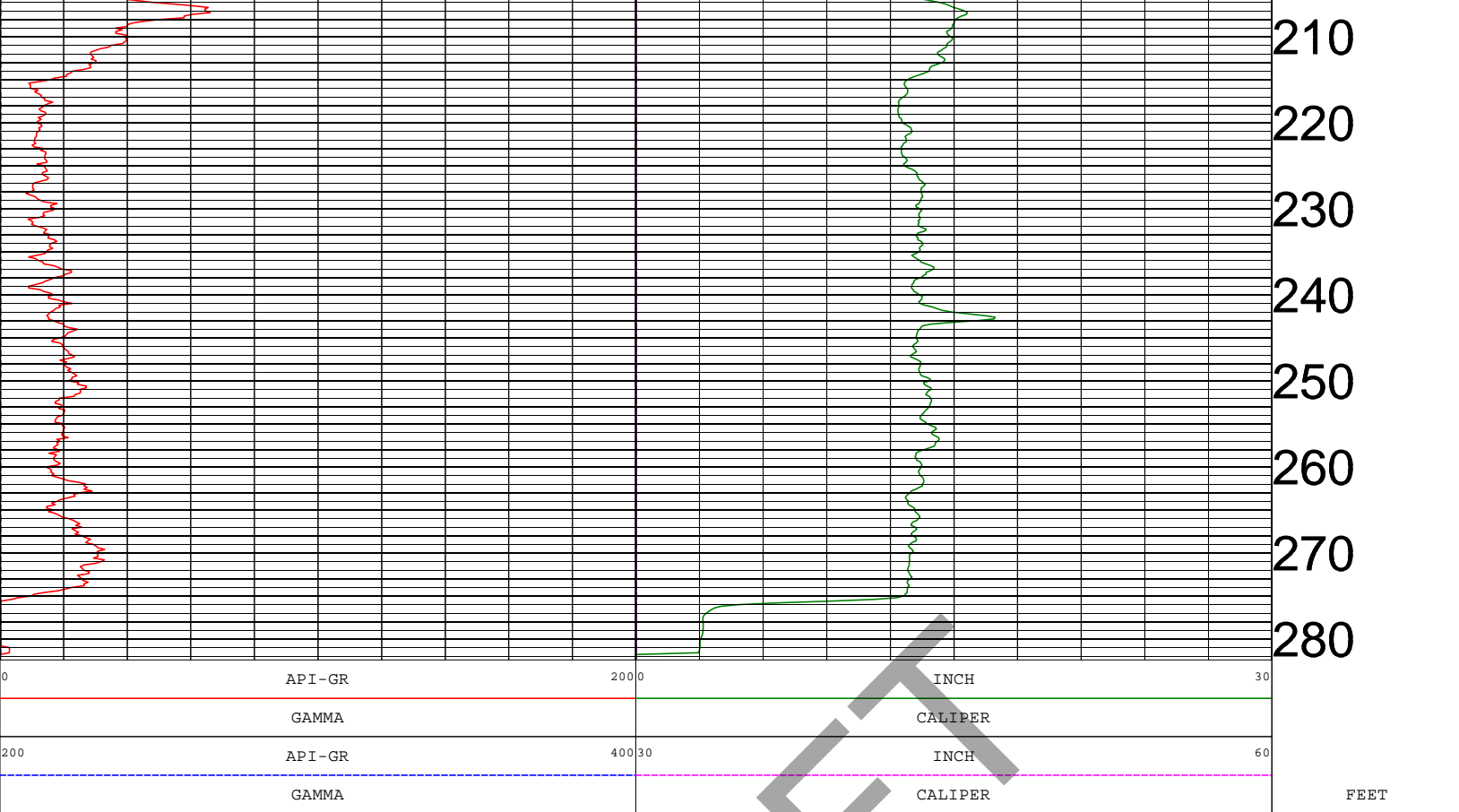
CASING OD	: 12
CASING BOTTOM	: XX
CASING TYPE	: STEEL
BOREHOLE FLUID	: FOR
RM TEMPERATURE	:
MUD RES	:
MUD WEIGHT	:
WITNESSED BY	: MAC DADDY
RECORDED BY	: AFB

REMARKS 1	:
REMARKS 2	:

ALL SERVICES PROVIDED SUBJECT TO STANDARD TERMS AND CONDITIONS





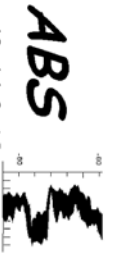


DRAFT

TOOL CALIBRATION WELL 2 03/25/20 09:00  
TOOL 9074A1 TM VERSION 2004  
SERIAL NUMBER 3158

	DATE	TIME	SENSOR	STANDARD	RESPONSE
1	Jul16,19	11:01:03	GAMMA	1.000 [API-GR ]	0.000 [CPS]
	Jul16,19	11:01:03	GAMMA	340.000 [API-GR ]	365.000 [CPS]
2	Jul16,19	11:04:12	CALIPER	4.000 [INCH ]	69017.000 [CPS]
	Jul16,19	11:04:12	CALIPER	6.000 [INCH ]	85360.000 [CPS]
3	Oct03,19	08:06:13	CALIPERL	8.000 [INCH ]	82709.000 [CPS]
	Oct03,19	08:06:13	CALIPERL	12.000 [INCH ]	107387.000 [CPS]
4	Jul16,19	11:00:52	CALIPERX	Default [CPS]	Default [CPS]
	Jul16,19	11:00:52	CALIPERX	Default [CPS]	Default [CPS]

DRAFT



# COMBINATION LOG

## STATIC WATER QUALITY

### WELL 2

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 2  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY ---  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 2  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : ---  
 API NO. :  
 UNIQ ID : NONE

SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 TOWNSHIP :  
 RANGE :

DISPLAY7\_JL56

PERMANENT DATUM  
 DRL MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

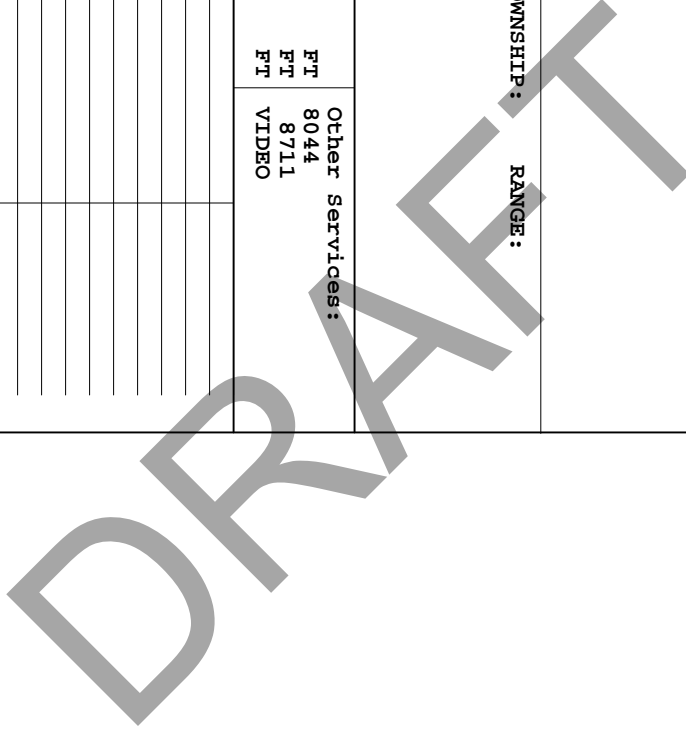
MSL  
 NA  
 NA  
 FT

Elevations:  
 KB  
 DF  
 GL

Other Services:  
 FT 8044  
 FT 8711  
 FT VIDEO

DATE	TIME	DEPTH DRILLER	DEPTH LOGGER	FIRST READING	LAST READING	BIT SIZE	CASING -- DRILLER	CASING -- LOGGER	CASING O.D.	CASING TYPE	FLUID TYPE	FLUID DENSITY	FLUID VISCOSITY	FLUID PH	MUD SOURCE	RM @ MEAS TEMP	RMF @ MEAS TEMP	RMC @ MEAS TEMP	CIRC STOPPED	RIG NUMBER
		NA	151	FT	FT	6	XX	XX	12	STEEL	FOR		LB/GAL			@ F	@ F	@ F		

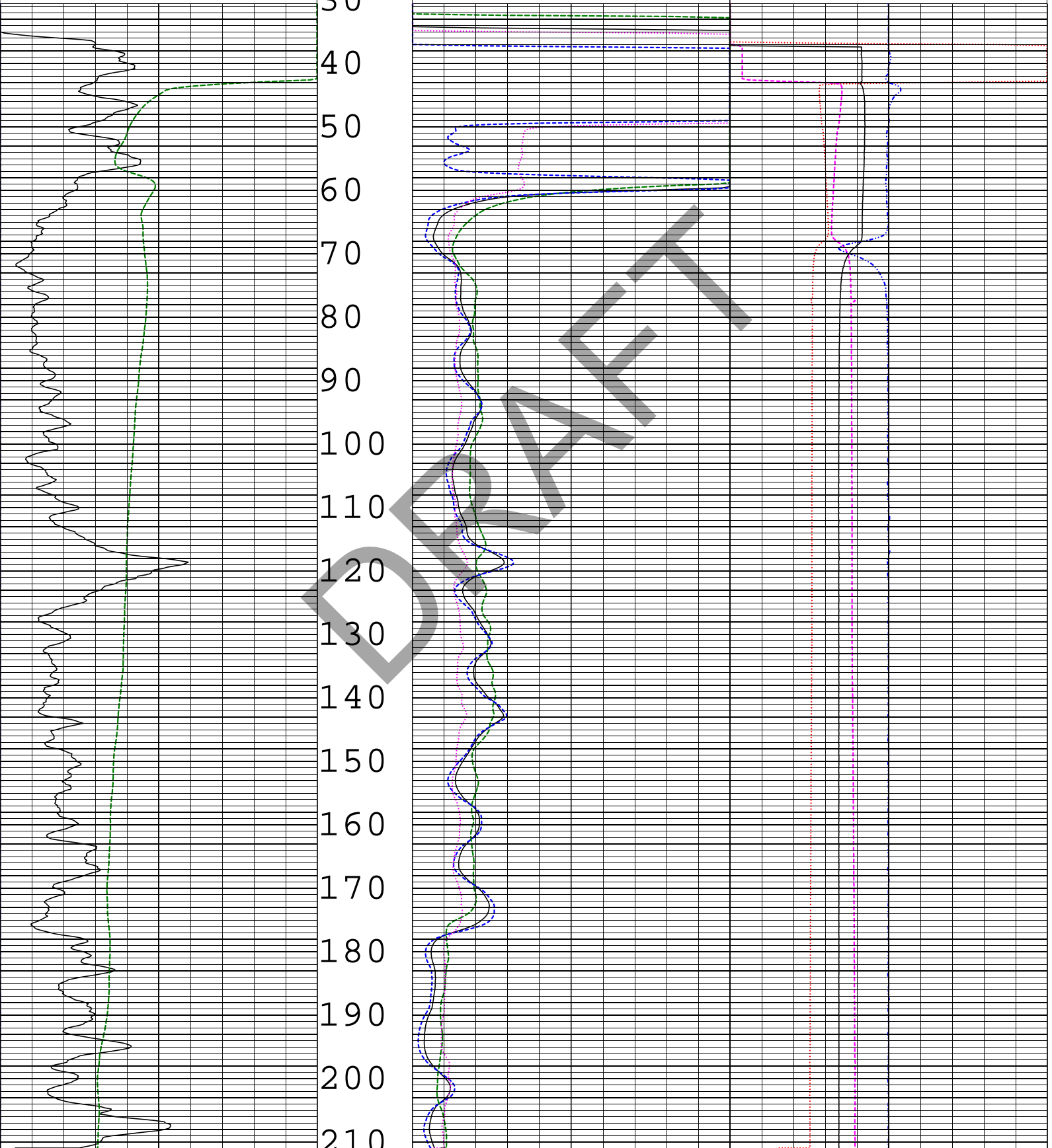
RECORDED BY AFB  
 WITNESSED BY MAC DADDY  
 REMARKS 1  
 REMARKS 2  
 REMARKS 3



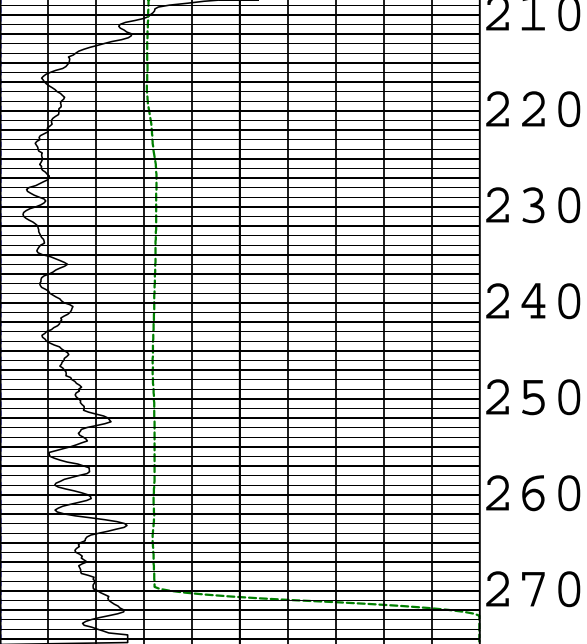
FEET

SP		
-500	MV	0
GAMMA		
100	API-GR	400
GAMMA		
0	API-GR	100

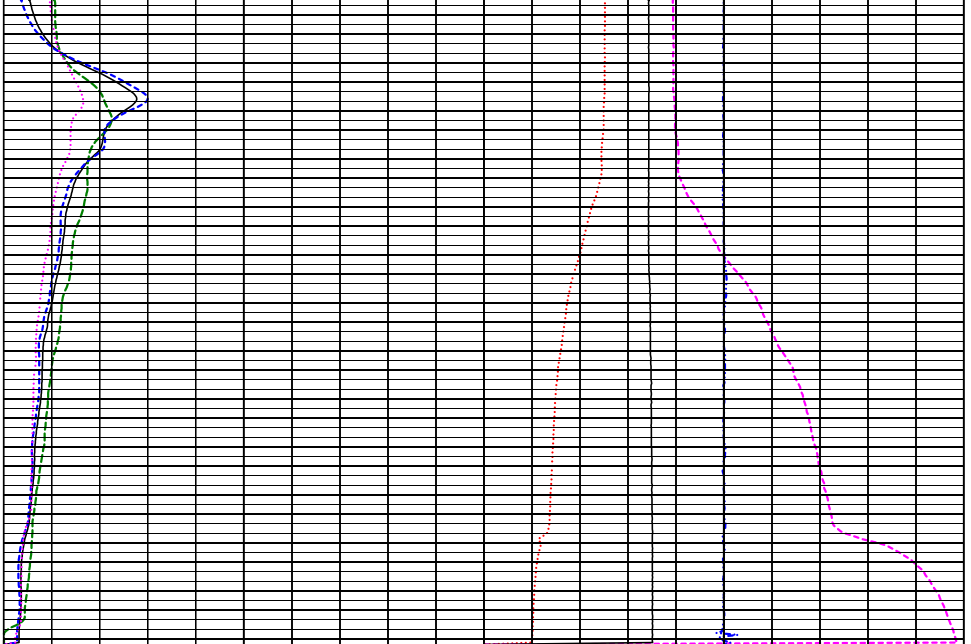
RES			SP_COND		
0	OHM	500	0	US/CM	2000
LATERAL			RES (FL)		
0	OHM-M	500	0	OHM-M	50
RES (64N)			DEL_TEMP		
0	OHM-M	500	-0.25	DEG-F	0.25
RES (16N)			TEMP		
0	OHM-M	500	70	DEG-F	90







0	API-GR	100
	GAMMA	
100	API-GR	400
	GAMMA	
-500	MV	0
	SP	



0	OHM-M	500	70	DEG-F	90
	RES (16N)			TEMP	
0	OHM-M	500	-0.25	DEG-F	0.25
	RES (64N)			DEL_TEMP	
0	OHM-M	500	0	OHM-M	50
	LATERAL			RES (FL)	
0	OHM	500	0	US/CM	2000
	RES			SP_COND	

FEET

DRAFT

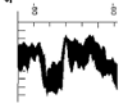
TOOL CALIBRATION WELL 2 03/25/20 09:26

TOOL 8044A TM VERSION 5500

SERIAL 938

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jan03,03	02:49:05	GAMMA [API-GR]	0.001	180.000	0.000	169
2	Sep29,19	19:01:50	RES(FL) [OHM-M]	30.300	7.290	34341	16551
3	Aug17,14	12:00:23	SP [MV]	0.000	395.000	59670	23612
4	Feb02,20	14:59:18	RES(16N) [OHM-M]	0.000	1996.000	4010	103211
5	Feb02,20	15:00:15	RES(64N) [OHM-M]	0.000	1990.000	4089	103487
6	Sep29,19	18:57:40	TEMP [DEG-F]	71.700	86.100	63355	57070
7	Aug17,14	10:39:11	RES [OHM]	0.000	988.000	9855	58788

DRAFT



# PRODUCTION-STATIC-PUMPING WELL 2

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 2  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY ---  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 2  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : ---  
 API NO. :  
 UNIQ ID : NONE

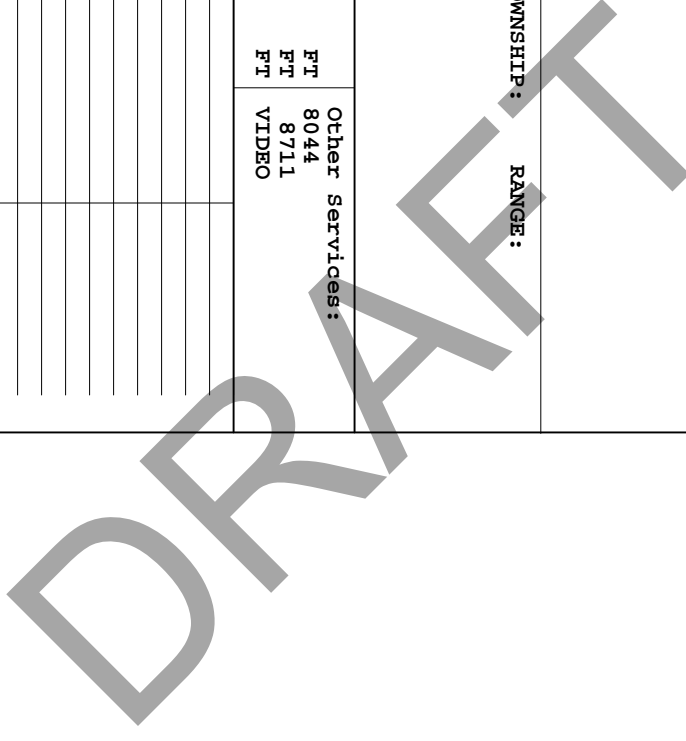
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 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY\_ J156

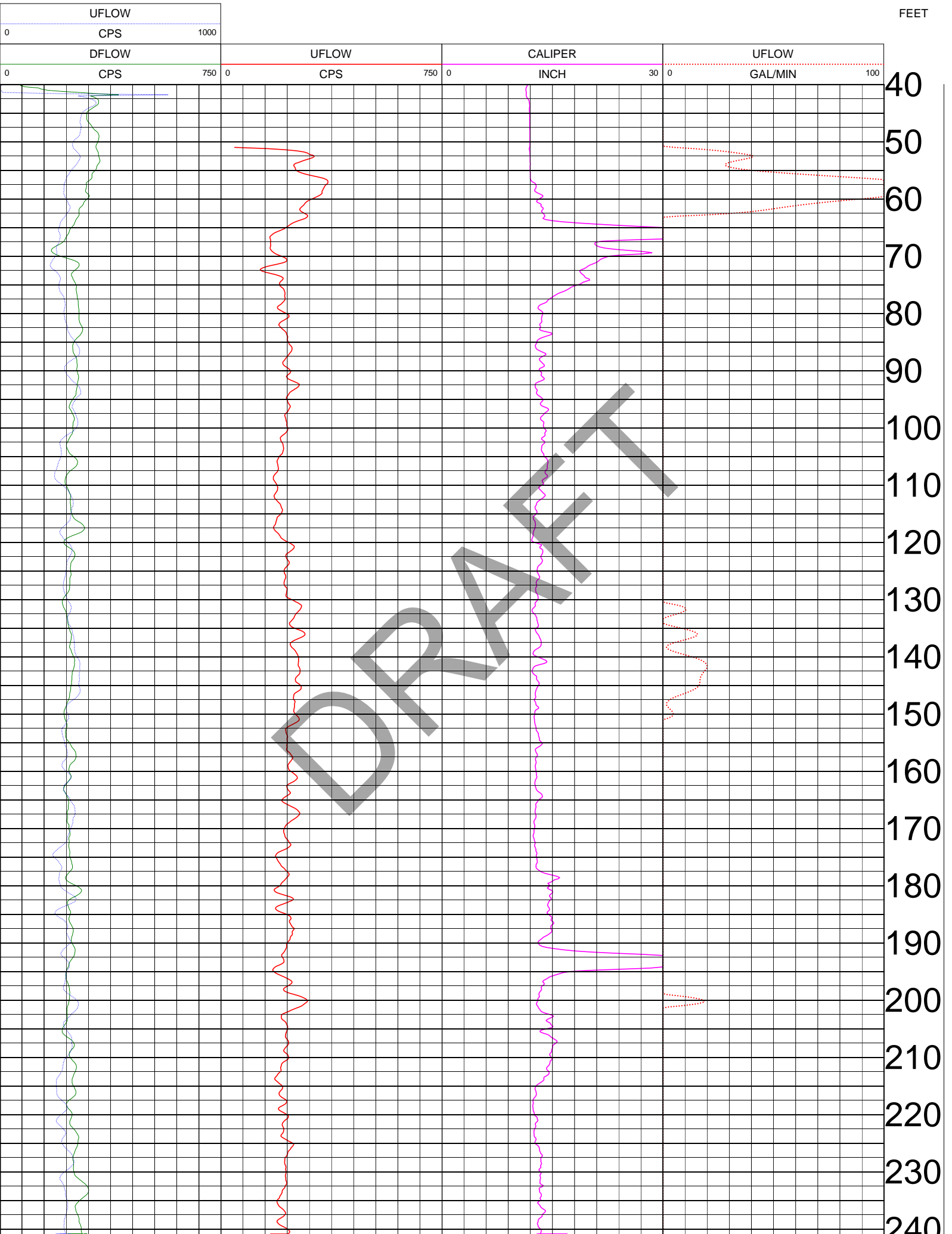
TOWNSHIP :  
 RANGE :

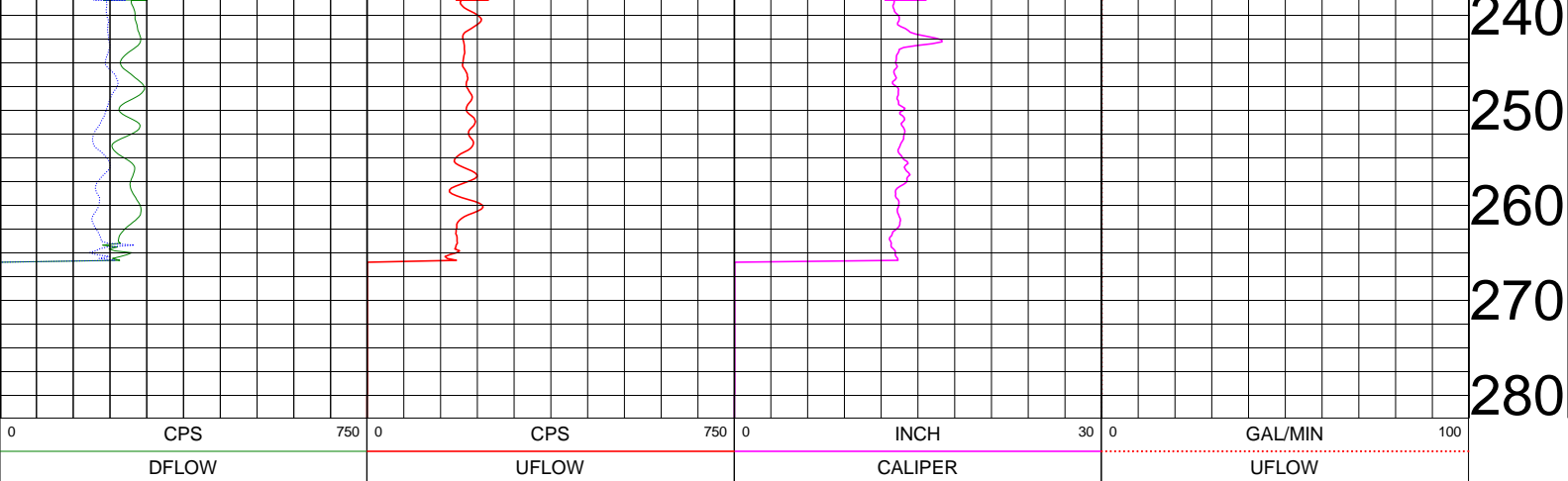
PERMANENT DATUM	MSL	Elevations:	Other Services:
DRL MEASURED FROM	NA	KB	FT 8044
LOG MEASURED FROM	PAD	DF	FT 8711
ELEV. PERM. DATUM	FT	GL	FT VIDEO

DATE	TIME	DEPTH DRILLER	DEPTH LOGGER	FIRST READING	LAST READING	BIT SIZE	CASING -- DRILLER	CASING -- LOGGER	CASING O.D.	CASING TYPE	FLUID TYPE	FLUID DENSITY	FLUID VISCOSITY	FLUID PH	MUD SOURCE	RM @ MEAS TEMP	RMF @ MEAS TEMP	RMC @ MEAS TEMP	CIRC STOPPED	RIG NUMBER
		NA	151	FT	FT	6	52	FT	12	STEEL	FOR		LB/GAL			@ F	@ F	@ F		

RECORDED BY AFB  
 WITNESSED BY MAC DADDY  
 REMARKS 1  
 REMARKS 2  
 REMARKS 3







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FEET



# PUMPING WATER QUALITY

## WELL 2

COMPANY : APPLIED DRILLING ENG.

WELL : WELL 2

FIELD :

COUNTY : PINELLAS

STATE : FLORIDA

OTHER SERVICES:

8044

8711

VIDEO

LOCATION : BELLEAIRE

SECTION :

TOWNSHIP :

RANGE :

API NO. :

UNIQUE WELL ID. : NONE

PERMANENT DATUM : MSL

LOG MEASURED FROM: PAD

DRL MEASURED FROM: NA

ELEVATION KB :

ELEVATION DF :

ELEVATION GL :

DATE : 03/25/20

DEPTH DRILLER : NA

BIT SIZE : 6

LOG TOP : 45.60

LOG BOTTOM : 276.60

CASING OD : 12

CASING BOTTOM : 52

CASING TYPE : STEEL

BOREHOLE FLUID : FOR

RM TEMPERATURE :

MUD RES :

MUD WEIGHT :

WITNESSED BY : MAC DADDY

RECORDED BY : AFB

REMARKS 1 :

REMARKS 2 :

RIG NUMBER :

LOGGER TD : 151

ARRIVAL TIME : 0800

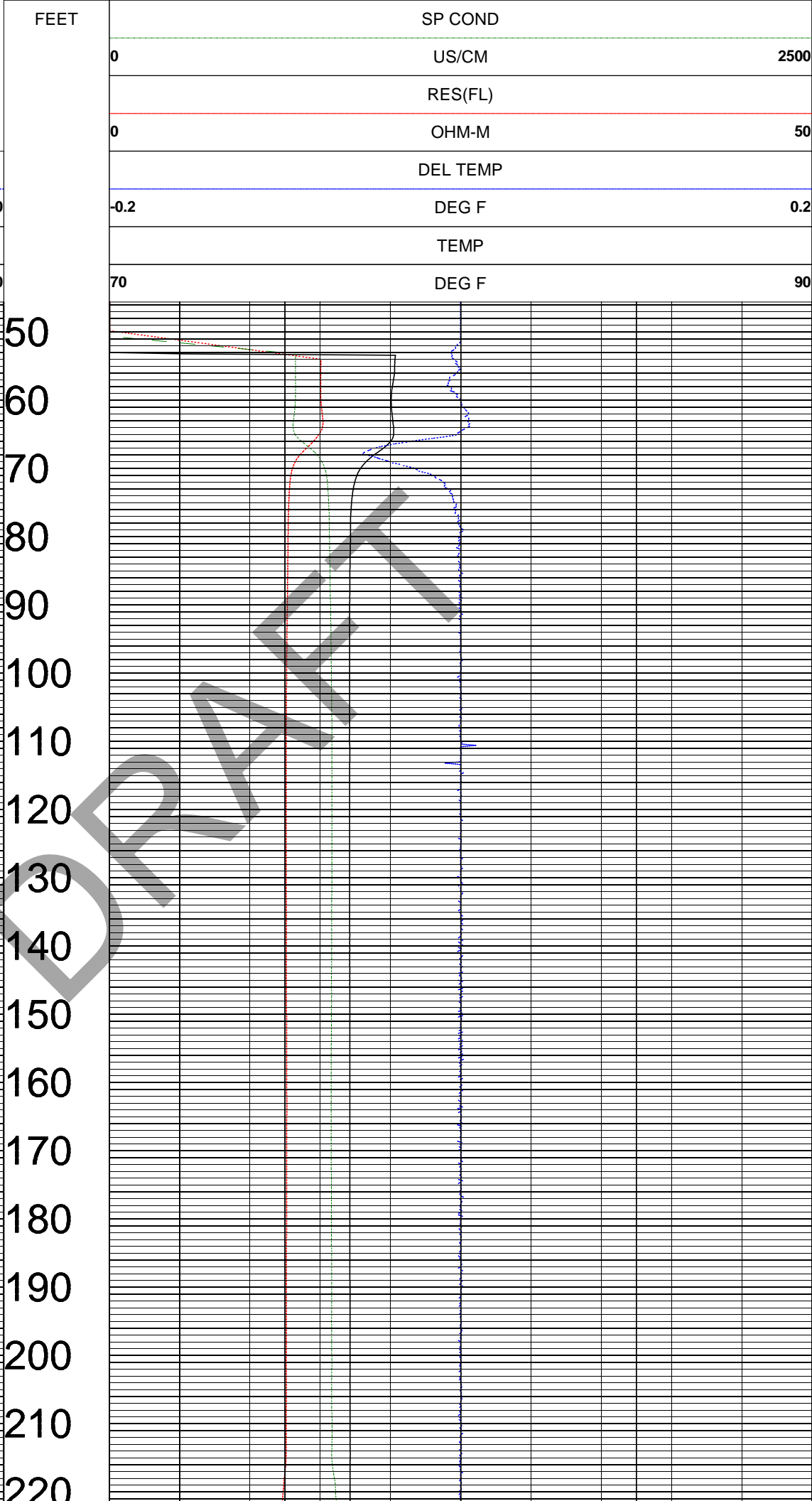
DEPARTURE TIME:

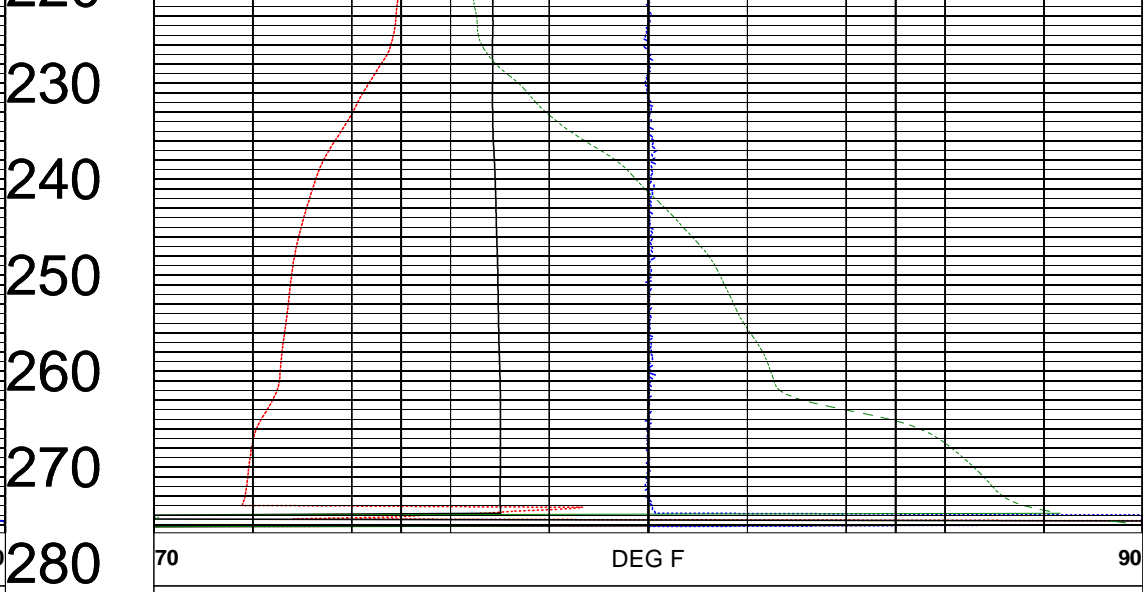
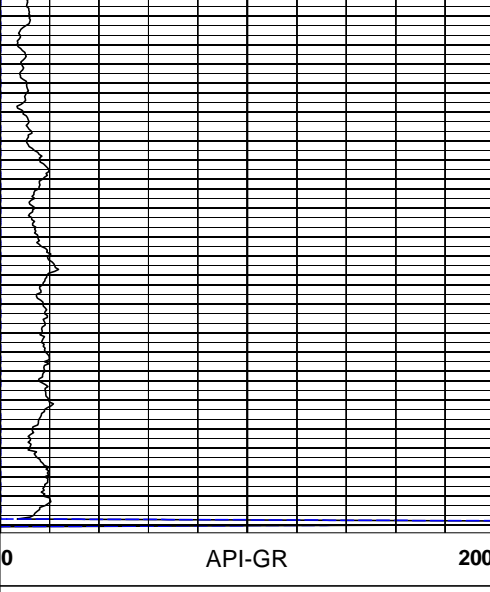
CIRC STOPPED :

ALL SERVICES PROVIDED SUBJECT TO STANDARD TERMS AND CONDITIONS

DRAFT

			FEET
	GAMMA		
200	API-GR	400	
	GAMMA		
0	API-GR	200	





0	API-GR	200
	GAMMA	
200	API-GR	400
	GAMMA	

70	DEG F	90
	TEMP	
-0.2	DEG F	0.2
	DEL TEMP	
0	OHM-M	50
	RES(FL)	
0	US/CM	2500
	SP COND	

FEET

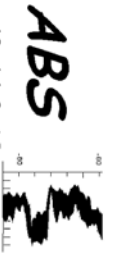
DRAFT



TOOL CALIBRATION WELL 2 03/25/20 10:46  
TOOL 8044A TM VERSION 5500  
SERIAL NUMBER 938

	DATE	TIME	SENSOR		STANDARD		RESPONSE
1	Jan03,03	02:49:05	GAMMA	0.001	[API-GR ]	0.000	[CPS]
	Jan03,03	02:49:05	GAMMA	180.000	[API-GR ]	169.000	[CPS]
2	Sep29,19	19:01:50	RES(FL)	30.300	[OHM-M ]	34341.000	[CPS]
	Sep29,19	19:01:50	RES(FL)	7.290	[OHM-M ]	16551.000	[CPS]
3	Aug17,14	12:00:23	SP	0.000	[MV ]	59670.000	[CPS]
	Aug17,14	12:00:23	SP	395.000	[MV ]	23612.000	[CPS]
4	Feb02,20	14:59:18	RES(16N)	0.000	[OHM-M ]	4010.000	[CPS]
	Feb02,20	14:59:18	RES(16N)	1996.000	[OHM-M ]	103211.000	[CPS]
5	Feb02,20	15:00:15	RES(64N)	0.000	[OHM-M ]	4089.000	[CPS]
	Feb02,20	15:00:15	RES(64N)	1990.000	[OHM-M ]	103487.000	[CPS]
6	Sep29,19	18:57:40	TEMP	71.700	[DEG F ]	63355.000	[CPS]
	Sep29,19	18:57:40	TEMP	86.100	[DEG F ]	57070.000	[CPS]
7	Aug17,14	10:39:11	RES	0.000	[OHM ]	9855.000	[CPS]
	Aug17,14	10:39:11	RES	988.000	[OHM ]	58788.000	[CPS]

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# GAMMA RAY (API) - CALIPER WELL 5

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 5  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 5  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE

SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY\_ J156

PERMANENT DATUM  
 DRL MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

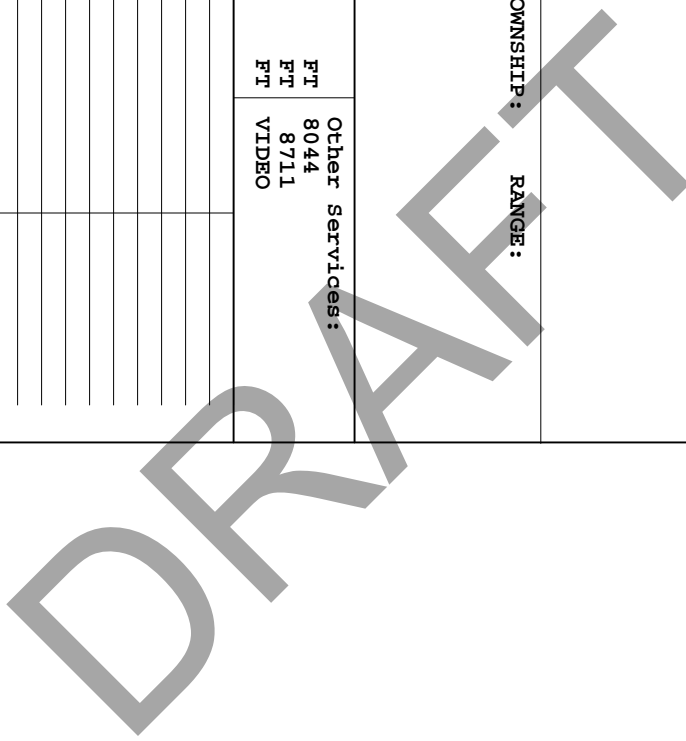
MSL  
 NA  
 PAD  
 FT

Elevations:  
 KB  
 DF  
 GL

Other Services:  
 FT 8044  
 DF 8711  
 FT VIDEO

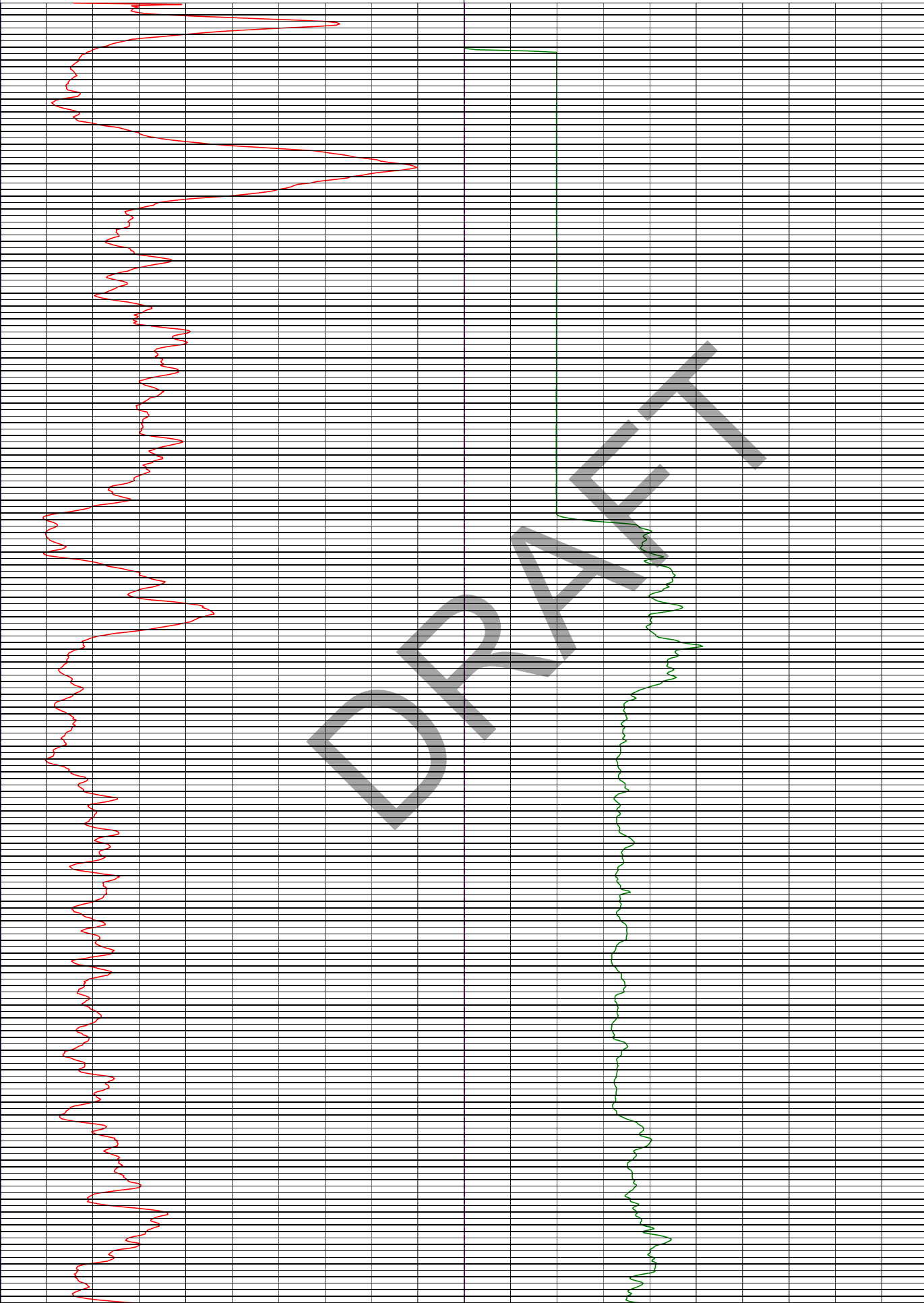
DATE	TIME	DEPTH DRILLER	DEPTH LOGGER	FIRST READING	LAST READING	BIT SIZE	CASING -- DRILLER	CASING -- LOGGER	CASING O.D.	CASING TYPE	FLUID TYPE	FLUID DENSITY	FLUID VISCOSITY	FLUID PH	MUD SOURCE	RM @ MEAS TEMP	RMF @ MEAS TEMP	RMC @ MEAS TEMP	CIRC STOPPED	RIG NUMBER
		NA	256	FT	FT	6	NA	80	6	PVC	FOR		LB/GAL			@ F	@ F	@ F		

RECORDED BY AFB  
 WITNESSED BY MAC DADDY  
 REMARKS 1  
 REMARKS 2  
 REMARKS 3

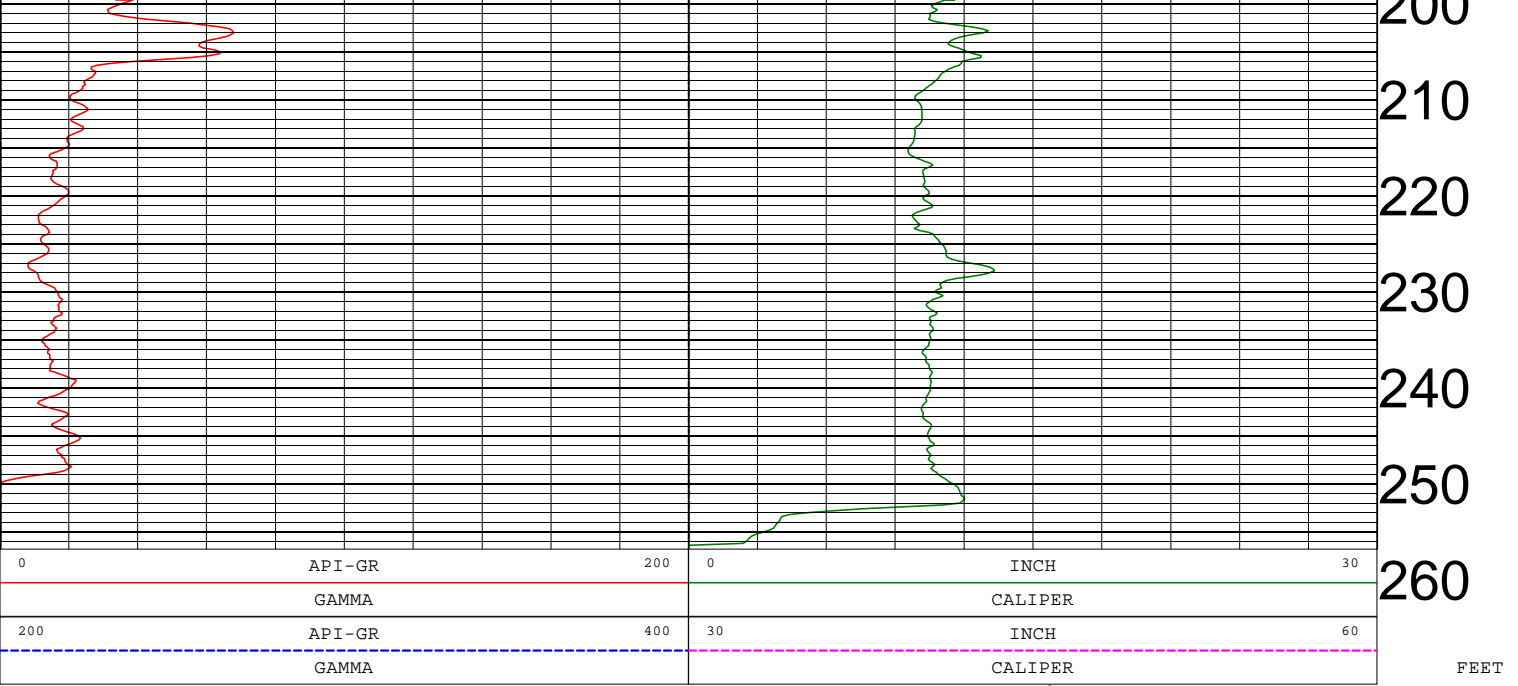


GAMMA		CALIPER	
200	API - GR	30	INCH
400		60	
GAMMA		CALIPER	
0	API - GR	0	INCH
200		30	

FEET



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TOOL CALIBRATION WELL 5 04/29/20 08:44

TOOL 9074A1 TM VERSION 2004

SERIAL 3158

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jul16,19	11:01:03	GAMMA [API-GR]	1.000	340.000	0.000	365
2	Jul16,19	11:04:12	CALIPER [INCH]	4.000	6.000	69017	85360
3	Oct03,19	08:06:13	CALIPERL [INCH]	8.000	12.000	82709	107387
4	Jul16,19	11:00:52	CALIPERX [CPS]	Default	Default	Default	Default

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# COMBINATION LOG

## STATIC WATER QUALITY

### WELL 5

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 5  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 5  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE

SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 TOWNSHIP :  
 RANGE :  
 DISPLAY7\_JL56

PERMANENT DATUM  
 DRL MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

MSL  
 NA  
 PAD  
 FT

Elevations:  
 KB  
 DF  
 GL

FT  
 FT  
 FT  
 Other Services:  
 8044  
 8711  
 VIDEO

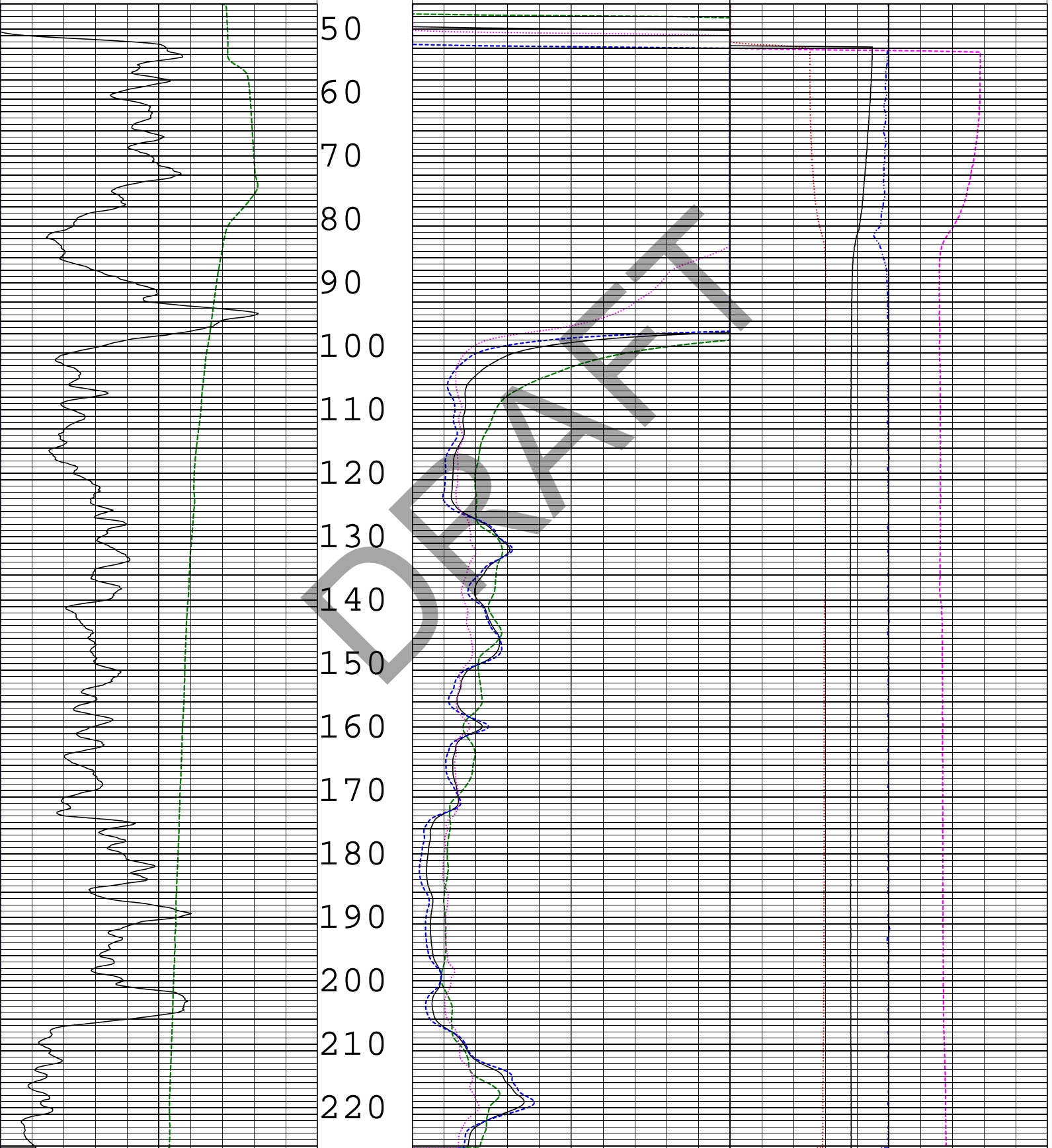
DATE	TIME	DEPTH DRILLER	DEPTH LOGGER	FIRST READING	LAST READING	BIT SIZE	CASING -- DRILLER	CASING -- LOGGER	CASING O.D.	CASING TYPE	FLUID TYPE	FLUID DENSITY	FLUID VISCOSITY	FLUID PH	MUD SOURCE	RM @ MEAS TEMP	RMF @ MEAS TEMP	RMC @ MEAS TEMP	CIRC STOPPED	RIG NUMBER	RECORDED BY	WITNESSED BY	REMARKS 1	REMARKS 2	REMARKS 3
		NA	256			6	NA	80	6	PVC	FOR					@ F	@ F	@ F			AFB	MAC DADDY			

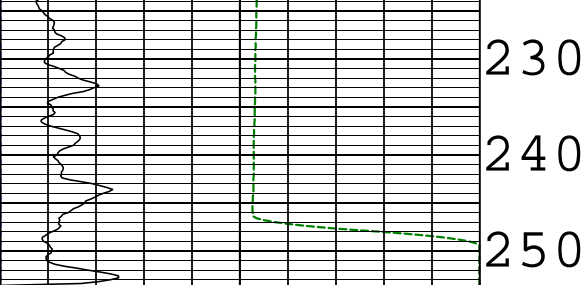
ALL SERVICES PROVIDED SUBJECT TO STANDARD TERMS AND CONDITIONS

FEET

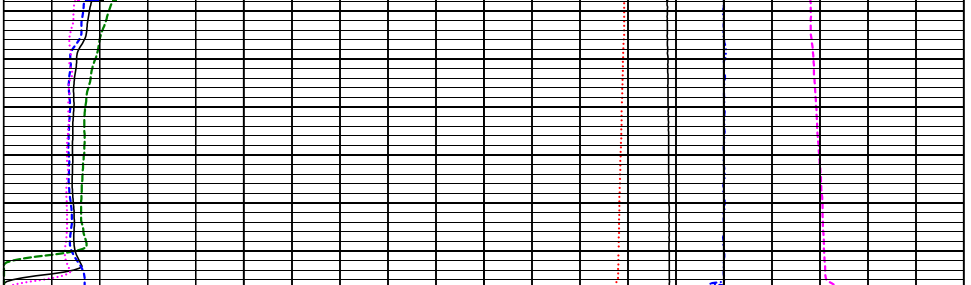
SP		
-500	MV	0
GAMMA		
100	API-GR	400
GAMMA		
0	API-GR	100

RES			SP_COND		
0	OHM	500	0	US/CM	1000
LATERAL			RES (FL)		
0	OHM-M	500	0	OHM-M	50
RES (64N)			DEL_TEMP		
0	OHM-M	500	-0.25	DEG-F	0.25
RES (16N)			TEMP		
0	OHM-M	500	70	DEG-F	90





0	API-GR	100
GAMMA		
100	API-GR	400
GAMMA		
-500	MV	0
SP		



0	OHM-M	500	70	DEG-F	90
RES ( 16N)			TEMP		
0	OHM-M	500	-0.25	DEG-F	0.25
RES ( 64N)			DEL_TEMP		
0	OHM-M	500	0	OHM-M	50
LATERAL			RES ( FL )		
0	OHM	500	0	US / CM	1000
RES			SP_COND		

FEET

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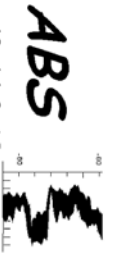
TOOL CALIBRATION WELL 5 04/29/20 09:19

TOOL 8044A TM VERSION 2002

SERIAL 938

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jan03,03	02:49:05	GAMMA [API-GR]	0.001	180.000	0.000	169
2	Apr09,20	11:12:41	RES(FL) [OHM-M]	3.060	28.160	13413	37152
3	Aug17,14	12:00:23	SP [MV]	0.000	395.000	59670	23612
4	Feb02,20	14:59:18	RES(16N) [OHM-M]	0.000	1996.000	4010	103211
5	Feb02,20	15:00:15	RES(64N) [OHM-M]	0.000	1990.000	4089	103487
6	Sep29,19	18:57:40	TEMP [DEG-F]	71.700	86.100	63355	57070
7	Aug17,14	10:39:11	RES [OHM]	0.000	988.000	9855	58788

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# PRODUCTION-STATIC-PUMPING WELL 5

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 5  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 5  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE

SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY\_ J156

PERMANENT DATUM  
 DRL MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

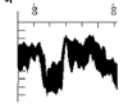
MSL  
 NA  
 PAD  
 FT

Elevations:  
 KB  
 DF  
 GL

Other Services:  
 FT 8044  
 DF 8711  
 FT VIDEO

DATE	TIME	DEPTH DRILLER	DEPTH LOGGER	FIRST READING	LAST READING	BIT SIZE	CASING -- DRILLER	CASING -- LOGGER	CASING O.D.	CASING TYPE	FLUID TYPE	FLUID DENSITY	FLUID VISCOSITY	FLUID PH	MUD SOURCE	RM @ MEAS TEMP	RMF @ MEAS TEMP	RMC @ MEAS TEMP	CIRC STOPPED	RIG NUMBER	RECORDED BY	WITNESSED BY	REMARKS 1	REMARKS 2	REMARKS 3
		NA	256			6	NA	80	6	PVC						@ F	@ F	@ F			AFB	MAC DADDY			





# PUMPED WATER QUALITY WELL 5

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 5  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 5  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE

SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY7\_JL56

TOWNSHIP :  
 RANGE :

PERMANENT DATUM  
 DRL MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

MSL  
 NA  
 PAD  
 FT

Elevations:  
 KB  
 DF  
 GL

FT  
 FT  
 FT

Other Services:  
 8044  
 8711  
 VIDEO

DATE	TIME	DEPTH DRILLER	DEPTH LOGGER	FIRST READING	LAST READING	BIT SIZE	CASING -- DRILLER	CASING -- LOGGER	CASING O.D.	CASING TYPE	FLUID TYPE	FLUID DENSITY	FLUID VISCOSITY	FLUID PH	MUD SOURCE	RM @ MEAS TEMP	RMF @ MEAS TEMP	RMG @ MEAS TEMP	CIRC STOPPED	RIG NUMBER	RECORDED BY	WITNESSED BY	REMARKS 1	REMARKS 2	REMARKS 3
		NA	256			6	NA	80	6	PVC						@ F	@ F	@ F			AFB	MAC DADDY			

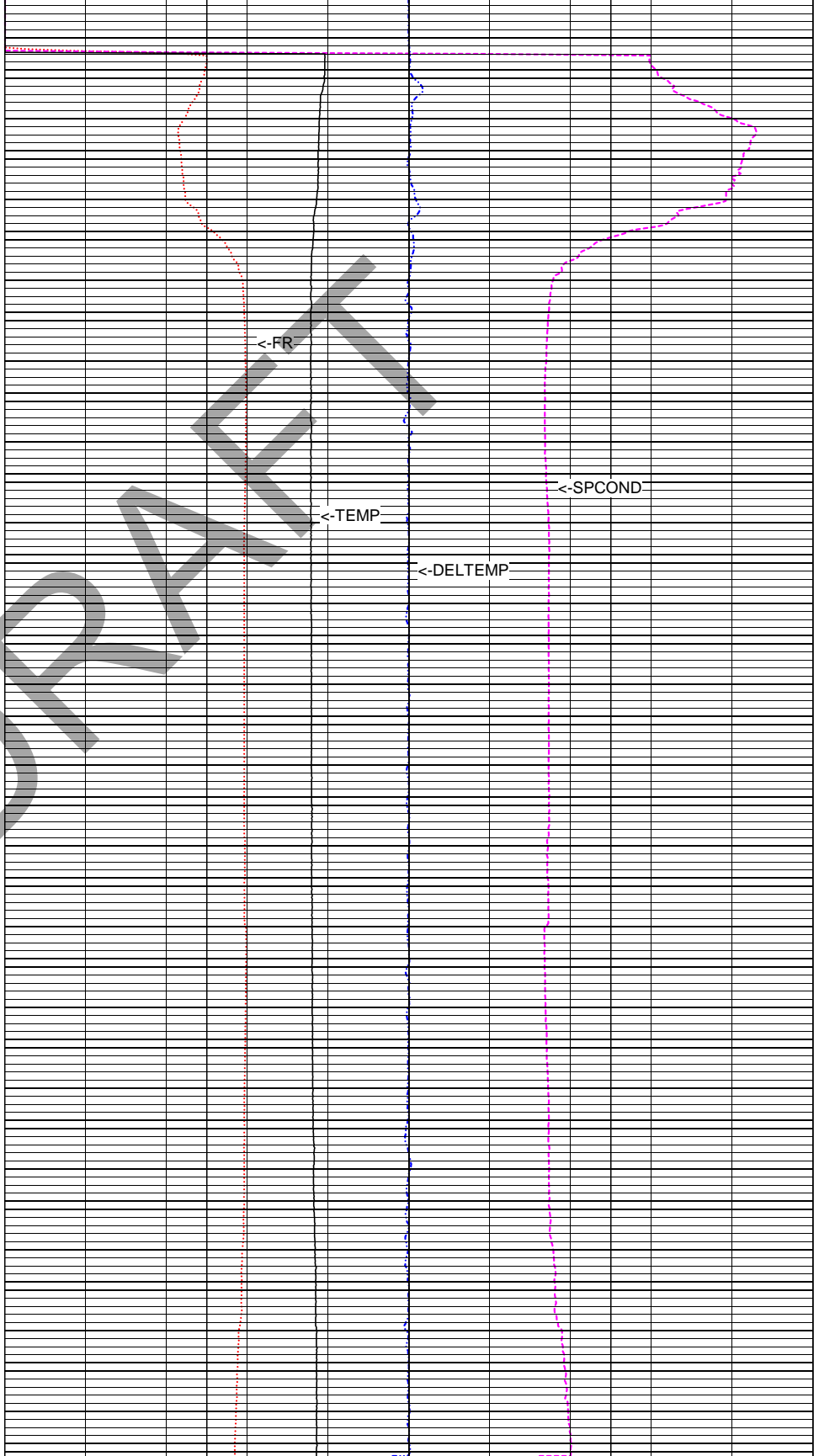
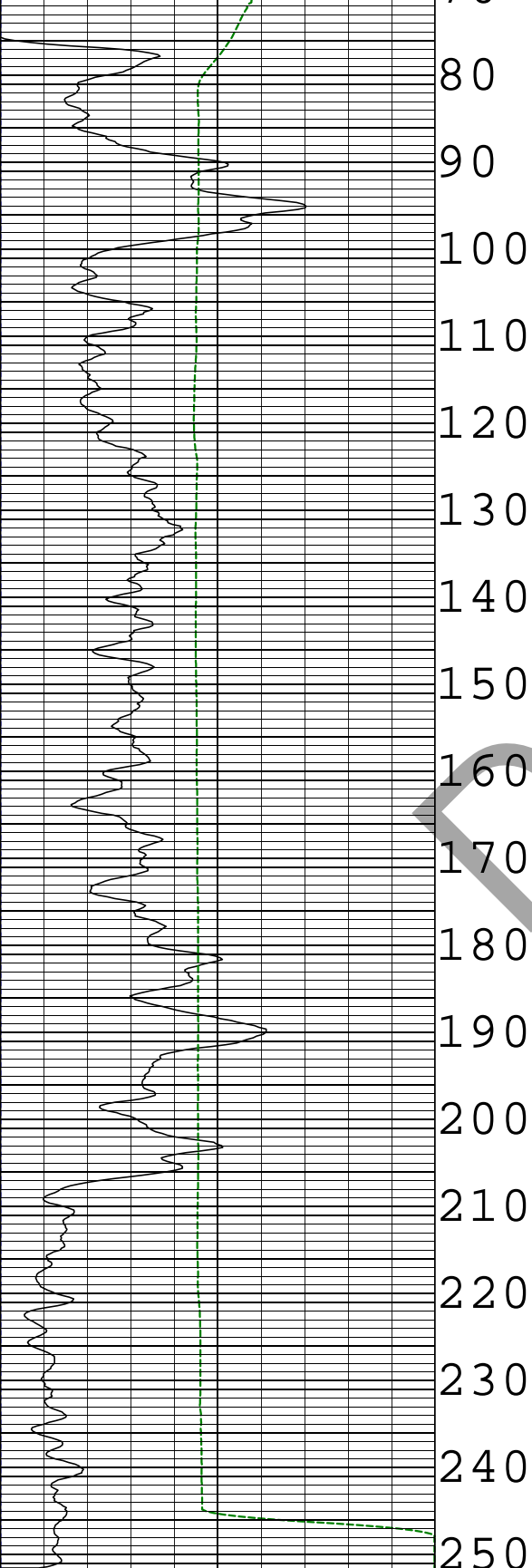
FEET

SP		
-500	MV	0

GAMMA		
100	API-GR	400

GAMMA		
0	API-GR	100

SP_COND		
0	US/CM	1000
RES (FL)		
0	OHM-M	50
DEL_TEMP		
-0.25	DEG-F	0.25
TEMP		
70	DEG-F	90



0	API-GR	100
GAMMA		
100	API-GR	400
GAMMA		
-500	MV	0
SP		

250

70	DEG-F	90
TEMP		
-0.25	DEG-F	0.25
DEL_TEMP		
0	OHM-M	50
RES (FL)		
0	US/CM	1000
SP_COND		

F E E T

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TOOL CALIBRATION WELL 5 04/29/20 09:56

TOOL 8044A TM VERSION 2002

SERIAL 938

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jan03,03	02:49:05	GAMMA [API-GR]	0.001	180.000	0.000	169
2	Apr09,20	11:12:41	RES(FL) [OHM-M]	3.060	28.160	13413	37152
3	Aug17,14	12:00:23	SP [MV]	0.000	395.000	59670	23612
4	Feb02,20	14:59:18	RES(16N) [OHM-M]	0.000	1996.000	4010	103211
5	Feb02,20	15:00:15	RES(64N) [OHM-M]	0.000	1990.000	4089	103487
6	Sep29,19	18:57:40	TEMP [DEG-F]	71.700	86.100	63355	57070
7	Aug17,14	10:39:11	RES [OHM]	0.000	988.000	9855	58788

DRAFT



# GAMMA RAY (API) - CALIPER WELL 6

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 6  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY ---  
 API NO.

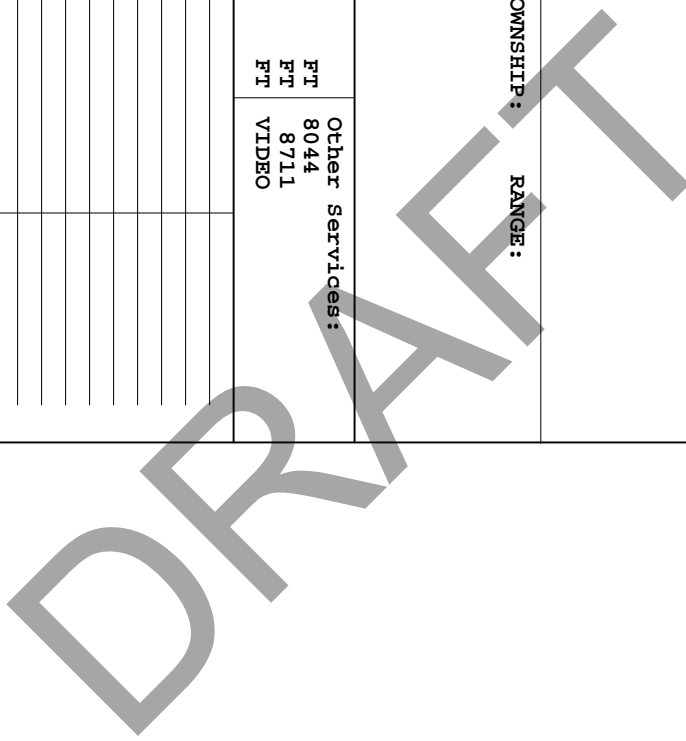
COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 6  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : ---  
 API NO. :  
 UNIQ ID : NONE

SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY\_ J156

PERMANENT DATUM	MSL	Elevations:	Other Services:
DRL MEASURED FROM	NA	KB	FT 8044
LOG MEASURED FROM	PAD	DF	FT 8711
ELEV. PERM. DATUM	FT	GL	FT VIDEO

DATE	TIME	DEPTH DRILLER	DEPTH LOGGER	FIRST READING	LAST READING	BIT SIZE	CASING -- DRILLER	CASING -- LOGGER	CASING O.D.	CASING TYPE	FLUID TYPE	FLUID DENSITY	FLUID VISCOSITY	FLUID PH	MUD SOURCE	RM @ MEAS TEMP	RMF @ MEAS TEMP	RMG @ MEAS TEMP	CIRC STOPPED	RIG NUMBER
		NA	299	FT	FT	6	XX	XX	12	STEEL	FOR		LB/GAL			@ F	@ F	@ F		

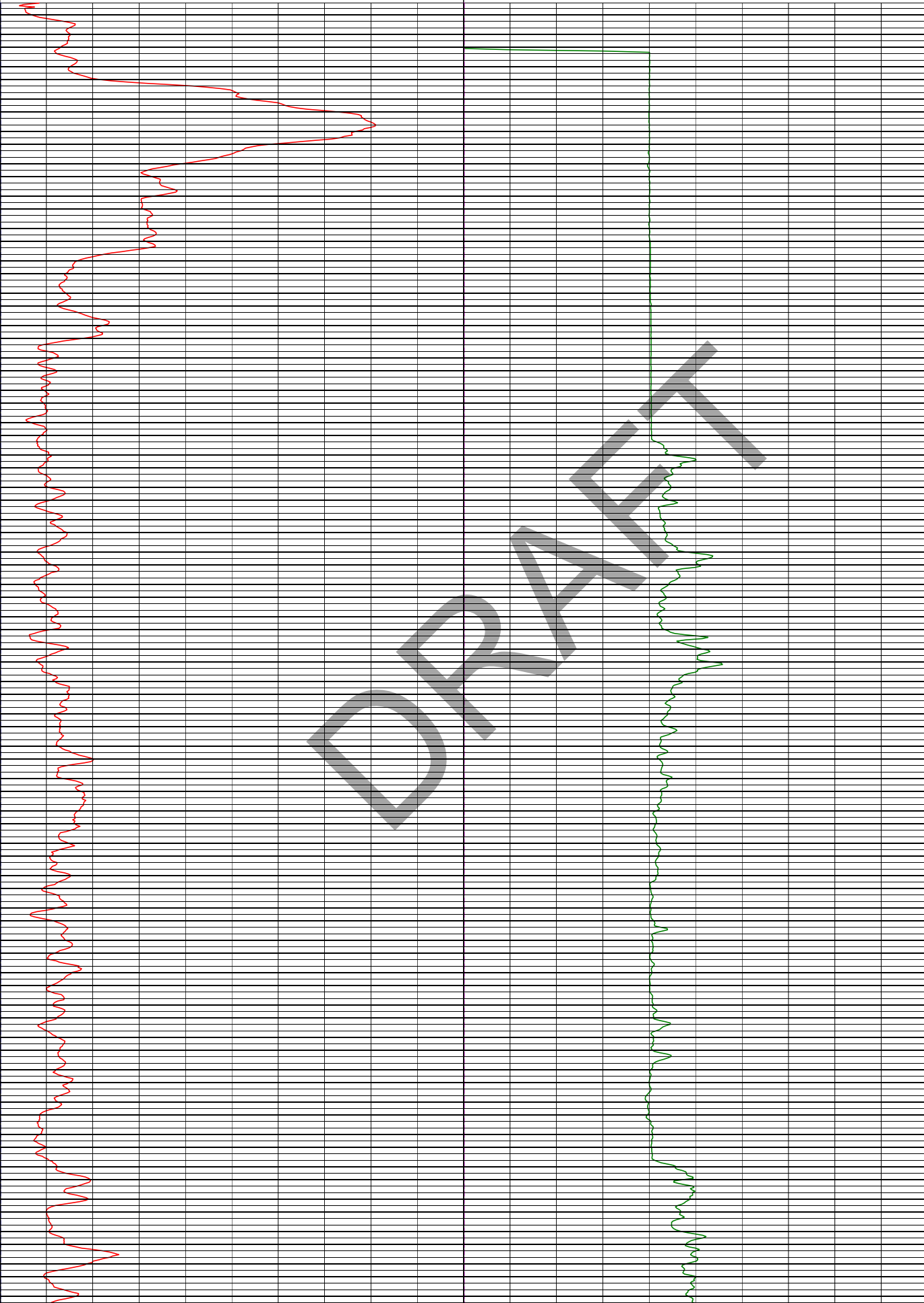
RECORDED BY AFB  
 WITNESSED BY MAC DADDY  
 REMARKS 1  
 REMARKS 2  
 REMARKS 3





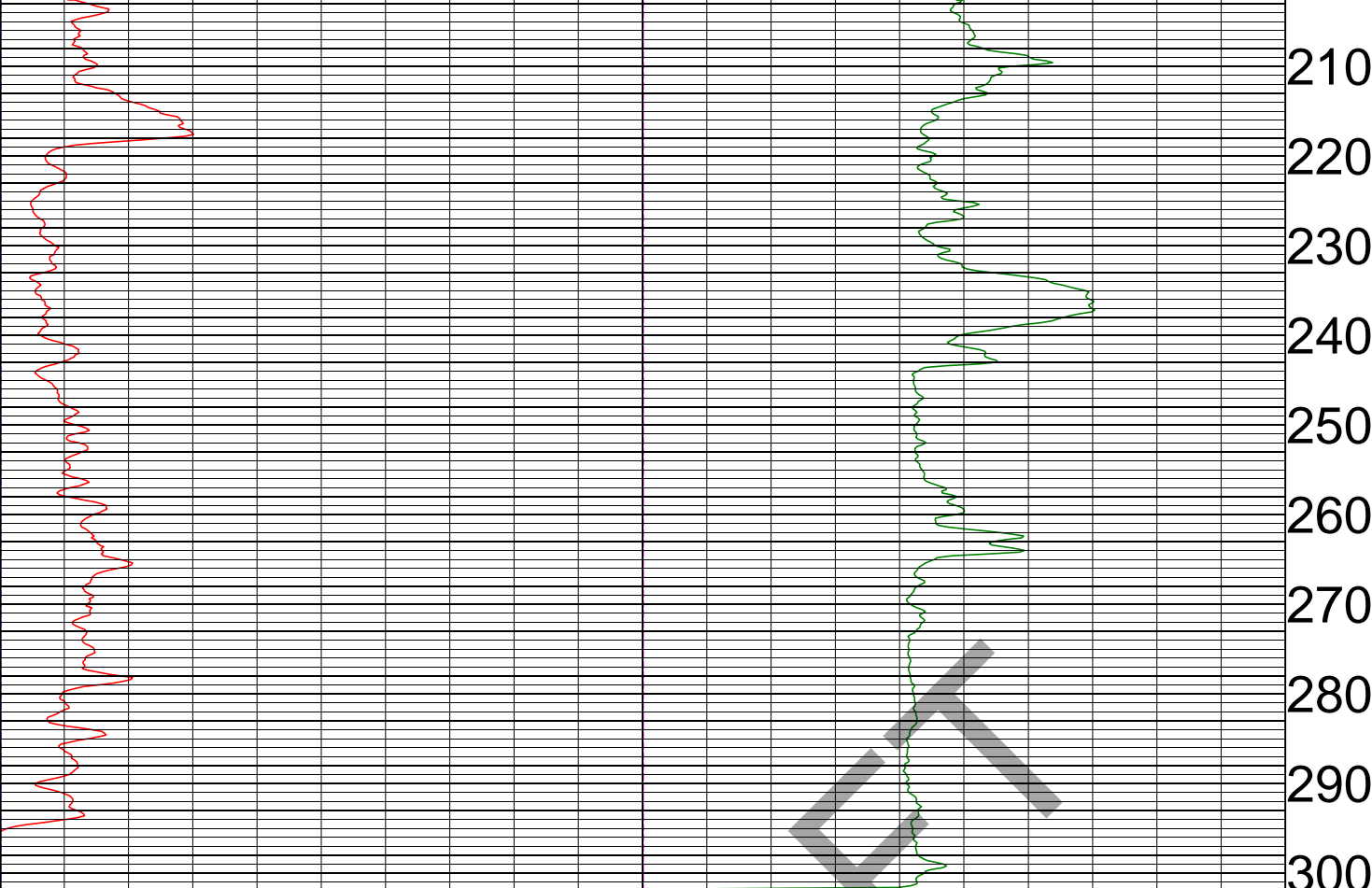
GAMMA		CALIPER	
200	API - GR	30	INCH
400		60	
GAMMA		CALIPER	
0	API - GR	0	INCH
200		30	

FEET



10  
20  
30  
40  
50  
60  
70  
80  
90  
100  
110  
120  
130  
140  
150  
160  
170  
180  
190  
200

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0	API-GR	200	0	INCH	30
	GAMMA			CALIPER	
200	API-GR	400	30	INCH	60
	GAMMA			CALIPER	

FEET

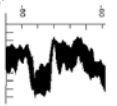
TOOL CALIBRATION WELL 6 04/16/20 09:02

TOOL 9074A1 TM VERSION 2004

SERIAL 3158

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jul16,19	11:01:03	GAMMA [API-GR]	1.000	340.000	0.000	365
2	Jul16,19	11:04:12	CALIPER [INCH]	4.000	6.000	69017	85360
3	Oct03,19	08:06:13	CALIPERL [INCH]	8.000	12.000	82709	107387
4	Jul16,19	11:00:52	CALIPERX [CPS]	Default	Default	Default	Default

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# COMBINATION LOG STATIC WATER QUALITY WELL 6

COMPANY	: APPLIED DRILLING ENG.	OTHER SERVICES:
WELL	: WELL 6	8044
FIELD	:	8711
COUNTY	: PINELLAS	VIDEO
STATE	: FLORIDA	

LOCATION	: BELLEAIRE
SECTION	:
TOWNSHIP	:
RANGE	:
API NO.	:
UNIQUE WELL ID.	: NONE

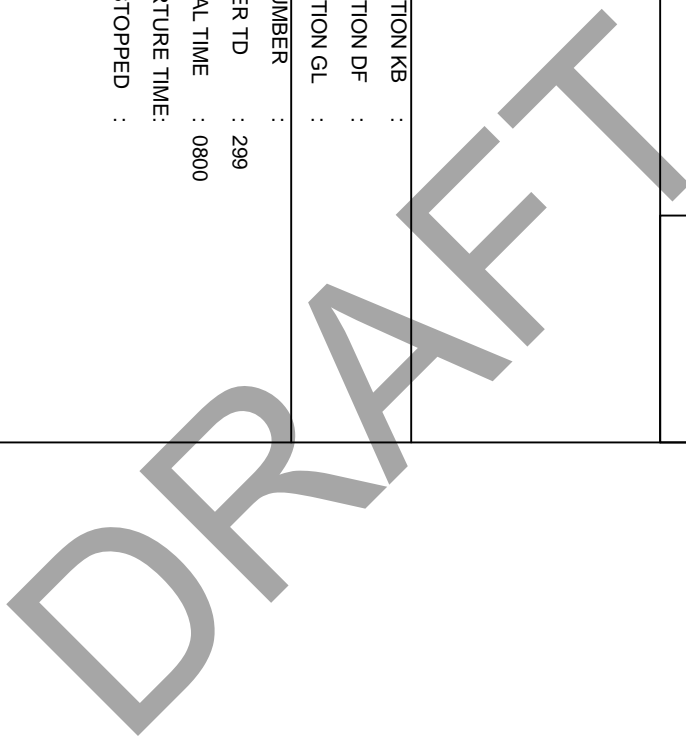
PERMANENT DATUM	: MSL	ELEVATION KB	:
LOG MEASURED FROM:	PAD	ELEVATION DF	:
DRL MEASURED FROM:	NA	ELEVATION GL	:

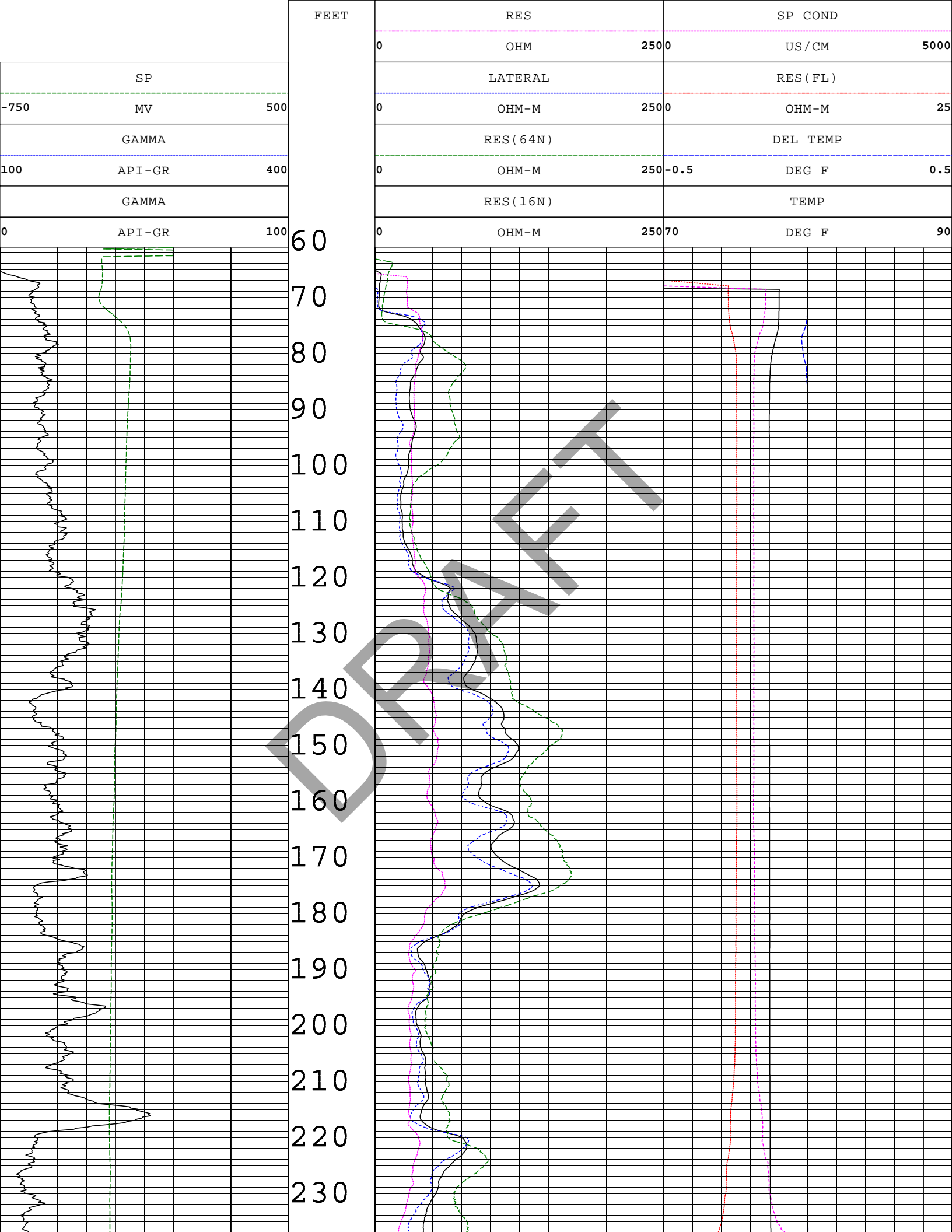
DATE	: 04/16/20	RIG NUMBER	:
DEPTH DRILLER	: NA	LOGGER TD	: 299
BIT SIZE	: 6	ARRIVAL TIME	: 0800
LOG TOP	: 61.20	DEPARTURE TIME:	
LOG BOTTOM	: 298.00	CIRC STOPPED	:

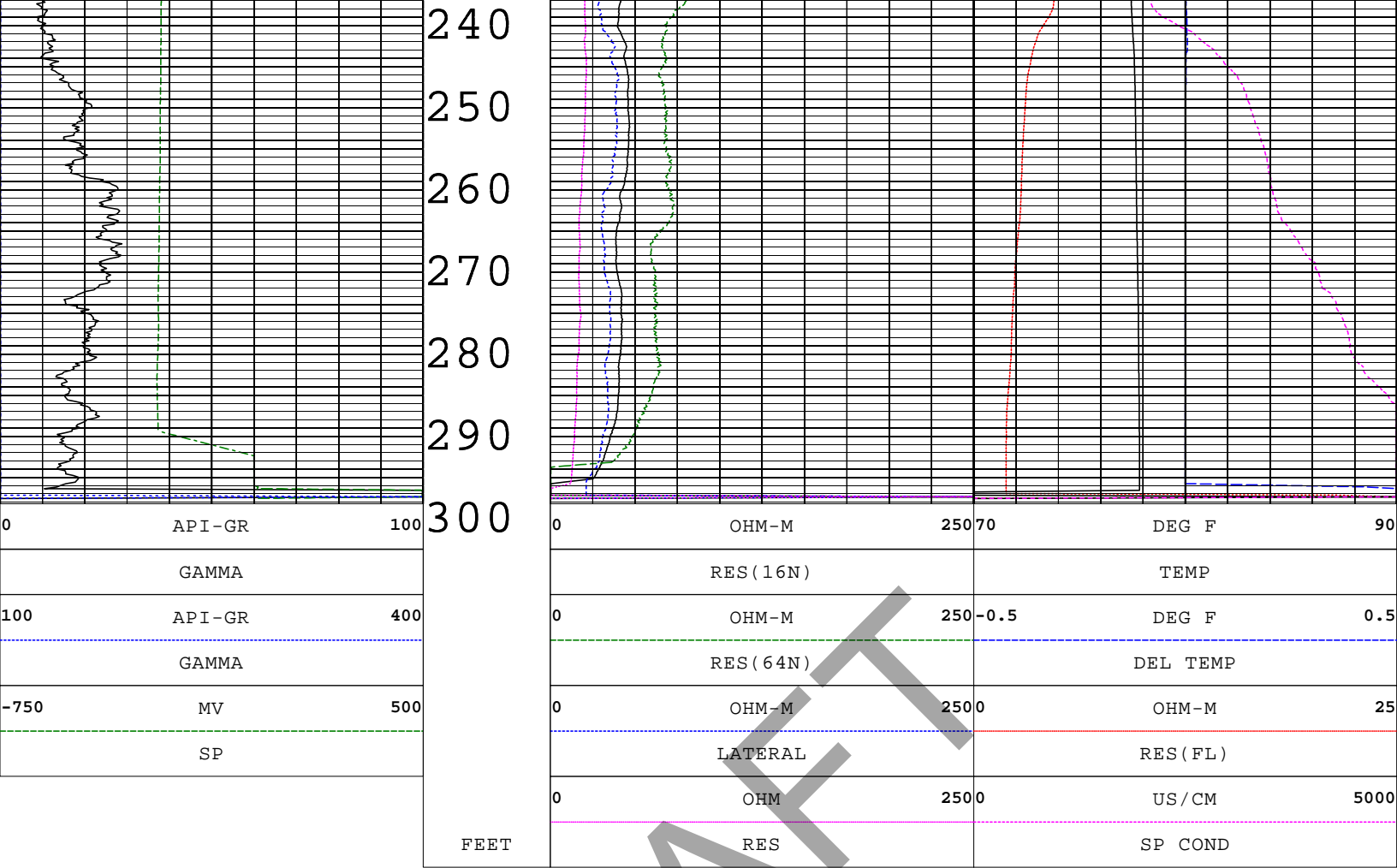
CASING OD	: 12
CASING BOTTOM	: XX
CASING TYPE	: STEEL
BOREHOLE FLUID	: FOR
RM TEMPERATURE	:
MUD RES	:
MUD WEIGHT	:
WITNESSED BY	: MAC DADDY
RECORDED BY	: AFB

REMARKS 1	:
REMARKS 2	:

ALL SERVICES PROVIDED SUBJECT TO STANDARD TERMS AND CONDITIONS





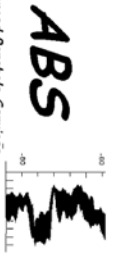


DRAFT

TOOL CALIBRATION WELL 6 04/16/20 09:41  
TOOL 8044A TM VERSION 2002  
SERIAL NUMBER 938

	DATE	TIME	SENSOR		STANDARD		RESPONSE
1	Jan03,03	02:49:05	GAMMA	0.001	[API-GR ]	0.000	[CPS]
	Jan03,03	02:49:05	GAMMA	180.000	[API-GR ]	169.000	[CPS]
2	Apr09,20	11:12:41	RES (FL	3.060	[OHM-M ]	13413.000	[CPS]
	Apr09,20	11:12:41	RES (FL	28.160	[OHM-M ]	37152.000	[CPS]
3	Aug17,14	12:00:23	SP	0.000	[MV ]	59670.000	[CPS]
	Aug17,14	12:00:23	SP	395.000	[MV ]	23612.000	[CPS]
4	Feb02,20	14:59:18	RES (16I	0.000	[OHM-M ]	4010.000	[CPS]
	Feb02,20	14:59:18	RES (16I	1996.000	[OHM-M ]	103211.000	[CPS]
5	Feb02,20	15:00:15	RES (64I	0.000	[OHM-M ]	4089.000	[CPS]
	Feb02,20	15:00:15	RES (64I	1990.000	[OHM-M ]	103487.000	[CPS]
6	Sep29,19	18:57:40	TEMP	71.700	[DEG F ]	63355.000	[CPS]
	Sep29,19	18:57:40	TEMP	86.100	[DEG F ]	57070.000	[CPS]
7	Aug17,14	10:39:11	RES	0.000	[OHM ]	9855.000	[CPS]
	Aug17,14	10:39:11	RES	988.000	[OHM ]	58788.000	[CPS]

DRAFT



# PRODUCTION-STATIC-PUMPING

## WELL 6

COMPANY	: APPLIED DRILLING ENG.	OTHER SERVICES:
WELL	: WELL 6	8044
FIELD	:	8711
COUNTY	: PINELLAS	VIDEO
STATE	: FLORIDA	

LOCATION	: BELLEAIRE
SECTION	:
TOWNSHIP	:
RANGE	:
API NO.	:
UNIQUE WELL ID.	: NONE

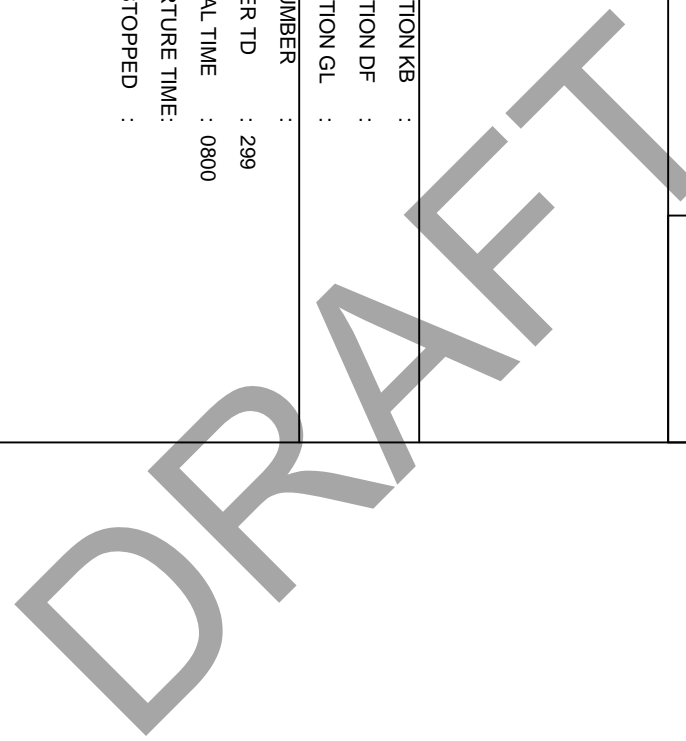
PERMANENT DATUM	: MSL	ELEVATION KB	:
LOG MEASURED FROM:	PAD	ELEVATION DF	:
DRL MEASURED FROM:	NA	ELEVATION GL	:

DATE	: 04/16/20	RIG NUMBER	:
DEPTH DRILLER	: NA	LOGGER TD	: 299
BIT SIZE	: 6	ARRIVAL TIME	: 0800
LOG TOP	: 4.20	DEPARTURE TIME:	
LOG BOTTOM	: 302.00	CIRC STOPPED	:

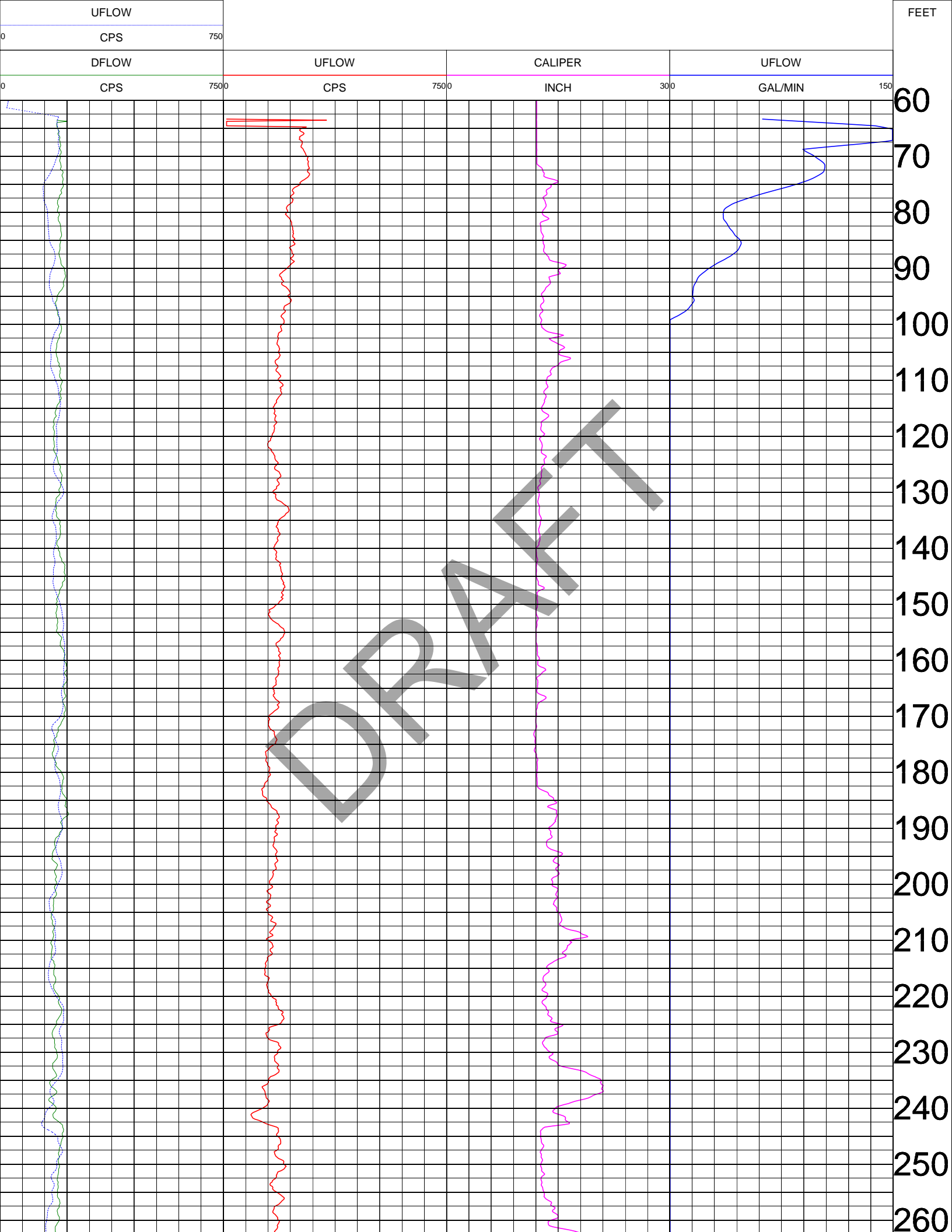
CASING OD	: 12
CASING BOTTOM	: XX
CASING TYPE	: STEEL
BOREHOLE FLUID	: FOR
RM TEMPERATURE	:
MUD RES	:
MUD WEIGHT	:
WITNESSED BY	: MAC DADDY
RECORDED BY	: AFB

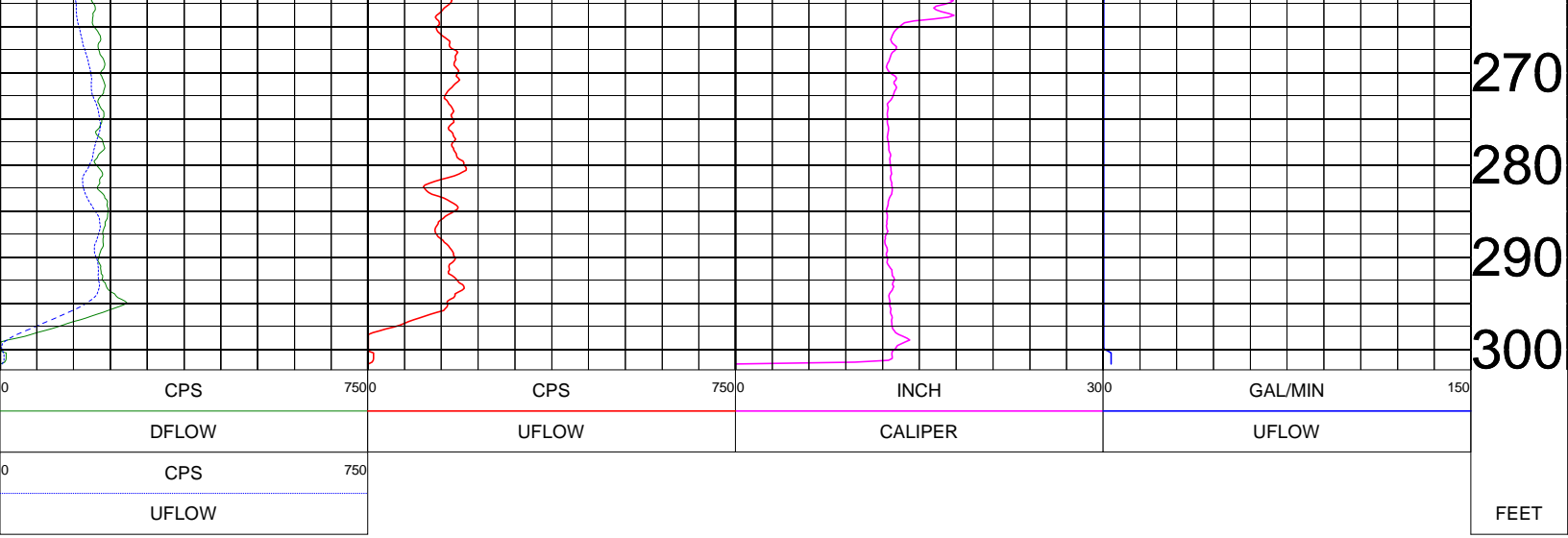
REMARKS 1	:
REMARKS 2	:

ALL SERVICES PROVIDED SUBJECT TO STANDARD TERMS AND CONDITIONS

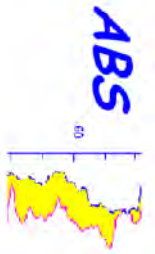








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# PUMPING WATER QUALITY WELL 6

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 6  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY ---  
 APH NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 6  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : ---  
 API NO. :  
 UNIQ ID : NONE

SECTION :  
 LOCATION : BELLEAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY7\_JL56

PERMANENT DATUM MSL  
 DRI MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

Elevations:  
 KB  
 DF  
 GL

FT  
 FT  
 FT

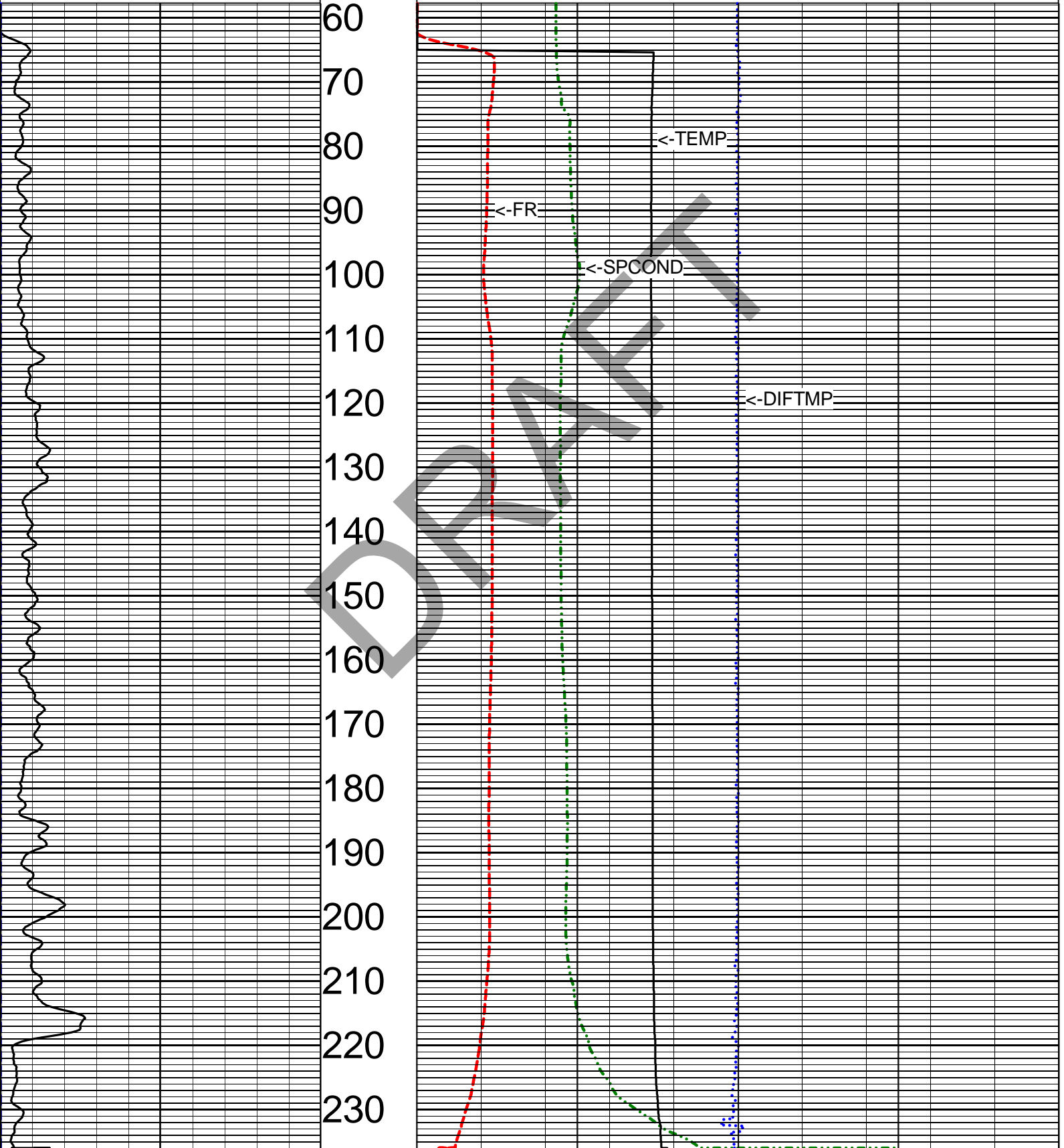
Other Services:  
 8044  
 8711  
 VIDEO

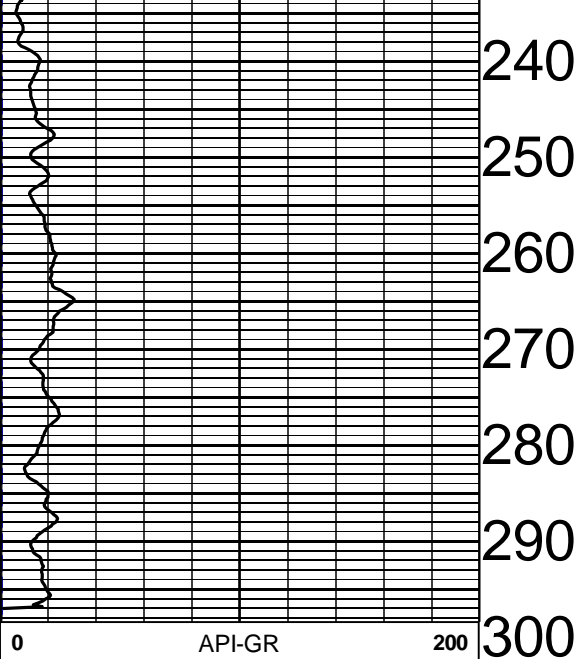
DATE	TIME			
DEPTH DRILLER	NA	FT		
DEPTH LOGGER	299	FT		
FIRST READING		FT		
LAST READING		FT		
BIT SIZE	6	IN		
CASING -- DRILLER	XX	FT		
CASING -- LOGGER		FT		
CASING O.D.	12	IN		
CASING TYPE	STEEL			
FLUID TYPE	FOR			
FLUID DENSITY			LB/GAL	
FLUID VISCOSITY				
FLUID PH				
MUD SOURCE				
RM @ MEAS TEMP	@ F			
RMF @ MEAS TEMP	@ F			
RMC @ MEAS TEMP	@ F			
CIRC STOPPED				
RIG NUMBER				
RECORDED BY	AFB			
WITNESSED BY	MAC DADDY			
REMARKS 1				
REMARKS 2				
REMARKS 3				

FEET

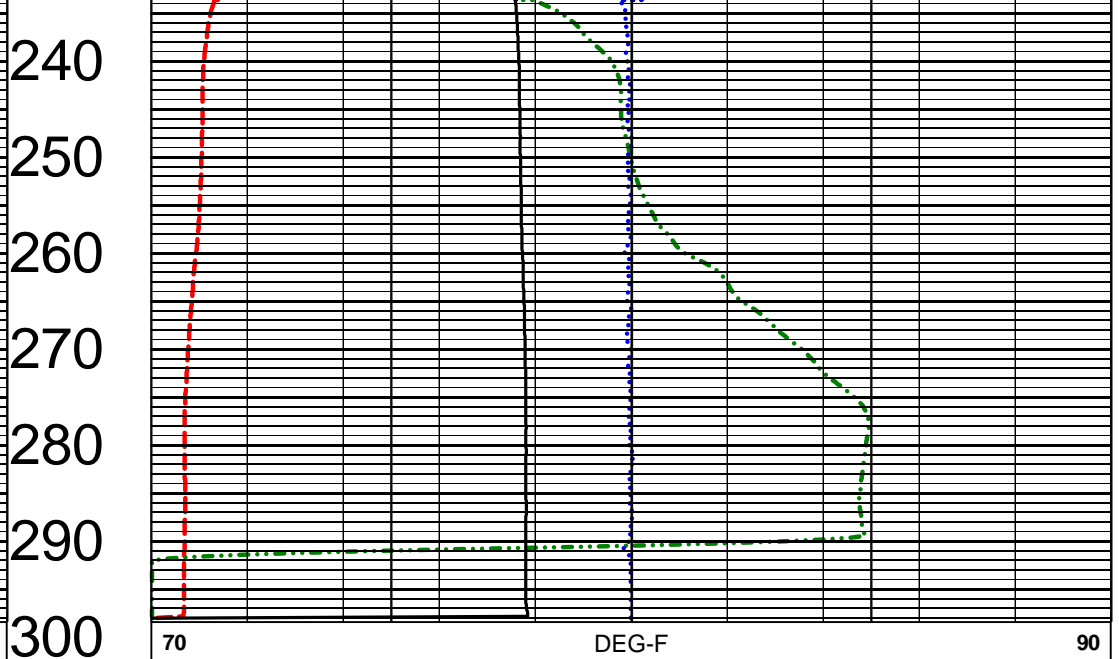
0	SP_COND	7500
0	RES(FL)	50
-0.25	DEL_TEMP	0.25
70	TEMP	90

200	GAMMA	400
0	API-GR	200





0	API-GR	200
	GAMMA	
200	API-GR	400
	GAMMA	



	TEMP	
-0.25	DEG-F	0.25
	DEL_TEMP	
0	OHM-M	50
	RES(FL)	
0	US/CM	7500
	SP_COND	

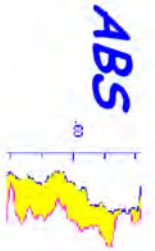
FEET

DRAFT

TOOL CALIBRATION WELL 6 04/16/20 10:04  
TOOL 8044A TM VERSION 2002  
SERIAL NUMBER 938

	DATE	TIME	SENSOR		STANDARD		RESPONSE
1	Jan03,03	02:49:05	GAMMA	0.001	[API-GR ]	0.000	[CPS]
	Jan03,03	02:49:05	GAMMA	180.000	[API-GR ]	169.000	[CPS]
2	Apr09,20	11:12:41	RES(FL)	3.060	[OHM-M ]	13413.000	[CPS]
	Apr09,20	11:12:41	RES(FL)	28.160	[OHM-M ]	37152.000	[CPS]
3	Aug17,14	12:00:23	SP	0.000	[MV ]	59670.000	[CPS]
	Aug17,14	12:00:23	SP	395.000	[MV ]	23612.000	[CPS]
4	Feb02,20	14:59:18	RES(16N)	0.000	[OHM-M ]	4010.000	[CPS]
	Feb02,20	14:59:18	RES(16N)	1996.000	[OHM-M ]	103211.000	[CPS]
5	Feb02,20	15:00:15	RES(64N)	0.000	[OHM-M ]	4089.000	[CPS]
	Feb02,20	15:00:15	RES(64N)	1990.000	[OHM-M ]	103487.000	[CPS]
6	Sep29,19	18:57:40	TEMP	71.700	[DEG F ]	63355.000	[CPS]
	Sep29,19	18:57:40	TEMP	86.100	[DEG F ]	57070.000	[CPS]
7	Aug17,14	10:39:11	RES	0.000	[OHM ]	9855.000	[CPS]
	Aug17,14	10:39:11	RES	988.000	[OHM ]	58788.000	[CPS]

DRAFT



**GAMMA RAY (API) - CALIPER  
WELL 7**

COMPANY APPLIED DRILLING ENG.  
WELL WELL 7  
WELL EXT  
FIELD  
COUNTY PINELLAS  
STATE FLORIDA  
COUNTRY USA  
API NO.

COMPANY : APPLIED DRILLING ENG.  
WELL : WELL 7  
WELL EXT :  
FIELD :  
COUNTY : PINELLAS  
STATE : FLORIDA  
COUNTRY : USA  
API NO. :  
UNIQ ID : NONE

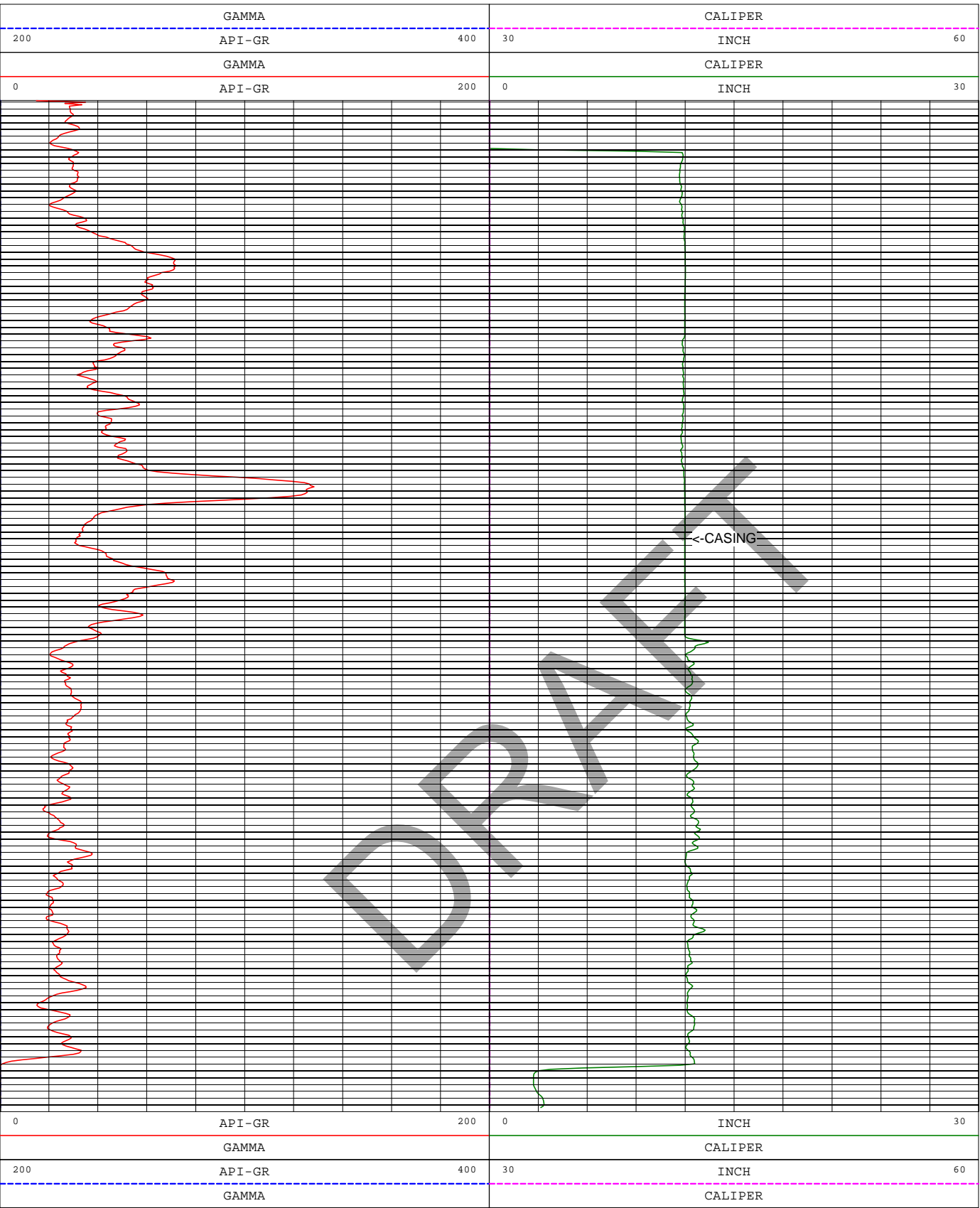
SECTION :  
LOCATION : BELLEAIR  
LAT GPS UTM  
LON GPS UTM  
TOWNSHIP :  
RANGE :

DISPLAY7\_JL56

PERMANENT DATUM	MSL	Elevations:	Other Services:
DRL MEASURED FROM	NA	KB	FT 8044
LOG MEASURED FROM	PAD	DF	FT 8711
ELEV. PERM. DATUM	FT	GL	FT VIDEO

DATE	TIME	DEPTH DRILLER	DEPTH LOGGER	FIRST READING	LAST READING	BIT SIZE	CASING -- DRILLER	CASING -- LOGGER	CASING O.D.	CASING TYPE	FLUID TYPE	FLUID DENSITY	FLUID VISCOSITY	FLUID PH	MUD SOURCE	RM @ MEAS TEMP	RMF @ MEAS TEMP	RMC @ MEAS TEMP	CIRC STOPPED	RIG NUMBER
		NA	148	FT	FT	6	NA	80	12	STEEL	FOR		LB/GAL			@ F	@ F	@ F		

RECORDED BY AFB  
WITNESSED BY MAC DADDY  
REMARKS 1  
REMARKS 2  
REMARKS 3



F E E T

0  
10  
20  
30  
40  
50  
60  
70  
80  
90  
100  
110  
120  
130  
140  
150

F E E T



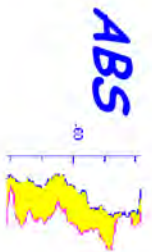
TOOL CALIBRATION WELL 7 05/13/20 09:05

TOOL 9074A1 TM VERSION 2004

SERIAL 3158

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jul16,19	11:01:03	GAMMA [API-GR]	1.000	340.000	0.000	365
2	Jul16,19	11:04:12	CALIPER [INCH]	4.000	6.000	69017	85360
3	May06,20	12:24:06	CALIPERL [INCH]	8.000	12.000	85899	107387
4	Jul16,19	11:00:52	CALIPERX [CPS]	Default	Default	Default	Default

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# COMBINATION LOG

## WELL 7

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 7  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 7  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE

SECTION :  
 LOCATION : BELLEAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY7\_JL56

PERMANENT DATUM  
 DRL MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

MSL  
 NA  
 PAD  
 FT

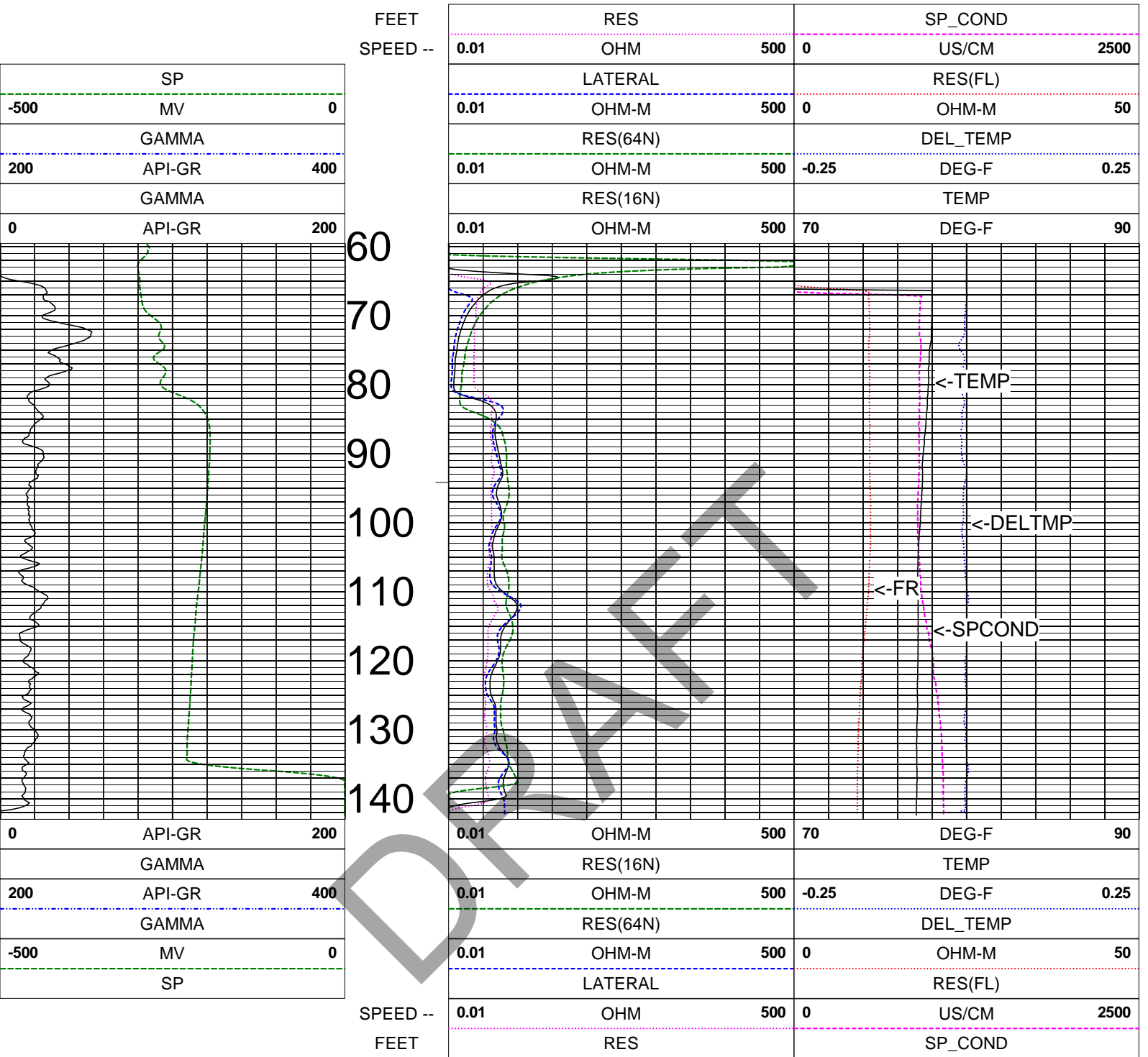
Elevations:  
 KB  
 DF  
 GL

FT  
 FT  
 FT  
 FT  
 VIDEO

Other Services:  
 8044  
 8711

DATE	TIME	DEPTH DRILLER	DEPTH LOGGER	FIRST READING	LAST READING	BIT SIZE	CASING -- DRILLER	CASING -- LOGGER	CASING O.D.	CASING TYPE	FLUID TYPE	FLUID DENSITY	FLUID VISCOSITY	FLUID PH	MUD SOURCE	RM @ MEAS TEMP	RMF @ MEAS TEMP	RMC @ MEAS TEMP	CIRC STOPPED	RIG NUMBER
		NA	148	FT	FT	6	NA	80	12	STEEL	FOR		LB/GAL			@ F	@ F	@ F		

RECORDED BY AFB  
 WITNESSED BY MAC DADDY  
 REMARKS 1  
 REMARKS 2  
 REMARKS 3



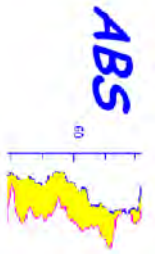
TOOL CALIBRATION WELL 7 05/13/20 09:23

TOOL 8044A TM VERSION 2002

SERIAL 938

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jan03,03	02:49:05	GAMMA [API-GR]	0.001	180.000	0.000	169
2	Apr09,20	11:12:41	RES(FL) [OHM-M]	3.060	28.160	13413	37152
3	Aug17,14	12:00:23	SP [MV]	0.000	395.000	59670	23612
4	Feb02,20	14:59:18	RES(16N) [OHM-M]	0.000	1996.000	4010	103211
5	Feb02,20	15:00:15	RES(64N) [OHM-M]	0.000	1990.000	4089	103487
6	Sep29,19	18:57:40	TEMP [DEG-F]	71.700	86.100	63355	57070
7	Aug17,14	10:39:11	RES [OHM]	0.000	988.000	9855	58788

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# PRODUCTION - STATIC - PUMPING WELL 7

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 7  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 APH NO.

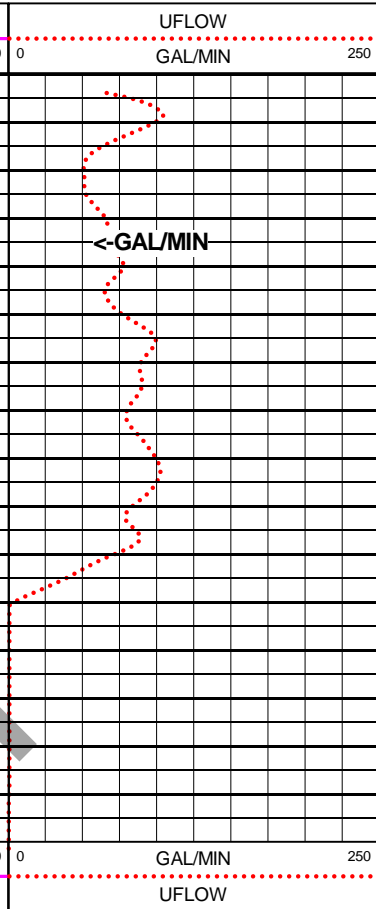
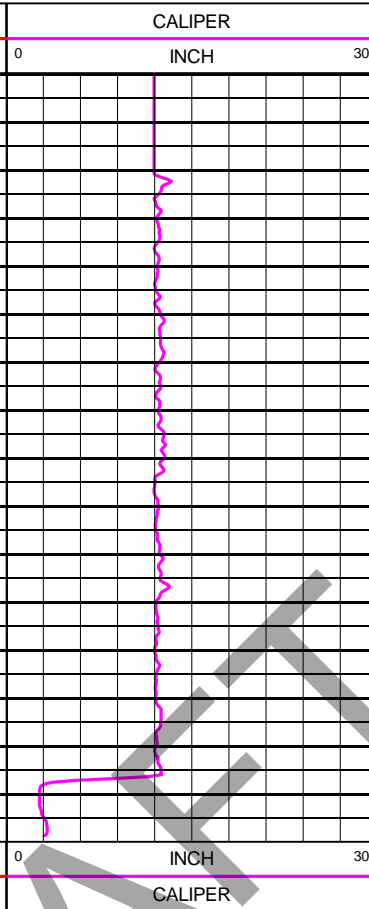
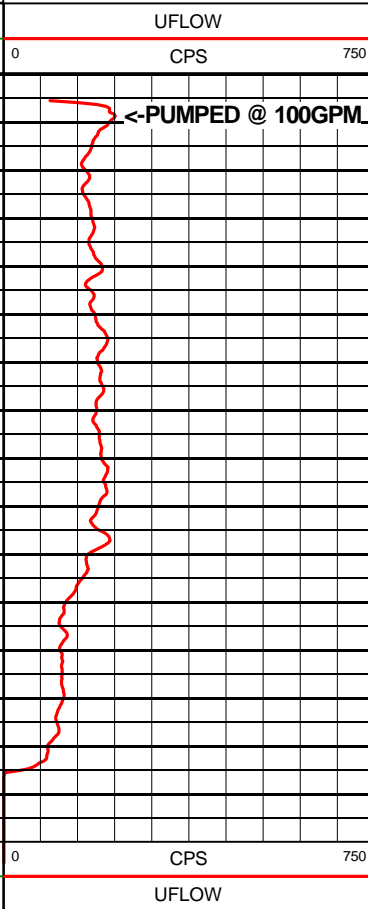
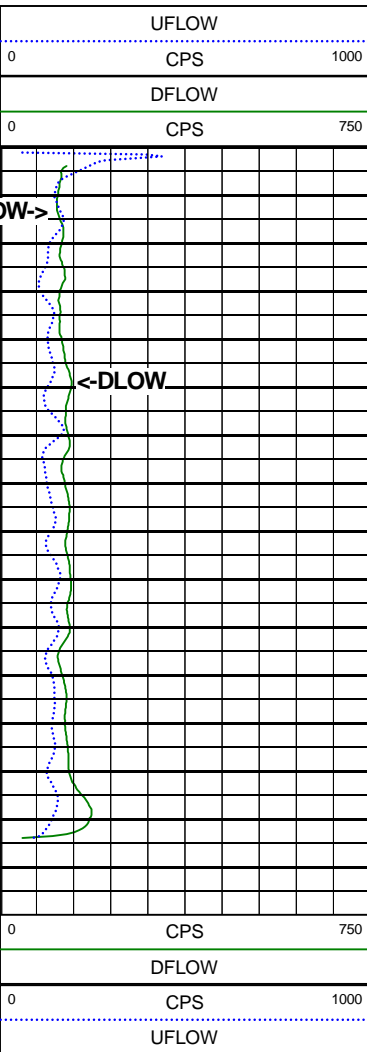
COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 7  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE

SECTION :  
 LOCATION : BELLEAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY7\_JL56

TOWNSHIP :  
 RANGE :

PERMANENT DATUM MSL Elevations:  
 DRI MEASURED FROM NA KB 8044  
 LOG MEASURED FROM PAD DF 8711  
 ELEV. PERM. DATUM FT FT VIDEO

DATE	TIME	DEPTH DRILLER	FT	Other Services:
		DEPTH LOGGER	148	
		FIRST READING		
		LAST READING		
		BIT SIZE	6	
		CASING -- DRILLER	NA	
		CASING -- LOGGER	80	
		CASING O.D.	12	
		CASING TYPE	STEEL	
		FLUID TYPE	FOR	
		FLUID DENSITY		
		FLUID VISCOSITY		
		FLUID PH		
		MUD SOURCE		
		RM @ MEAS TEMP	@ F	
		RMF @ MEAS TEMP	@ F	
		RMG @ MEAS TEMP	@ F	
		CIRC STOPPED		
		RIG NUMBER		
		RECORDED BY	AFB	
		WITNESSED BY	MAC DADDY	
		REMARKS 1		
		REMARKS 2		
		REMARKS 3		



FEET

70

80

90

100

110

120

130

140

150

FEET

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# PUMPING WATER QUALITY WELL 7

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 7  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

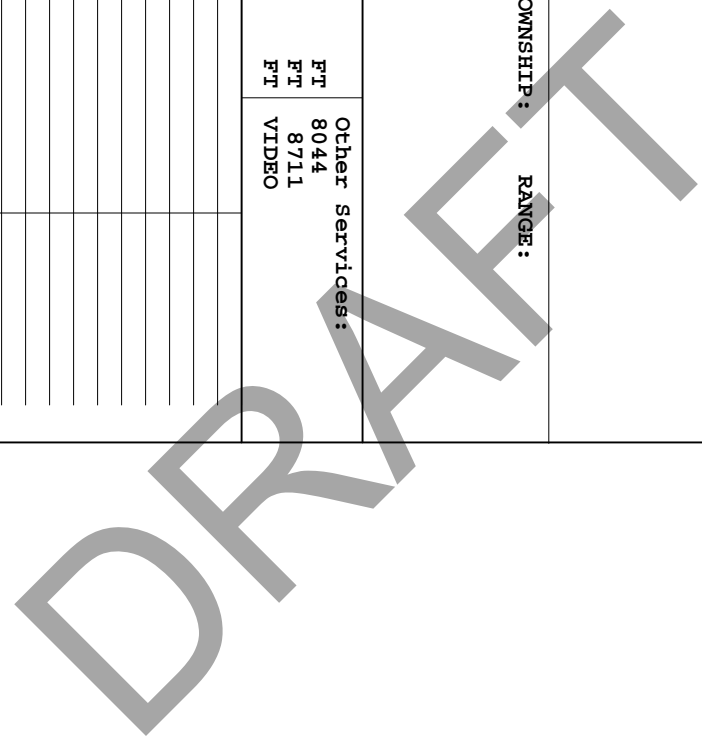
COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 7  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE

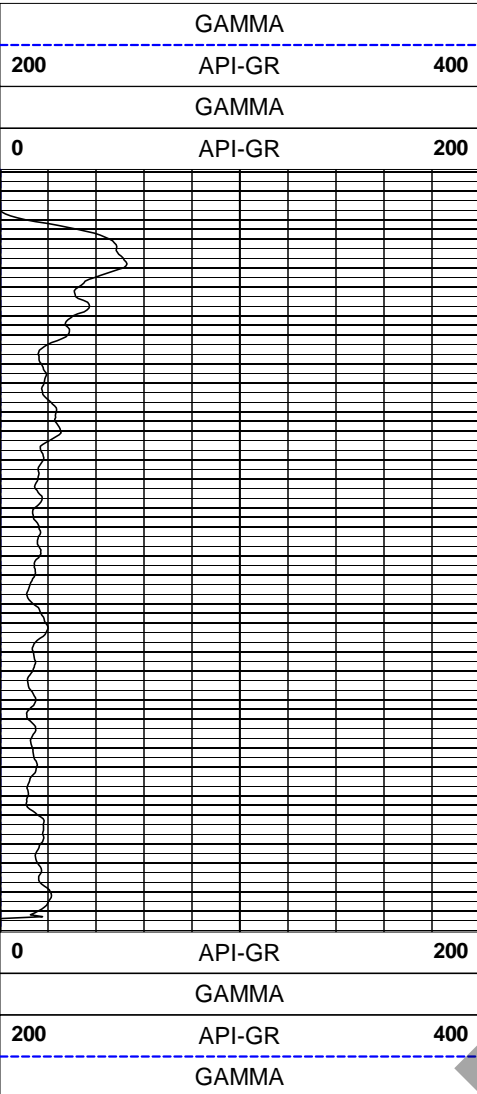
SECTION :  
 LOCATION : BELLEAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY7\_JL56

PERMANENT DATUM MSL Elevations:  
 DRL MEASURED FROM NA NA KB  
 LOG MEASURED FROM PAD PAD DF  
 ELEV. PERM. DATUM FT FT  
 Other Services:  
 8044  
 8711  
 VIDEO

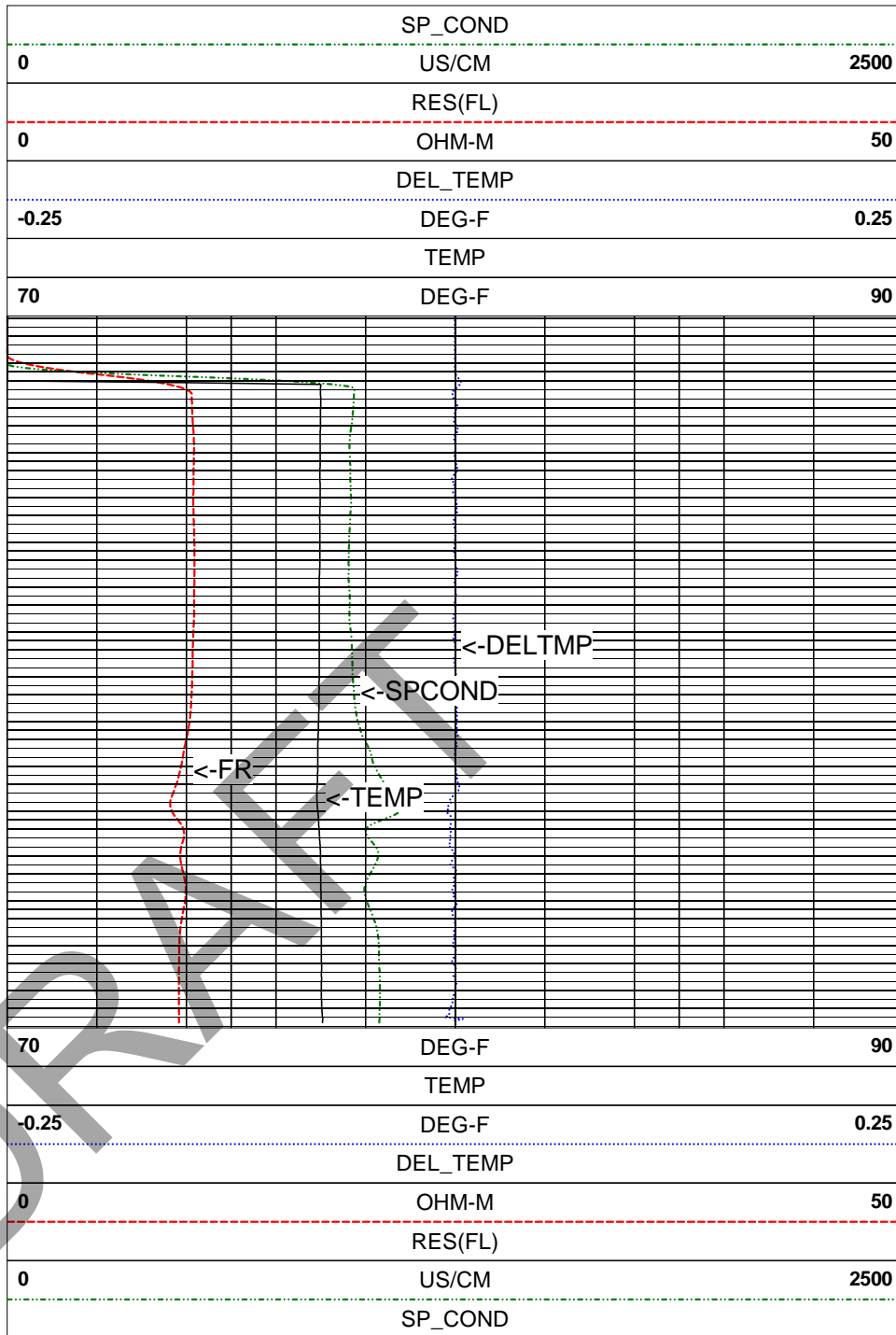
DATE	TIME	FT	FT	FT	FT
DEPTH DRILLER	NA	FT			
DEPTH LOGGER	148	FT			
FIRST READING		FT			
LAST READING		FT			
BIT SIZE	6	IN			
CASING -- DRILLER	NA	FT			
CASING -- LOGGER	80	FT			
CASING O.D.	12	IN			
CASING TYPE	STEEL				
FLUID TYPE	FOR				
FLUID DENSITY		LB/GAL			
FLUID VISCOSITY					
FLUID PH					
MUD SOURCE					
RM @ MEAS TEMP	@ F				
RMF @ MEAS TEMP	@ F				
RMG @ MEAS TEMP	@ F				
CIRC STOPPED					
RIG NUMBER					

RECORDED BY AFB  
 WITNESSED BY MAC DADDY  
 REMARKS 1  
 REMARKS 2  
 REMARKS 3





FEET



FEET



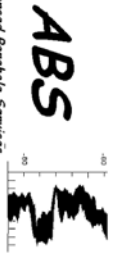
TOOL CALIBRATION WELL 7 05/13/20 09:57

TOOL 8044A TM VERSION 2002

SERIAL 938

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jan03,03	02:49:05	GAMMA [API-GR]	0.001	180.000	0.000	169
2	Apr09,20	11:12:41	RES(FL) [OHM-M]	3.060	28.160	13413	37152
3	Aug17,14	12:00:23	SP [MV]	0.000	395.000	59670	23612
4	Feb02,20	14:59:18	RES(16N) [OHM-M]	0.000	1996.000	4010	103211
5	Feb02,20	15:00:15	RES(64N) [OHM-M]	0.000	1990.000	4089	103487
6	Sep29,19	18:57:40	TEMP [DEG-F]	71.700	86.100	63355	57070
7	Aug17,14	10:39:11	RES [OHM]	0.000	988.000	9855	58788

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# GAMMA RAY (API) - CALIPER WELL 9

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 9  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 9  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE

SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY7\_JL56

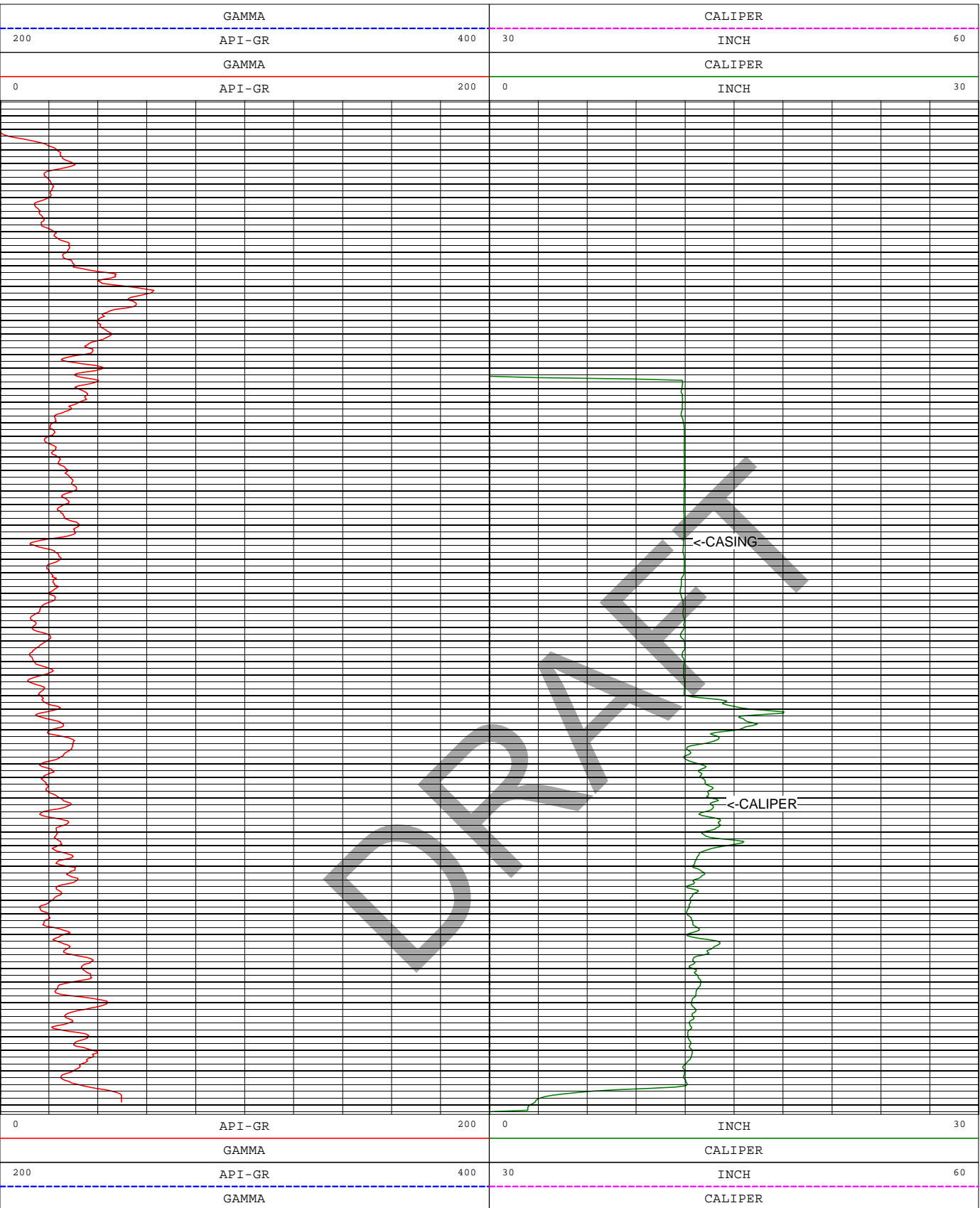
PERMANENT DATUM  
 DRL MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

MSL NA  
 NA  
 XXX

Elevations:  
 KB  
 DF  
 GL

Other Services:  
 FT 8044  
 DF 8711  
 FT  
 GL VIDEO

DATE	TIME	DEPTH DRILLER	DEPTH LOGGER	FIRST READING	LAST READING	BIT SIZE	CASING -- DRILLER	CASING -- LOGGER	CASING O.D.	CASING TYPE	FLUID TYPE	FLUID DENSITY	FLUID VISCOSITY	FLUID PH	MUD SOURCE	RM @ MEAS TEMP	RMF @ MEAS TEMP	RMC @ MEAS TEMP	CIRC STOPPED	RIG NUMBER	RECORDED BY	WITNESSED BY	REMARKS 1	REMARKS 2	REMARKS 3
		NA	XXX			6	NA	87	6	PVC						@ F	@ F	@ F			AFB	MAC DADDY			



FEET

0

10

20

30

40

50

60

70

80

90

100

110

120

130

140

150

<- GAMMA RAY

<- GRAY

<- CASING

<- CALIPER

FEET

TOOL CALIBRATION WELL 9 05/05/20 13:16

TOOL 9074A1 TM VERSION 2004

SERIAL 3158

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jul16,19	11:01:03	GAMMA [API-GR]	1.000	340.000	0.000	365
2	Jul16,19	11:04:12	CALIPER [INCH]	4.000	6.000	69017	85360
3	Oct03,19	08:06:13	CALIPERL [INCH]	8.000	12.000	82709	107387
4	Jul16,19	11:00:52	CALIPERX [CPS]	Default	Default	Default	Default

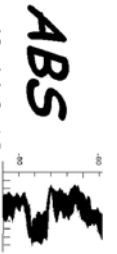
TOOL CALIBRATION WELL 9 Jul16,19 11:00:52

TOOL 8044A TM VERSION 0

SERIAL 938

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jan03,03	02:49:05	GAMMA [API-GR]	0.001	180.000	0.000	169
2	Apr09,20	11:12:41	RES(FL) [OHM-M]	3.060	28.160	13413	37152
3	Aug17,14	12:00:23	SP [MV]	0.000	395.000	59670	23612
4	Feb02,20	14:59:18	RES(16N) [OHM-M]	0.000	1996.000	4010	103211
5	Feb02,20	15:00:15	RES(64N) [OHM-M]	0.000	1990.000	4089	103487
6	Sep29,19	18:57:40	TEMP [DEG-F]	71.700	86.100	63355	57070
7	Aug17,14	10:39:11	RES [OHM]	0.000	988.000	9855	58788

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# COMBINATION LOG

## WELL 9

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 9  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 9  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE

SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY7\_JL56

PERMANENT DATUM  
 DRL MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

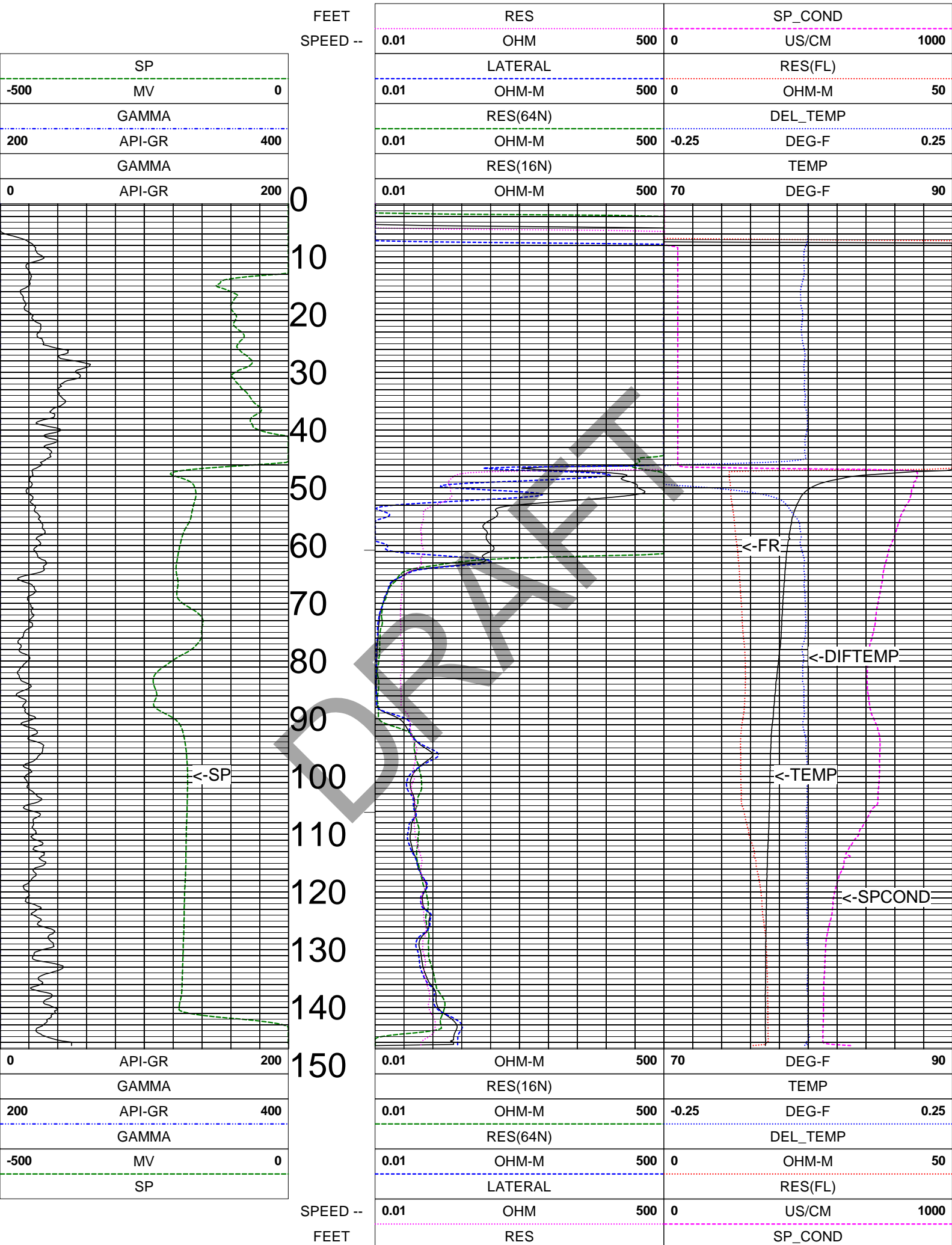
MSL  
 NA  
 PAD  
 FT

Elevations:  
 KB  
 DF  
 GL

Other Services:  
 FT 8044  
 DF 8711  
 FT  
 VIDEO

DATE	TIME	DEPTH DRILLER	DEPTH LOGGER	FIRST READING	LAST READING	BIT SIZE	CASING -- DRILLER	CASING -- LOGGER	CASING O.D.	CASING TYPE	FLUID TYPE	FLUID DENSITY	FLUID VISCOSITY	FLUID PH	MUD SOURCE	RM @ MEAS TEMP	RMF @ MEAS TEMP	RMC @ MEAS TEMP	CIRC STOPPED	RIG NUMBER	RECORDED BY	WITNESSED BY	REMARKS 1	REMARKS 2	REMARKS 3
		NA	XXX			6	NA	80	12	STEEL	FOR		LB/GAL			@ F	@ F	@ F			AFB	MAC DADDY			

ALL SERVICES PROVIDED SUBJECT TO STANDARD TERMS AND CONDITIONS



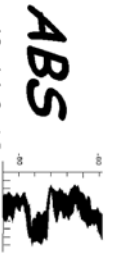
TOOL CALIBRATION WELL 9 05/05/20 13:34

TOOL 8044A TM VERSION 2002

SERIAL 938

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jan03,03	02:49:05	GAMMA [API-GR]	0.001	180.000	0.000	169
2	Apr09,20	11:12:41	RES(FL) [OHM-M]	3.060	28.160	13413	37152
3	Aug17,14	12:00:23	SP [MV]	0.000	395.000	59670	23612
4	Feb02,20	14:59:18	RES(16N) [OHM-M]	0.000	1996.000	4010	103211
5	Feb02,20	15:00:15	RES(64N) [OHM-M]	0.000	1990.000	4089	103487
6	Sep29,19	18:57:40	TEMP [DEG-F]	71.700	86.100	63355	57070
7	Aug17,14	10:39:11	RES [OHM]	0.000	988.000	9855	58788

DRAFT



# PRODUCTION-STATIC-PUMPING WELL 9

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 9  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 9  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE

SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY\_ J156

TOWNSHIP :  
 RANGE :

PERMANENT DATUM  
 DRL MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

MSL  
 NA  
 PAD  
 FT

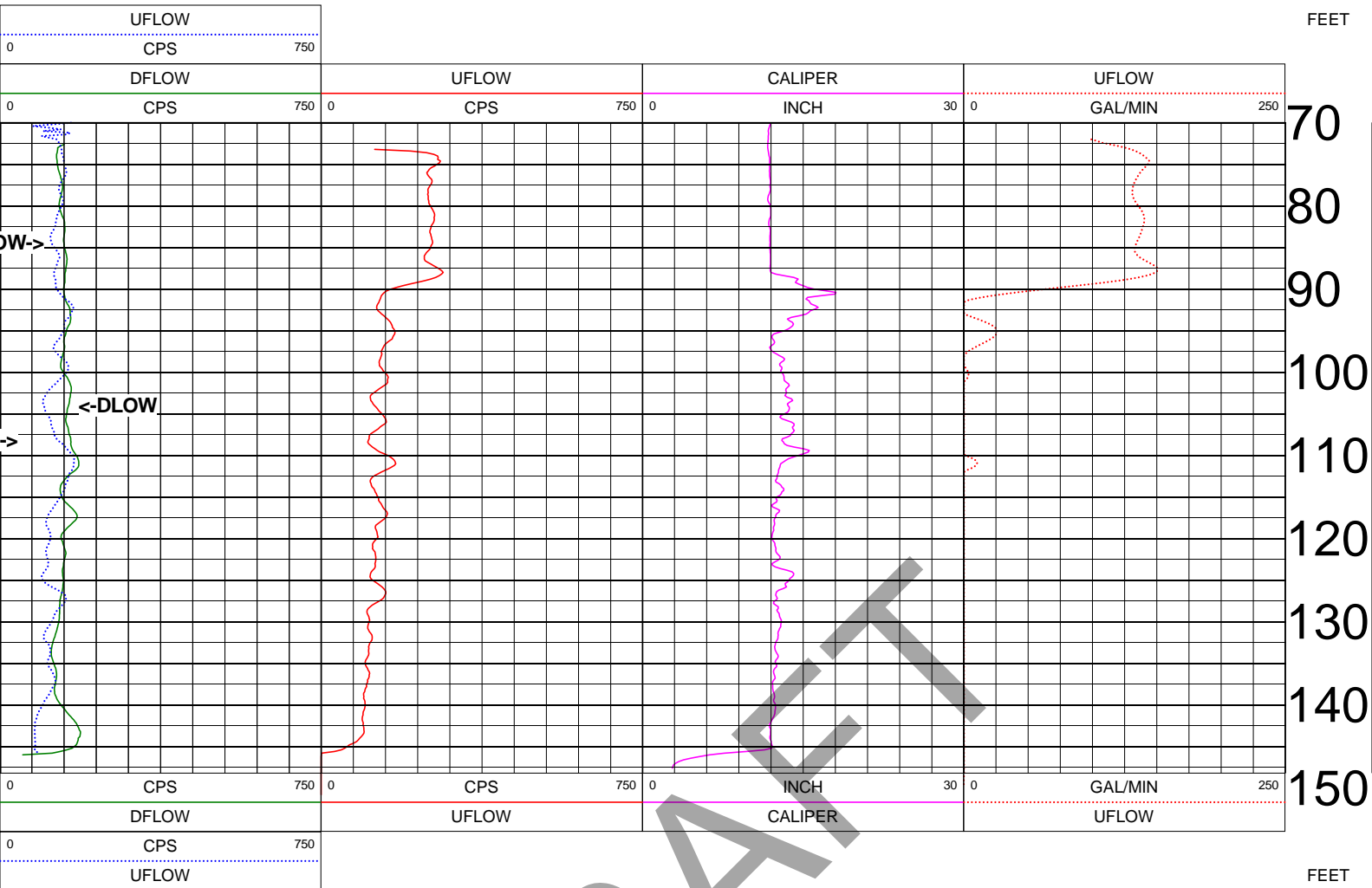
Elevations:  
 KB  
 DF  
 GL

FT  
 FT  
 FT

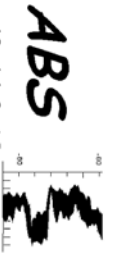
Other Services:  
 8044  
 8711  
 VIDEO

DATE	TIME	DEPTH DRILLER	DEPTH LOGGER	FIRST READING	LAST READING	BIT SIZE	CASING -- DRILLER	CASING -- LOGGER	CASING O.D.	CASING TYPE	FLUID TYPE	FLUID DENSITY	FLUID VISCOSITY	FLUID PH	MUD SOURCE	RM @ MEAS TEMP	RMF @ MEAS TEMP	RMC @ MEAS TEMP	CIRC STOPPED	RIG NUMBER	RECORDED BY	WITNESSED BY	REMARKS 1	REMARKS 2	REMARKS 3
		NA	148	FT	FT	6	NA	80	12	STEEL	FOR					@ F	@ F	@ F			AFB	MAC DADDY			





DRAFT



# PUMPING WATER QUALITY WELL 9

COMPANY APPLIED DRILLING ENG.  
 WELL WELL 9  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL 9  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE

SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY7\_JL56

TOWNSHIP :  
 RANGE :

PERMANENT DATUM  
 DRL MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

MSL  
 NA  
 PAD  
 FT

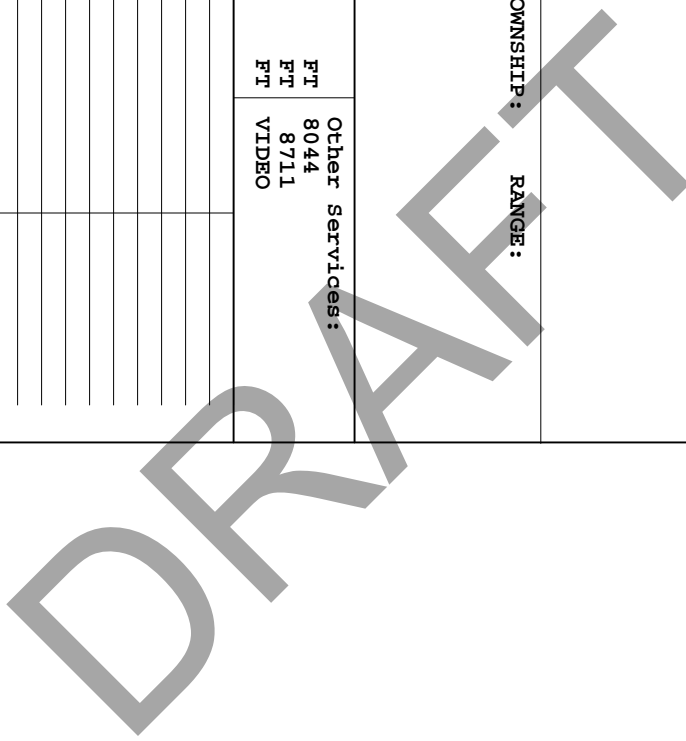
Elevations:  
 KB  
 DF  
 GL

FT  
 FT  
 FT

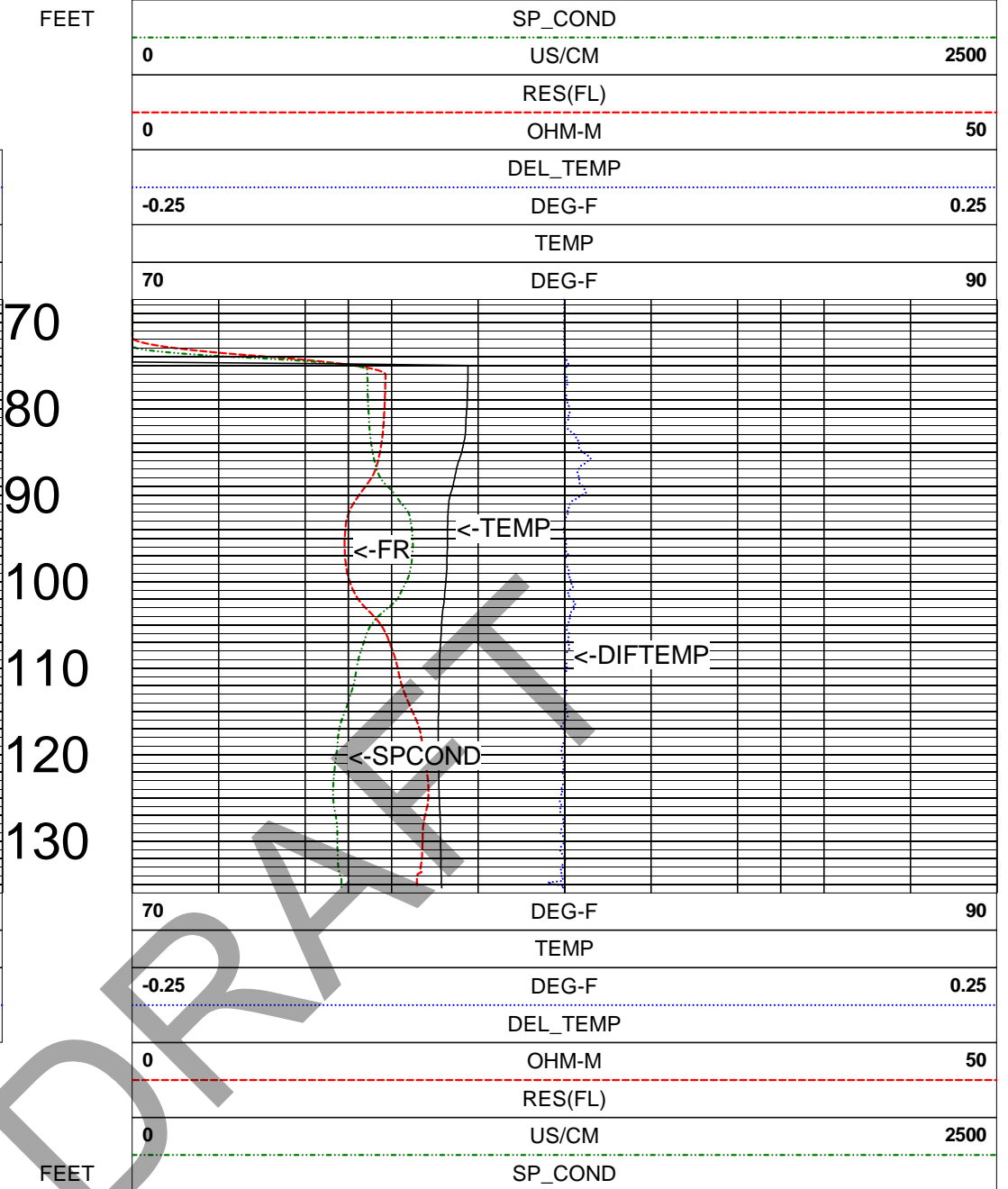
Other Services:  
 8044  
 8711  
 VIDEO

DATE	TIME	DEPTH DRILLER	DEPTH LOGGER	FIRST READING	LAST READING	BIT SIZE	CASING -- DRILLER	CASING -- LOGGER	CASING O.D.	CASING TYPE	FLUID TYPE	FLUID DENSITY	FLUID VISCOSITY	FLUID PH	MUD SOURCE	RM @ MEAS TEMP	RMF @ MEAS TEMP	RMG @ MEAS TEMP	CIRC STOPPED	RIG NUMBER	RECORDED BY	WITNESSED BY	REMARKS 1	REMARKS 2	REMARKS 3
		NA	148	FT	FT	6	NA	80	12	STEEL	FOR		LB/GAL			@ F	@ F	@ F			AFB	MAC DADDY			

ALL SERVICES PROVIDED SUBJECT TO STANDARD TERMS AND CONDITIONS



GAMMA		
200	API-GR	400
GAMMA		
0	API-GR	200
GAMMA		
0	API-GR	200
GAMMA		
200	API-GR	400
GAMMA		



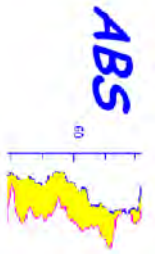
TOOL CALIBRATION WELL 9 05/05/20 13:56

TOOL 8044A TM VERSION 2002

SERIAL 938

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jan03,03	02:49:05	GAMMA [API-GR]	0.001	180.000	0.000	169
2	Apr09,20	11:12:41	RES(FL) [OHM-M]	3.060	28.160	13413	37152
3	Aug17,14	12:00:23	SP [MV]	0.000	395.000	59670	23612
4	Feb02,20	14:59:18	RES(16N) [OHM-M]	0.000	1996.000	4010	103211
5	Feb02,20	15:00:15	RES(64N) [OHM-M]	0.000	1990.000	4089	103487
6	Sep29,19	18:57:40	TEMP [DEG-F]	71.700	86.100	63355	57070
7	Aug17,14	10:39:11	RES [OHM]	0.000	988.000	9855	58788

DRAFT



# GAMMA RAY (API) - CALIPER WELL10

COMPANY APPLIED DRILLING ENG.  
 WELL WELL10  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL10  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE

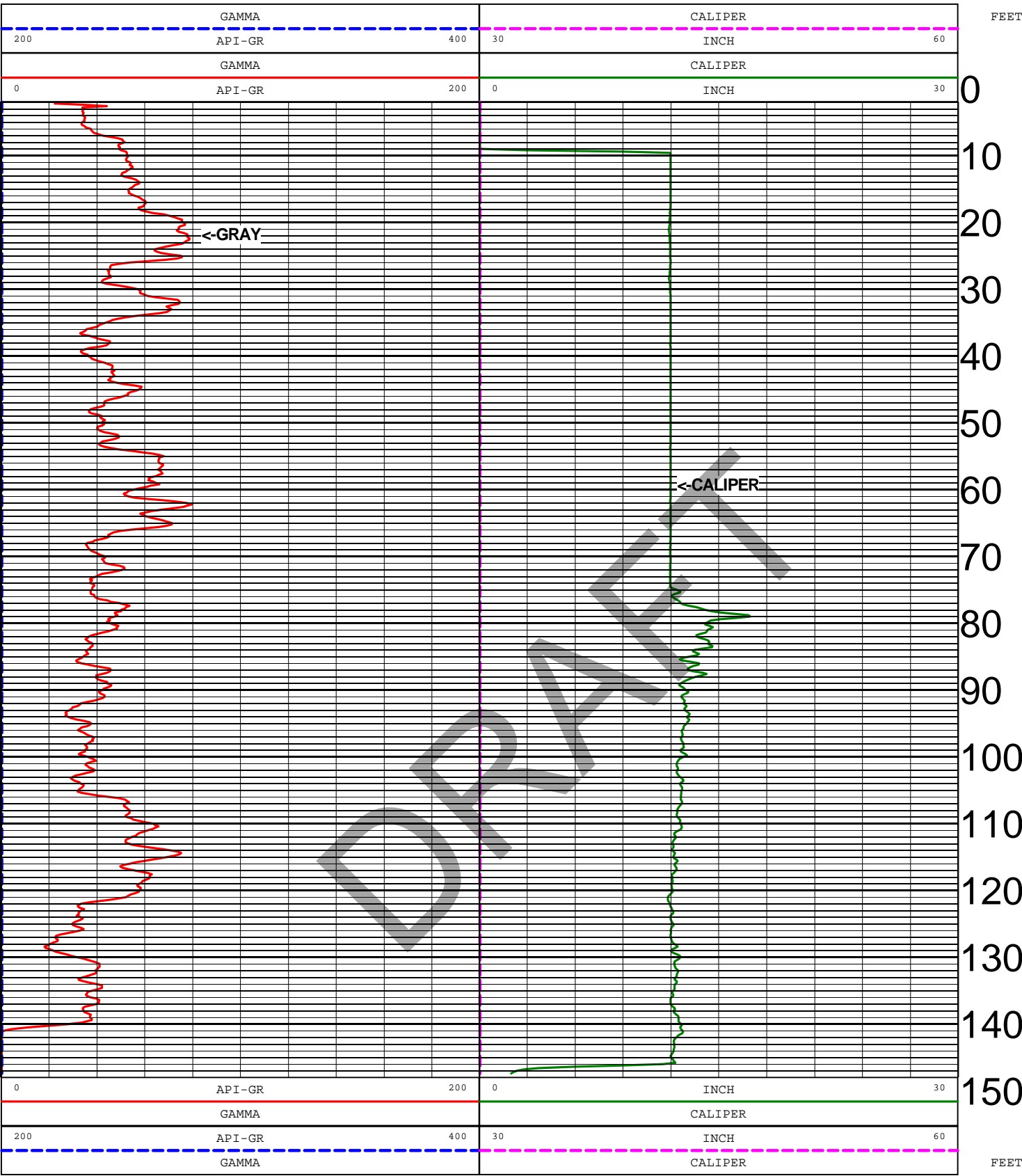
SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY7\_JL56

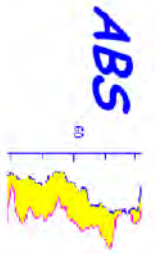
PERMANENT DATUM MSL  
 DRI MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

Elevations:  
 KB  
 DF  
 GL

Other Services:  
 FT 8044  
 DF 8711  
 FT VIDEO

DATE	TIME	DEPTH DRILLER	FT	FT	Other Services:
		DEPTH LOGGER	148	FT	
		FIRST READING		FT	
		LAST READING		FT	
		BIT SIZE	6	IN	
		CASING -- DRILLER	NA	FT	
		CASING -- LOGGER	74	FT	
		CASING O.D.	12	IN	
		CASING TYPE	STEEL		
		FLUID TYPE	FOR		
		FLUID DENSITY		LB/GAL	
		FLUID VISCOSITY			
		FLUID PH			
		MUD SOURCE			
		RM @ MEAS TEMP	@ F		
		RMF @ MEAS TEMP	@ F		
		RMC @ MEAS TEMP	@ F		
		CIRC STOPPED			
		RIG NUMBER			
		RECORDED BY	AFB		
		WITNESSED BY	MAC DADDY		
		REMARKS 1			
		REMARKS 2			
		REMARKS 3			





# GAMMA RAY (API) - CALIPER WELL10

COMPANY APPLIED DRILLING ENG.  
 WELL WELL10  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL10  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE  
 SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY7\_JL56  
 TOWNSHIP :  
 RANGE :

PERMANENT DATUM MSL  
 DRI MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

Elevations: FT 8044  
 KB DF FT 8711  
 GL VIDEO

DATE TIME

DEPTH DRILLER NA FT

DEPTH LOGGER 148 FT

FIRST READING FT

LAST READING FT

BIT SIZE 6 IN

CASING -- DRILLER NA FT

CASING -- LOGGER 74 FT

CASING O.D. 12 IN

CASING TYPE STEEL

FLUID TYPE FOR

FLUID DENSITY LB/GAL

FLUID VISCOSITY

FLUID PH

MUD SOURCE

RM @ MEAS TEMP @ F

RMC @ MEAS TEMP @ F

CIRC STOPPED

RIG NUMBER

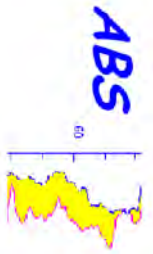
RECORDED BY AFB

WITNESSED BY MAC DADDY

REMARKS 1

REMARKS 2

REMARKS 3



# COMBINATION LOG

## STATIC WATER QUALITY

### WELL10

COMPANY APPLIED DRILLING ENG.  
 WELL WELL10  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL10  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE  
 SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY7\_JL56  
 TOWNSHIP :  
 RANGE :

PERMANENT DATUM MSL  
 DRI MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

Elevations:  
 KB FT 8044  
 DF FT 8711  
 GL FT VIDEO

DATE TIME

DEPTH DRILLER NA FT

DEPTH LOGGER 148 FT

FIRST READING FT

LAST READING FT

BIT SIZE 6 IN

CASING -- DRILLER NA FT

CASING -- LOGGER 74 FT

CASING O.D. 12 IN

CASING TYPE STEEL

FLUID TYPE FOR

FLUID DENSITY LB/GAL

FLUID VISCOSITY

FLUID PH

MUD SOURCE

RM @ MEAS TEMP @ F

RMC @ MEAS TEMP @ F

CIRC STOPPED

RIG NUMBER

RECORDED BY AFB

WITNESSED BY MAC DADDY

REMARKS 1

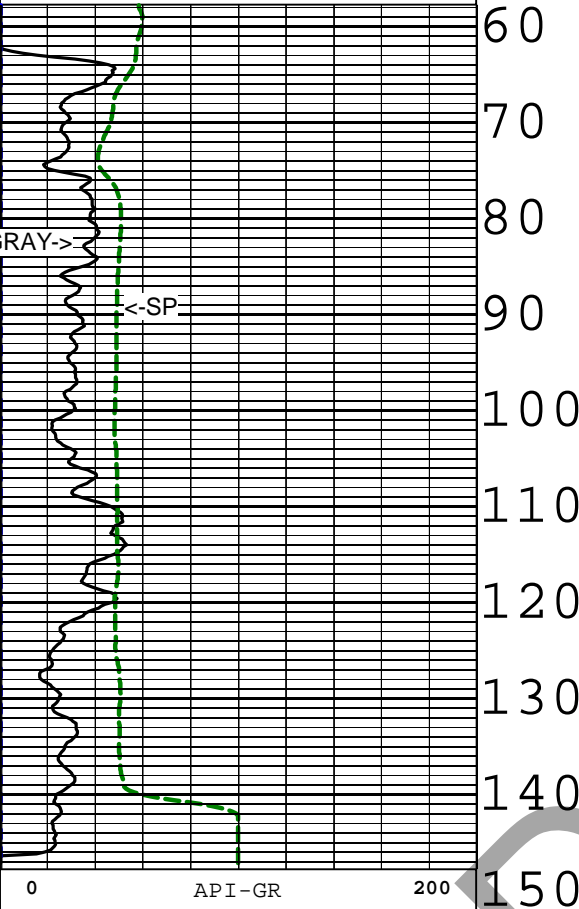
REMARKS 2

REMARKS 3

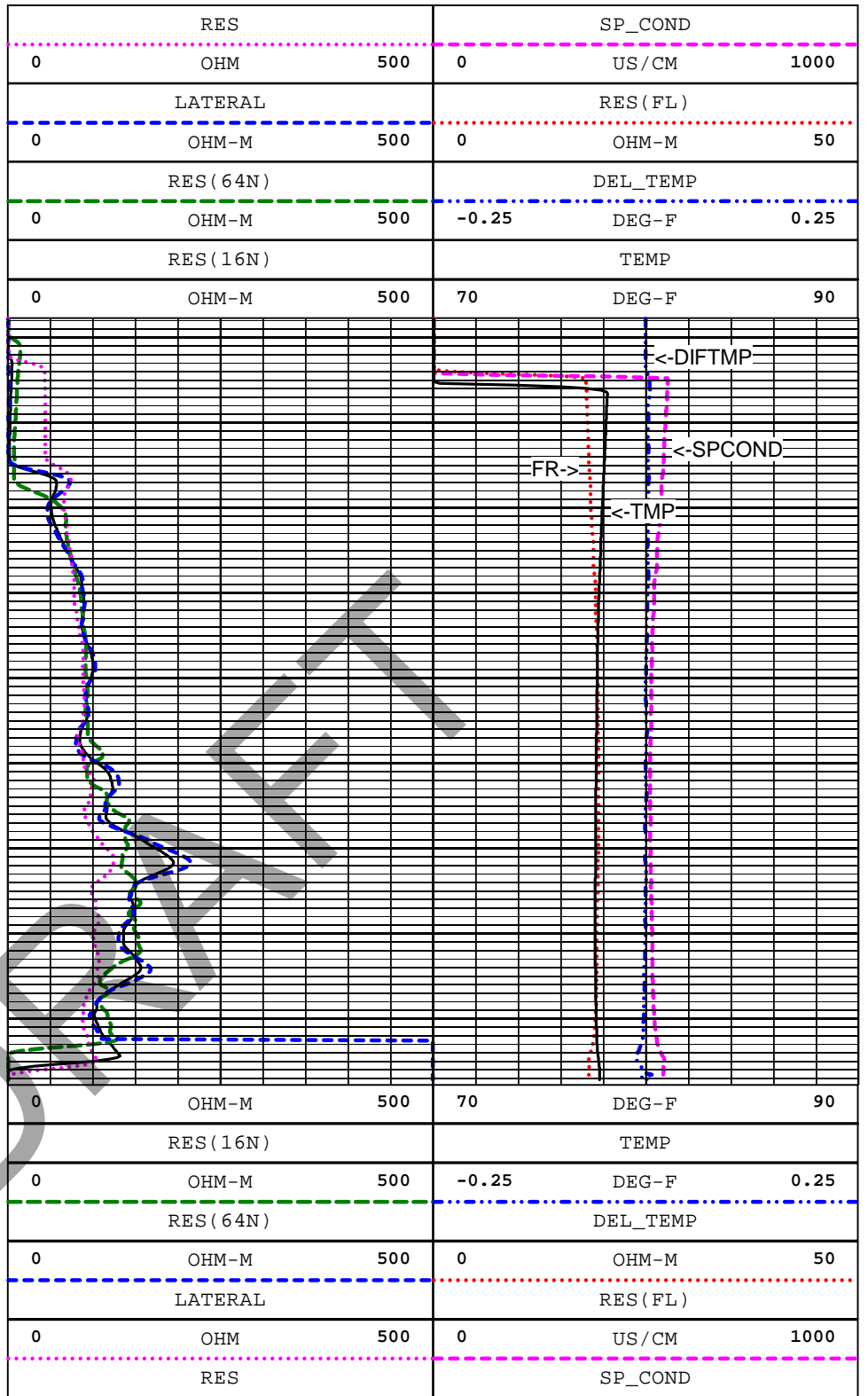


SP		
-250	MV	250
GAMMA		
200	API-GR	800
GAMMA		
0	API-GR	200
GAMMA		
200	API-GR	800
GAMMA		
-250	MV	250
SP		

FEET



150



FEET

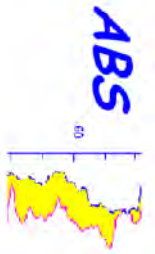
TOOL CALIBRATION WELL10 06/10/20 09:48

TOOL 8044A TM VERSION 2002

SERIAL 938

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jan03,03	02:49:05	GAMMA [API-GR]	0.001	180.000	0.000	169
2	Apr09,20	11:12:41	RES(FL) [OHM-M]	3.060	28.160	13413	37152
3	Aug17,14	12:00:23	SP [MV]	0.000	395.000	59670	23612
4	Feb02,20	14:59:18	RES(16N) [OHM-M]	0.000	1996.000	4010	103211
5	Feb02,20	15:00:15	RES(64N) [OHM-M]	0.000	1990.000	4089	103487
6	Sep29,19	18:57:40	TEMP [DEG-F]	71.700	86.100	63355	57070
7	Aug17,14	10:39:11	RES [OHM]	0.000	988.000	9855	58788

DRAFT



# PRODUCTION - STATIC - PUMPING WELL 10

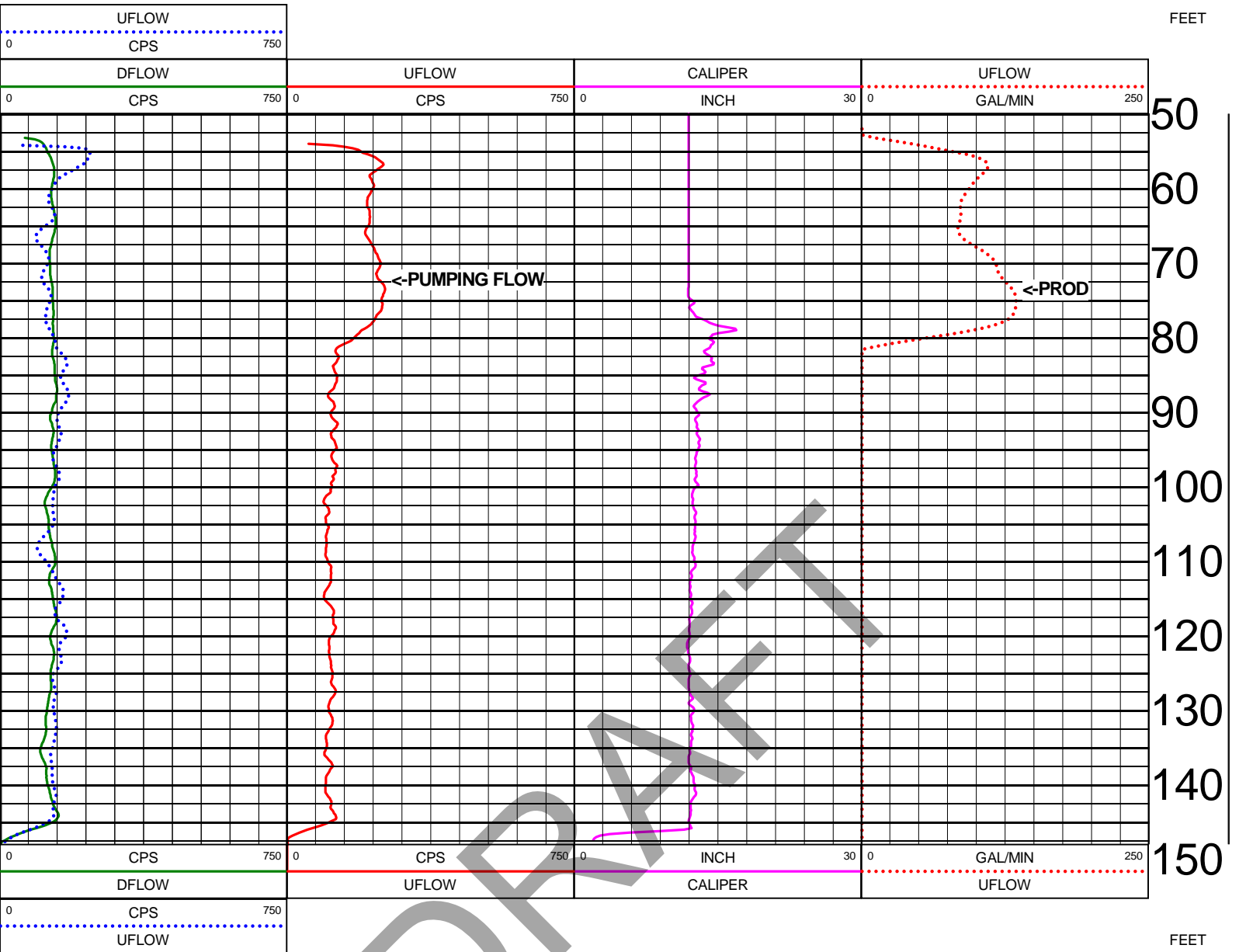
COMPANY APPLIED DRILLING ENG.  
 WELL WELL10  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 APH NO.

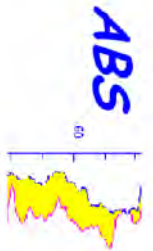
COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL10  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE  
 SECTION :  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY7\_JL56  
 TOWNSHIP :  
 RANGE :

PERMANENT DATUM MSL  
 DRI MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

Elevations:  
 KB FT  
 DF FT  
 GL FT  
 Other Services:  
 8044  
 8711  
 VIDEO

DATE	TIME	DEPTH DRILLER	FT	FT	Other Services:
		DEPTH LOGGER	148	FT	
		FIRST READING		FT	
		LAST READING		FT	
		BIT SIZE	6	IN	
		CASING -- DRILLER	NA	FT	
		CASING -- LOGGER	74	FT	
		CASING O.D.	12	IN	
		CASING TYPE	STEEL		
		FLUID TYPE	FOR		
		FLUID DENSITY		LB/GAL	
		FLUID VISCOSITY			
		FLUID PH			
		MUD SOURCE			
		RM @ MEAS TEMP	@	F	
		RMF @ MEAS TEMP	@	F	
		RMC @ MEAS TEMP	@	F	
		CIRC STOPPED			
		RIG NUMBER			
		RECORDED BY	AFB		
		WITNESSED BY	MAC DADDY		
		REMARKS 1			
		REMARKS 2			
		REMARKS 3			





# PUMPING WATER QUALITY WELL10

COMPANY APPLIED DRILLING ENG.  
 WELL WELL10  
 WELL EXT  
 FIELD  
 COUNTY PINELLAS  
 STATE FLORIDA  
 COUNTRY USA  
 API NO.

COMPANY : APPLIED DRILLING ENG.  
 WELL : WELL10  
 WELL EXT :  
 FIELD :  
 COUNTY : PINELLAS  
 STATE : FLORIDA  
 COUNTRY : USA  
 API NO. :  
 UNIQ ID : NONE

SECTION: TOWNSHIP: RANGE:  
 LOCATION : BELLAIRE  
 LAT GPS UTM  
 LON GPS UTM  
 DISPLAY7\_JL56

PERMANENT DATUM MSL  
 DRI MEASURED FROM NA  
 LOG MEASURED FROM PAD  
 ELEV. PERM. DATUM FT

Elevations: FT 8044  
 KB DF FT 8711  
 GL VIDEO

DATE TIME

DEPTH DRILLER NA FT

DEPTH LOGGER 148 FT

FIRST READING FT

LAST READING FT

BIT SIZE 6 IN

CASING -- DRILLER NA FT

CASING -- LOGGER 74 FT

CASING O.D. 12 IN

CASING TYPE STEEL

FLUID TYPE FOR

FLUID DENSITY LB/GAL

FLUID VISCOSITY

FLUID PH

MUD SOURCE

RM @ MEAS TEMP @ F

RMC @ MEAS TEMP @ F

CIRC STOPPED

RIG NUMBER

RECORDED BY AFB

WITNESSED BY MAC DADDY

REMARKS 1

REMARKS 2

REMARKS 3

GAMMA		
200	API-GR	400
GAMMA		
0	API-GR	200
GAMMA		
0	API-GR	200
GAMMA		
200	API-GR	400
GAMMA		

FEET

60

70

80

90

100

110

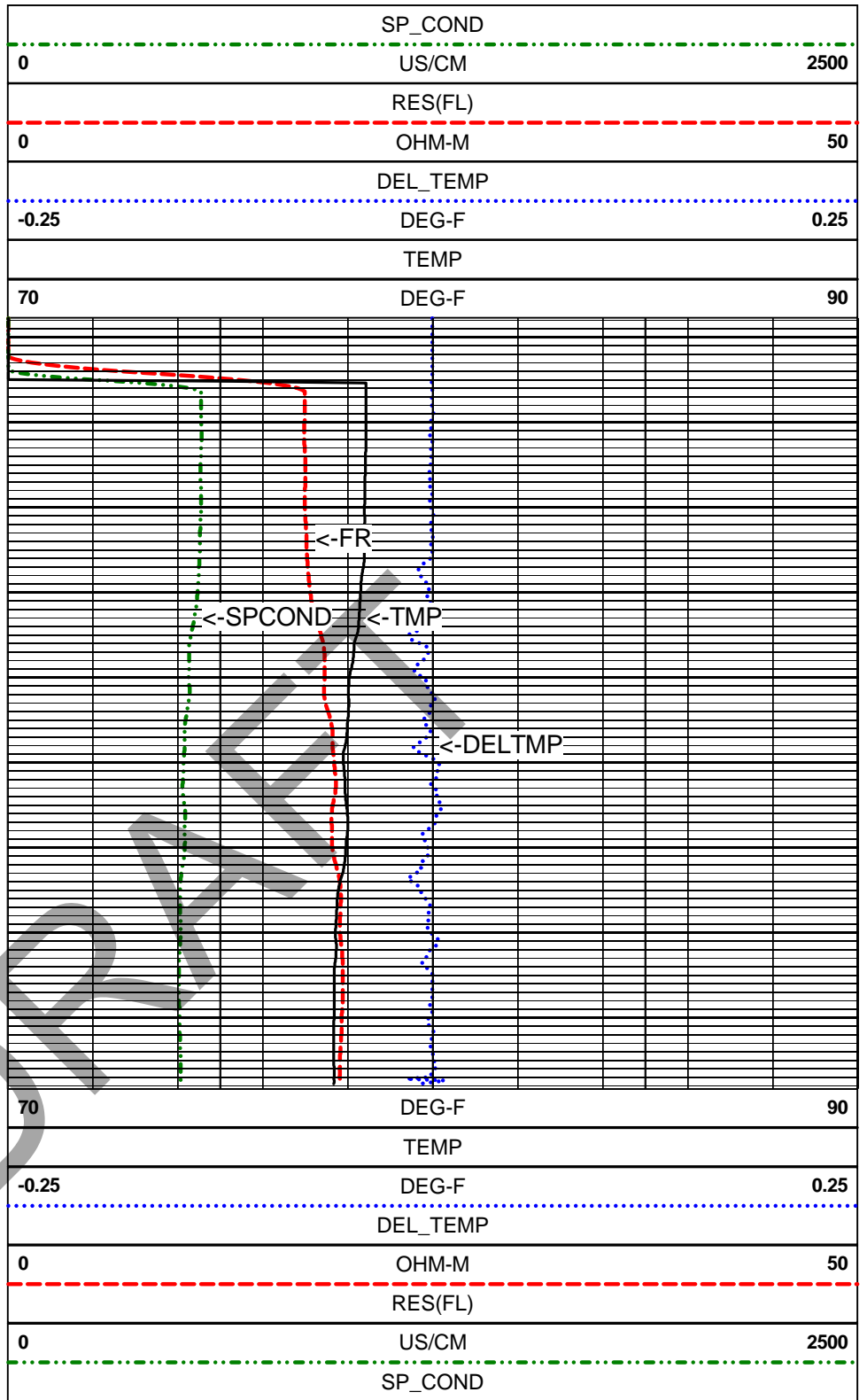
120

130

140

150

FEET



TOOL CALIBRATION WELL10 06/10/20 10:28

TOOL 8044A TM VERSION 2002

SERIAL 938

	DATE	TIME	SENSOR	STANDARD		RESPONSE [CPS]	
				Point1	Point2	Point1	Point2
1	Jan03,03	02:49:05	GAMMA [API-GR]	0.001	180.000	0.000	169
2	Apr09,20	11:12:41	RES (FL) [OHM-M]	3.060	28.160	13413	37152
3	Aug17,14	12:00:23	SP [MV]	0.000	395.000	59670	23612
4	Feb02,20	14:59:18	RES (16N) [OHM-M]	0.000	1996.000	4010	103211
5	Feb02,20	15:00:15	RES (64N) [OHM-M]	0.000	1990.000	4089	103487
6	Sep29,19	18:57:40	TEMP [DEG-F]	71.700	86.100	63355	57070
7	Aug17,14	10:39:11	RES [OHM]	0.000	988.000	9855	58788

DRAFT

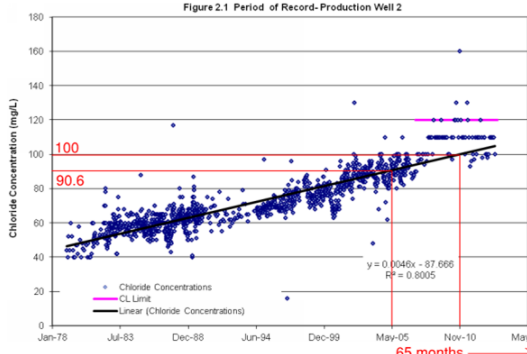
# APPENDIX C

DRAFT



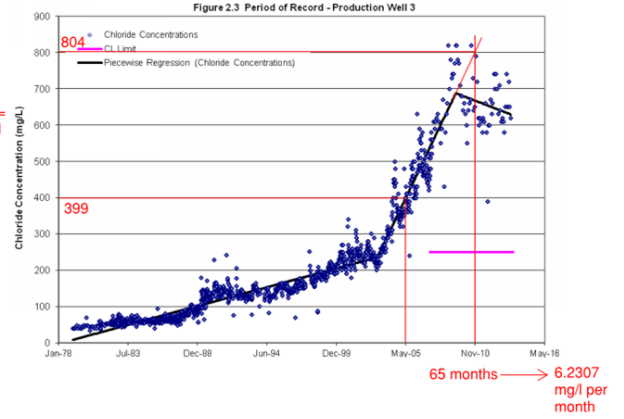
# Raw Water Quality (Chloride) Projections Calculations

**Chloride Concentration versus Time  
Production Well 2**

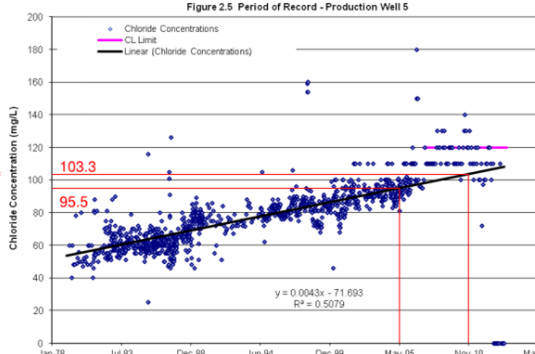


**Figure 2.2 Five-Year Period - Production Well 2**

**Chloride Concentration versus Time  
Production Well 3**

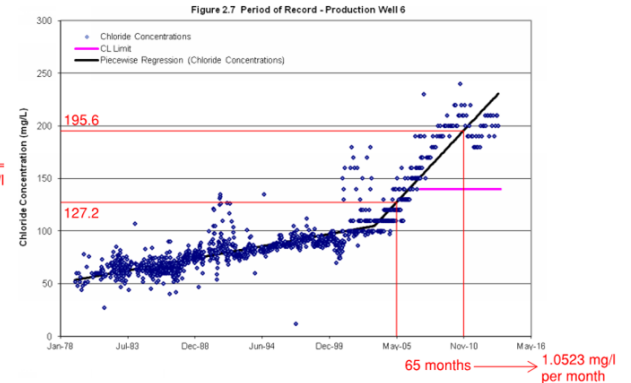


**Chloride Concentration versus Time  
Production Well 5**

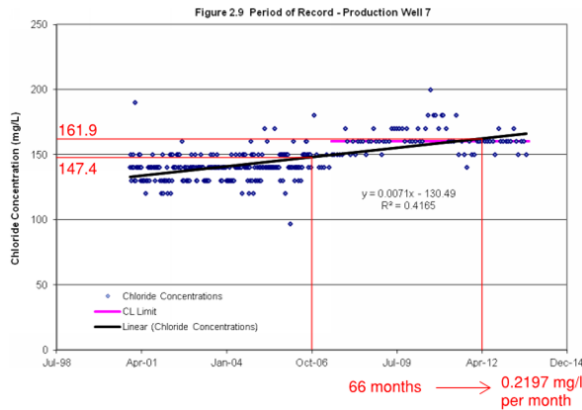


**Figure 2.6 Five-Year Period - Production Well 5**

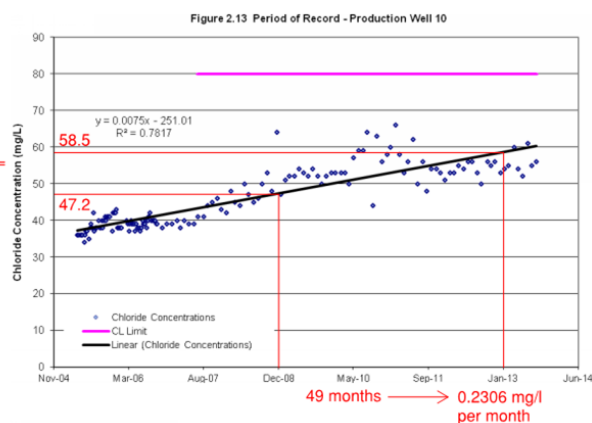
**Chloride Concentration versus Time  
Production Well 6**



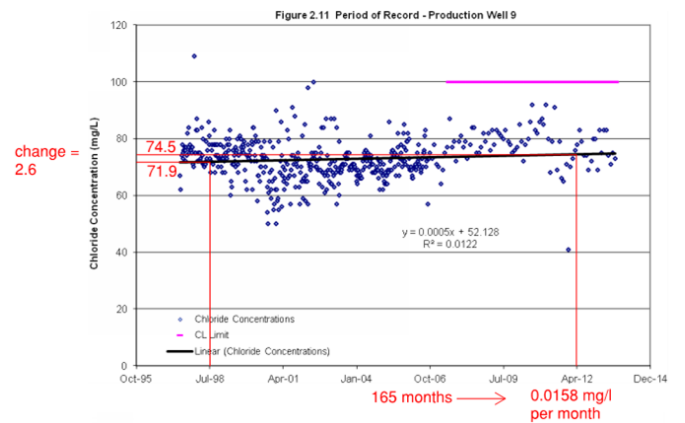
**Chloride Concentration versus Time  
Production Well 7**



**Chloride Concentration versus Time  
Production Well 10**



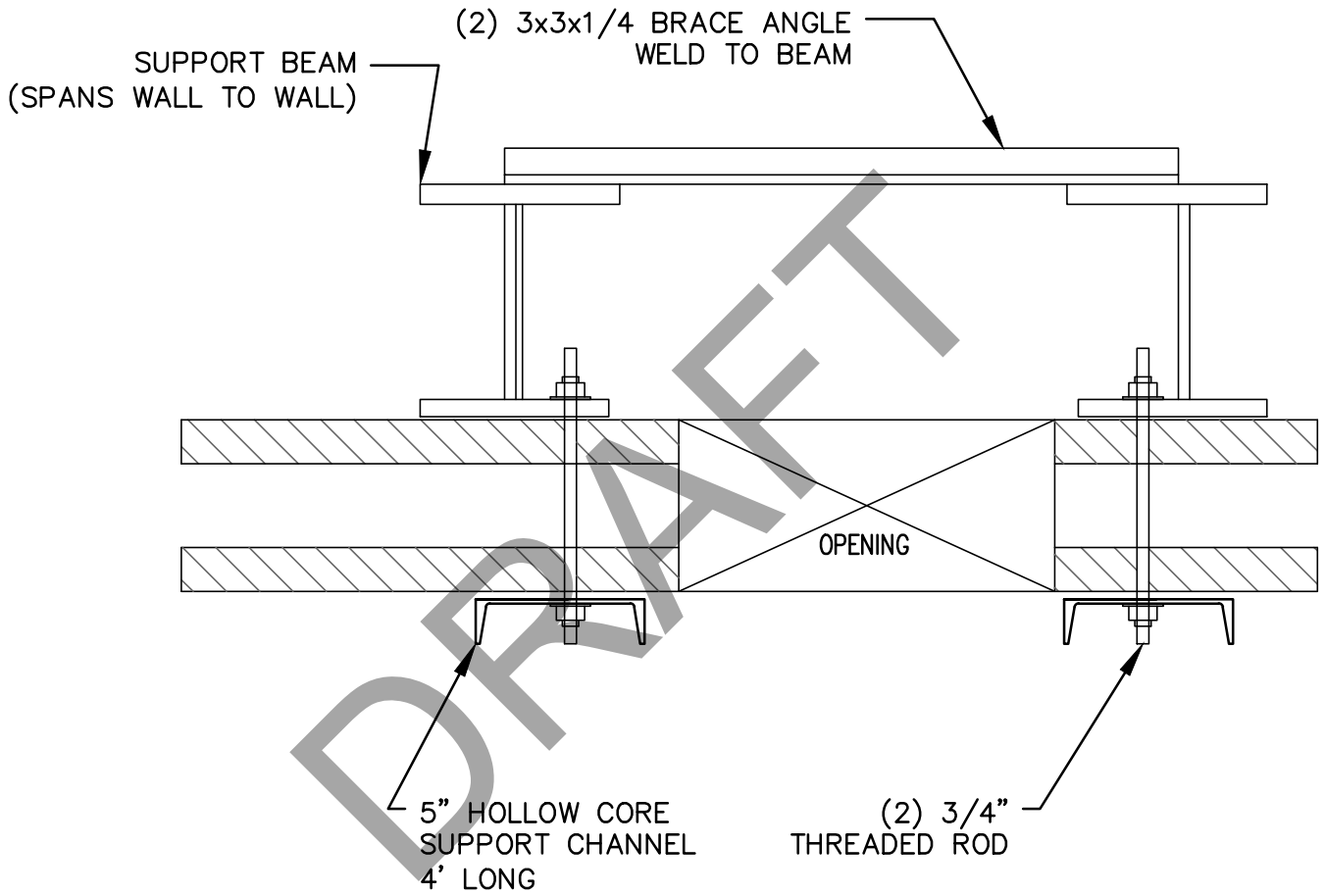
**Chloride Concentration versus Time  
Production Well 9**



# APPENDIX D

DRAFT

I:\01131\0014\WATER\80-DRAWINGS\FIGURES\FIGURE XX CLEARWELL ROOF.DWG 07/29/2020 15:43:45 CATHY WALKER



BELLEAIR WTP IMPROVEMENTS

CLEARWELL ROOF  
SUPPORT



**TOWN OF  
BELLEAIR**  
901 PONCE DE LEON BOULEVARD  
Florida 34104  
(727) 588-3769

DRAFT

SUBMITTED BY:

**MCKIM & CREED** | CLEARWATER, FL

727.442.7196 | MCKIMCREED.COM



**MCKIM & CREED**

ENGINEERS SURVEYORS PLANNERS



Legislation Details (With Text)

**File #:** 20-0214      **Version:** 1      **Name:**  
**Type:** Discussion Items      **Status:** Agenda Ready  
**File created:** 8/28/2020      **In control:** Infrastructure Board  
**On agenda:** 8/31/2020      **Final action:**  
**Title:** Discussion of Capital Improvements Master Plan  
**Sponsors:** JP Murphy  
**Indexes:**  
**Code sections:**  
**Attachments:** [MCKCIP](#)  
[CIP - Copy of Working Drafts](#)

Date	Ver.	Action By	Action	Result
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**Summary**

To: Infrastructure Board  
From: JP Murphy, Town Manager  
Date: 8/28/2020

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**Subject:**  
Discussion of Capital Improvements Master Plan

**Summary:**  
Phil Locke from McKim and Creed will be reviewing their engineering approach to development of the Capital Improvement Master Plan for approval by the board, once the methodology is approved staff will revise and rebalance a draft 5 year CIP budget based on project ranking. Mr. Locke will provide more detail at the meeting.



**Town of Belleair**  
**Infrastructure Master Plan**  
**Monday, August 10, 2020**  
**Infrastructure Prioritization Criteria**

No.	Criteria	Rating Considerations	Weighting Factor <sup>1</sup>	Weighting Factor Considerations
1	Pavement Quality Improvement	Includes goals of improving PCI to min of 70; all roads >50 PCI; considers existing pavement condition & appearance; mill/resurface = 3; rebuild = 5	1	Improved roadway structure
2	Project Cost/Ease of Implementation	Includes project costs (lower cost is higher rated); permitting requirements; etc.; considers cost of not making the improvements (i.e., maintenance costs)	2	Lower weighting factor - functionality considered higher than cost
3	Construction Cost Efficiency "Dig Once"	Higher rating for projects including multiple infrastructure components (paving improvements, lighting, stormwater, water mains, sidewalks, etc.)	2	Ensures that utility conflicts are reduced as compared to separate projects in same area
4	Safe Multi-Modal Connectivity	Biking/walking paths, general transportation safety, sidewalks (i.e., roadway widening)	2	Encourages alternative transportation; helps with traffic management
5	Public Perception / Support	Appearance, environmental benefits, overall community benefits	1	Lower weighting considers functionality
6	Drainage / Erosion Control Improvements	Higher rating for roadway projects that address ongoing flooding issues and erosion improvements projects such as the Bluff and Belleair Creek	3	Reduced flooding provides safety and bank stabilization reduces maintenance costs and risks
7	Public Safety	Overall community safety	4	Considers overall safety (i.e., lighting, flooding, bridges, Bluff, bike paths, etc.)
8	Overall Impact/Risk Reduction	Higher ratings for projects that address unknown financial and other consequential risks (i.e. bridge failure, creek bank failure)	4	Critical to inform Board of projected infrastructure improvements and budgetary costs
9	Funding / Cost-Sharing	Higher ratings for projects that include, or may include, funding from outside agencies	1	Obvious financial benefits; needs to consider and compare project needs vs financial benefits
10	Water Main Improvements	Higher rating for projects that replace CIP/galvanized pipe and/or address hydraulic deficiencies	2	Maintaining potable water service

1) Scale of 1-4; level of importance pertaining to the respective criteria; higher is more important









## Project Cost Summary

Worksheet #	Project	Total Estimated Capital Cost	Capital Cost from Town	Difference
1	Bayview Drive Bridge to IRR	\$86,000		
4	Carl	\$1,206,000	\$600,000	101%
5	IRR Poinsettia to Rosery	\$730,000	\$750,000	-3%
6	IRR Rosery to Mehlenbacher	\$1,373,000	\$1,500,000	-8%
7	IRR Bayview to Belleview (Alt 1 - Mill & Resurface)	\$207,000	\$1,531,160	-86%
8	IRR Bayview to Belleview (Alt 2 -Mill & Resurface and Bike Path)	\$684,000		
10	Magnolia Wall/One Way (Concept)	\$129,000	\$185,000	-30%
11	Ocala/Osceola (Osceola from Oleander to Manatee 2)	\$1,035,000	\$1,500,000	-31%
12	Ocala/Ponce (Ponce from Manatee to Oleander 2)	\$1,490,000	\$1,200,000	24%
13	Orlando/Osceola (Osceola from Oleander to Manatee 1)	\$956,000	\$1,600,000	-40%
14	Orlando/Ponce (Ponce from Manatee to Oleander 1)	\$2,279,000	\$2,014,100	13%
15	Osceola East of IRR	\$1,579,000	\$525,000	201%
16	Pinellas/Ponce (Phase 3)	\$930,000	\$1,200,000	-23%
17	Pinellas/Ponce (Phase 4)	\$1,315,000	\$1,500,000	-12%
18	Poinsettia	\$48,000	\$50,000	-4%
19	Ponce from Manatee to Rosery	\$1,459,391	\$1,595,000	-9%
20	Ponce from Roundabout to Trail	\$2,484,000	\$2,035,000	22%
23	The Mall/Gardenia	\$1,318,000	\$1,361,250	-3%
24	Wildwood/Woodlawn	\$117,000	\$182,488	-36%
2	Belleair Creek (Ponce to Bridge)	\$4,000	\$5,000,000	-100%
3	Bridge Repairs	\$4,000	\$5,000,000	-100%
21	Seawall Replacements	\$4,000	\$750,000	-99%
22	The Bluff	\$4,000	\$5,000,000	-100%

<b>Revenues</b>	<b>19/20</b>	<b>20/21</b>	<b>21/22</b>	<b>22/23</b>	<b>23/24</b>
Infrastructure Mill (1.2500)	\$997,509	\$1,057,630	\$1,154,550	\$1,203,150	\$1,236,050
Penny for Pinellas	\$508,165	\$451,500	\$460,550	\$469,750	\$550,050
Electric Utility Tax	\$425,000	\$430,000	\$430,000	\$430,000	\$430,000
SWFWMD Grant					
	<i>Pinellas</i>	\$1,375,000			
	<i>Bayview</i>				
	<i>Belleair Creek</i>				
	<i>Bluff</i>		\$135,000		
LAP (Federal) Funding (Targets)					
Other Governments				\$500,000	\$3,000,000
Stormwater Management Grant					
Intergovernmental Services Rendered					
Stormwater Fee	\$333,948	\$333,948	\$333,948	\$333,948	\$337,400
Interest	\$71,781				
Donations	\$50,000				
Miscellaneous	\$7,589				
BCC Lease	\$350,000	\$22,500			
Reserves Prior Years					
Transfers In					
PY PO Rev					
AHLF Property Sale	\$1,508,225				
Loan Proceeds	\$4,688,336				
AMOUNT TO BALANCE		\$835,290			
<b>Totals</b>	<b>\$10,315,553</b>	<b>\$3,265,868</b>	<b>\$2,379,048</b>	<b>\$2,936,848</b>	<b>\$5,553,500</b>

<b>Expenditures</b>	<b>19/20</b>	<b>20/21</b>	<b>21/22</b>	<b>22/23</b>	<b>23/24</b>
Park Improvements	\$21,971	\$75,000	\$25,000	\$25,000	\$25,000
Hunter Park					
Pavement Management	\$60,000				
Beautification & Entrances					
Street Light Replacement	\$25,000	\$25,000	\$25,000	\$25,000	\$25,000
Capital Parks					
	<i>Magnolia/Wall/Tackett</i>				
Street Signs	\$10,000				
Refund Exp					
Harold's Lake Cleanout				\$225,000	
ABM Electrical and Roofing					
ABM Field Lighting					
ABM Base Scope					
Small Roadway Projects					
	<i>Pavement Management/Resurfacing</i>	\$80,000	\$80,000	\$80,000	\$80,000
	<i>Sidewalk/Curb Management</i>	\$177,672	\$20,000	\$20,000	\$20,000
	<i>Point Repairs</i>	\$144,376	\$142,500	\$147,500	\$168,100

Indian Rocks Road

Projects Years 1-5	<u>19/20</u>	<u>20/21</u>	<u>21/22</u>	<u>22/23</u>	<u>23/24</u>
<b>Pinellas/Ponce(Phase 2)</b>					
<i>Professional Services</i>					
<i>Construction</i>	\$2,913,224	\$557,624			
<b>Rosery Rd</b>					
<b>Palmetto</b>					
<i>Professional Services</i>					
<i>Construction</i>	\$950,640	\$29,709			
<b>Palmetto (Phase 2) North</b>					
<i>Professional Services</i>					
<i>Construction</i>	\$1,817,135				
<b>Belleair Creek</b>					
<b>Carl</b>					
<i>Professional Sevices</i>	122669				
<i>Construction</i>		\$800,000			
<i>Shirley/Varona/Sunny/Barb (Op)</i>					
<b>Belforest</b>					
<i>Construction</i>					
<b>Bayview Bridge to IRR</b>					
<i>Professional Services</i>		\$16,320	\$163,200		
<i>Construction</i>				\$199,000	\$1,161,000
<b>IRR Ponce to Melenbacher</b>					
<i>Professional Services</i>					
<i>Construction</i>					
<b>The Bluff</b>					
<i>Study</i>		\$270,000			
<i>Point Repairs</i>					
<i>Professional Services (Conceptual)</i>			\$300,000		
<i>Construction (Conceptual)</i>				\$2,500,000	\$2,500,000
<i>Seawall Replacement</i>				\$220,000	
<b>Belleair Creek (Ponce to Bridge)</b>					
<i>Study</i>					
<i>Professional Services (Conceptual)</i>					\$850,000
<i>Construction (Conceptual)</i>					
<i>Point Repairs</i>					
<b>Ponce from Roundabout to Trail</b>					
<i>Professional Services (Conceptual)</i>					
<i>Construction (Conceptual)</i>			\$1,017,500	\$1,017,500	
<b>Pinellas/Ponce(Phase 3)</b>					
<i>Professional Sevices</i>					
<i>Construction</i>					
<b>Bridge Repairs</b>					
<i>Engineering</i>					

*Scour Protection  
Seawall Repairs  
Grout/Deck Repair  
Replacement*

**Magnolia Wall/One Way (Concept)**

*Professional Services  
Construction*

**IRR (Poinsettia to Rosery)**

*Professional Services  
Construction \$750,000  
Point Repairs/Overlay*

Projects Years Beyond 6 Years (Conceptual)	19/20	20/21	21/22	22/23
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**Seawall Replacements**

*Thompson Park  
Winston Park  
Coe Rd.*

**Pinellas/Ponce(Phase 4)**

*Professional Services  
Construction*

**IRR (Rosery to Mehlenbacher)**

*Professional Services  
Construction*

**The Mall/Gardenia**

*Professional Services  
Construction*

**Osceola East of IRR**

*Professional Services  
Construction*

**IRR Bayview to Belleview**

*Professional Services  
Construction*

**Ponce from Manatee to Oleander**

*Professional Services  
Construction*

**Wildwood/Woodlawn**

*Professional Services  
Construction*

**IRR Hunter Bayview to Poinsettia**

*Professional Services  
Construction*

**Poinsettia**

*Construction*

**Osecola from Oleander to Manatee**

*Professional Services  
Construction*

**Ponce from Manatee to Rosery**

**Professional Services**

**Construction**

**Other Expenses**

**Transfer to Reserves**

**Transfer to 001**

**BB&T Debt Service** \$603,141 \$794,000 \$794,000 \$794,000 \$794,000

**ABM Loan Debt Service** \$78,716 \$79,000 \$79,000 \$79,000 \$79,000

**GF Debt Service**

**Totals** **\$6,924,544** **\$2,889,153** **\$3,401,200** **\$5,332,000** **\$5,702,100**

**Fund Balance**

**19/20**

**20/21**

**21/22**

**22/23**

**23/24**

**Total Expenditures** \$6,924,544 \$2,889,153 \$3,401,200 \$5,332,000 \$5,702,100

**Total Revenue** \$10,315,553 \$3,265,868 \$2,379,048 \$2,936,848 \$5,553,500

**Ending Fund Balance** **\$9,757,790** **\$10,134,505** **\$9,112,353** **\$6,717,201** **\$6,568,601**